# DESIMONE

# **Foundation Permit Submittal**

Volume III - Tower Design

## 301 Mission Street San Francisco, CA

Prepared for:

## San Francisco Department of Building Inspection

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**DeSimone Project #4069** 

May 24, 2005

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# SECTION 8 – TOWER SUPERSTRUCTURE DESIGN

8.1 Primary Shear Wall/ Outrigger System

### 8.1.1 Shear Walls

**Load** - Seismic load cases control the design of the shear walls. RSA analysis results are used to obtain seismic (E) forces. Gravity loads are calculated by hand and are shown in spreadsheet format. Load combos are per UBC 1612.2.1, with equation 12-6 controlling for all walls.

**Vertical Steel Design** - The tower's main concrete core is broken into 4 parts. The rectangular walls on the north and south side are designed as a single piece. This methodology assumes plain section remains plain for the whole rectangular shape and is justified by looking at the shell stress distributions for the rectangular core. The two walls on the east and west, connecting the north and south rectangular walls, are designed as separate straight walls. The ETABS pier definitions are set as required to extract the total force in the 4 parts. PCA column is used to determine the quantity and placement of vertical steel.

**Horizontal Steel** – Shear forces come directly from  $V_u = V_E$  from ETABS. Pier definitions are set for each wall individually in order to extract design shears for each wall.

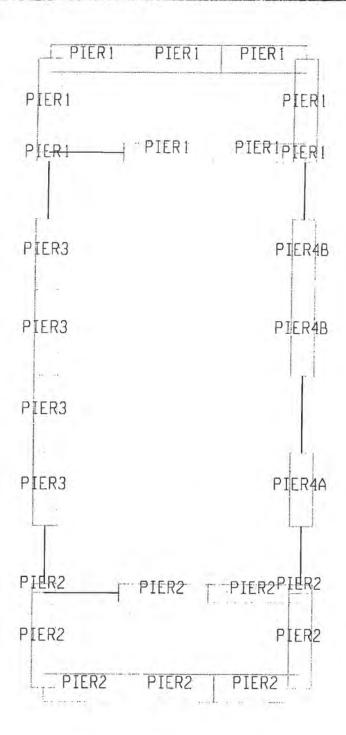
The excel spreadsheet shows calculations for the below ground walls (included in foundation permit) and above ground portion of the building (not included in foundation permit.)

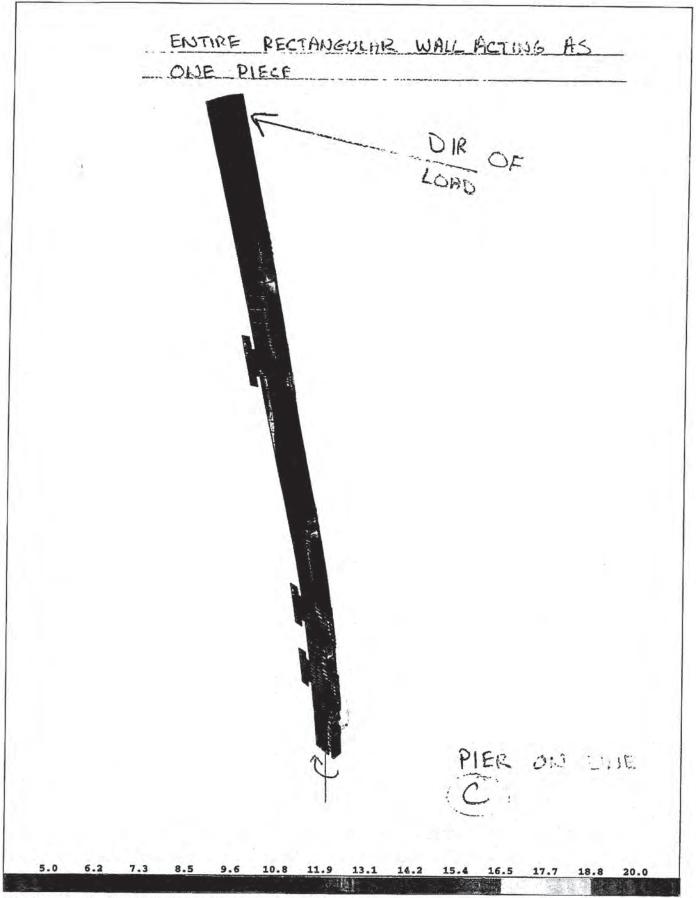
**Boundary Zone** – UBC 1921.6.6.4 methodology was used for foundation permit boundary element length for walls from pile cap to ground level. 25% is the maximum length of boundary element required by code. Preliminary detailed analysis (similar to the methodology of UBC 1921.6.6.5) indicates that 30% boundary element length is required at the bottom of the tower and is therefore provided.

Per Jack Moehle of UC Berkeley, the shear reinforcing and confinement requirements do not have to be added in the boundary zone. Therefore the shear reinforcing can be counted as part of the required confining steel.

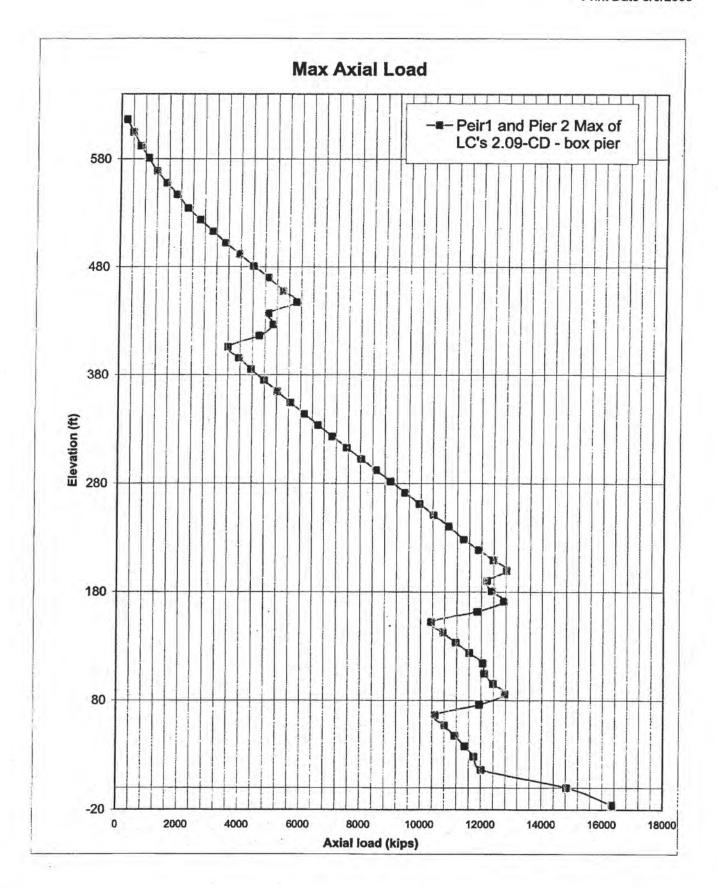
The excel spreadsheet shows calculations for the below ground walls (included in foundation permit) and above ground portion of the building (not included in foundation permit.)

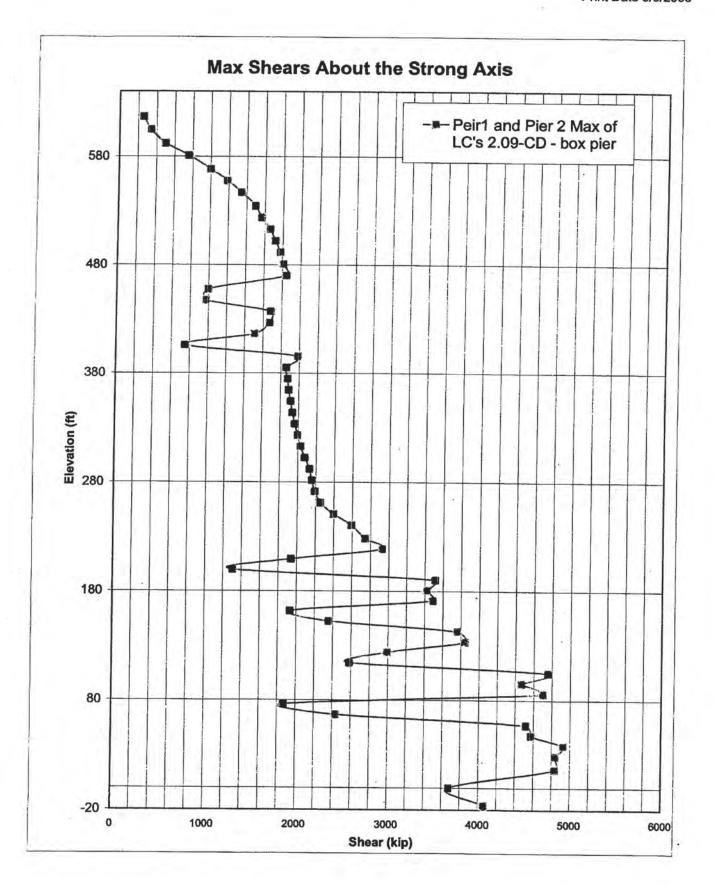
# PIER DEFINITION FOR VERTICAL STEEL DESIGN

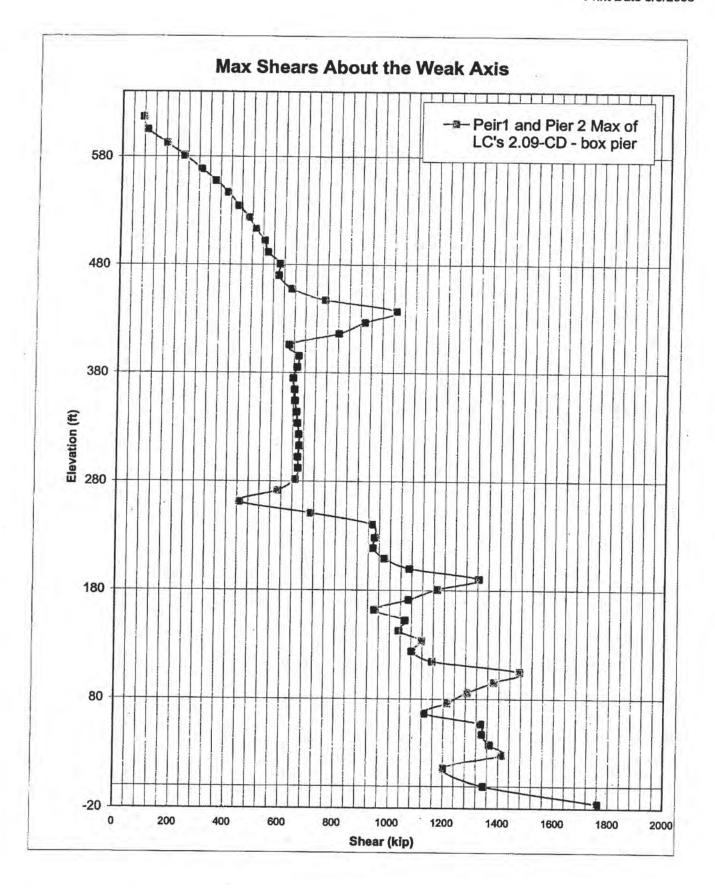


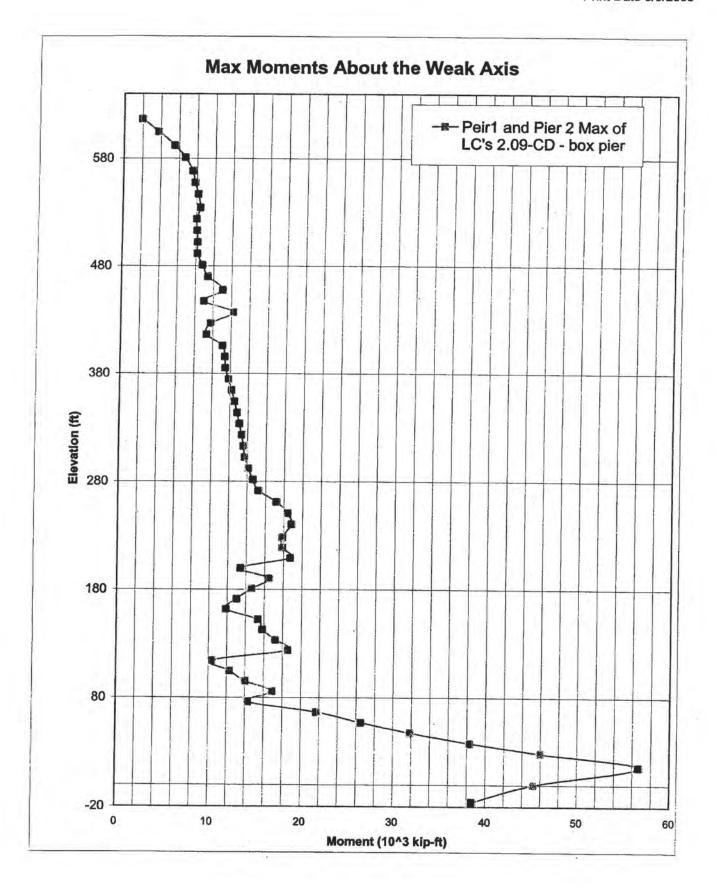


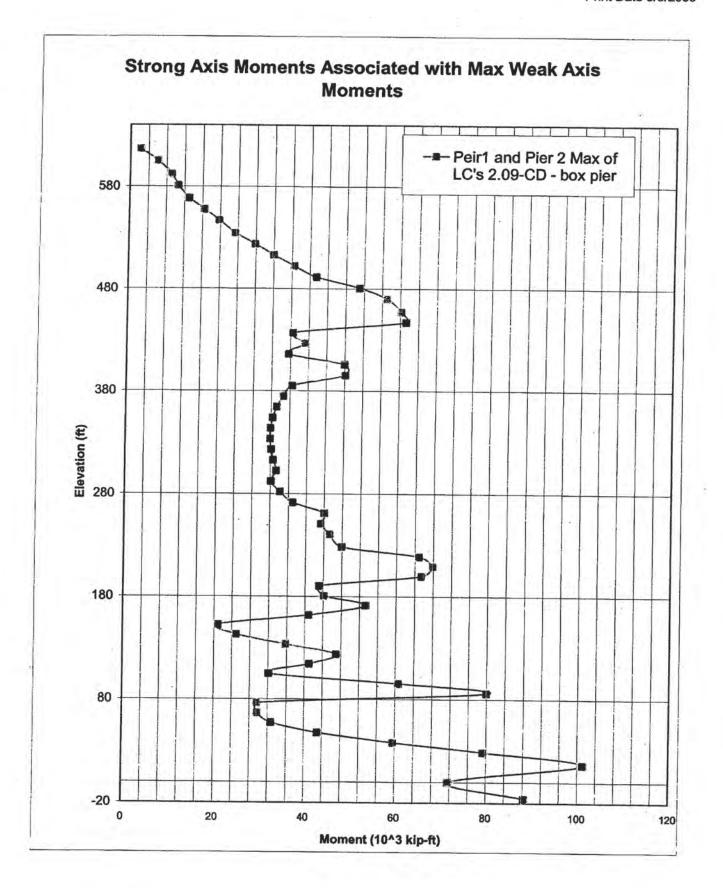
ETABS v8.5.0 - File: 4069-20050509-2.09-CD-straight - May 12,2005 15:18 3-D View Resultant F22 Diagram (UBCX) - Kip-in Units

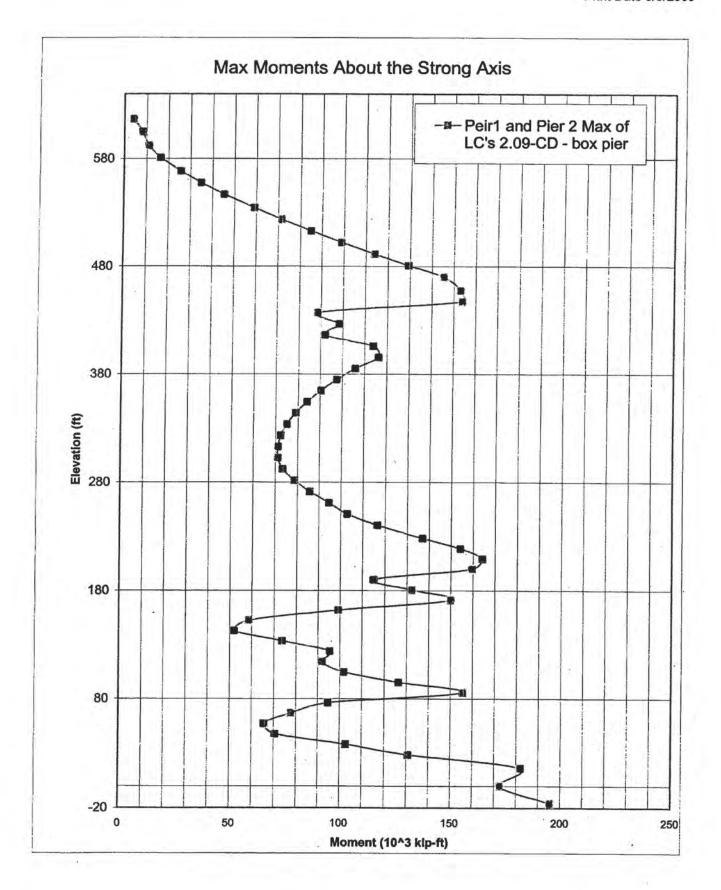


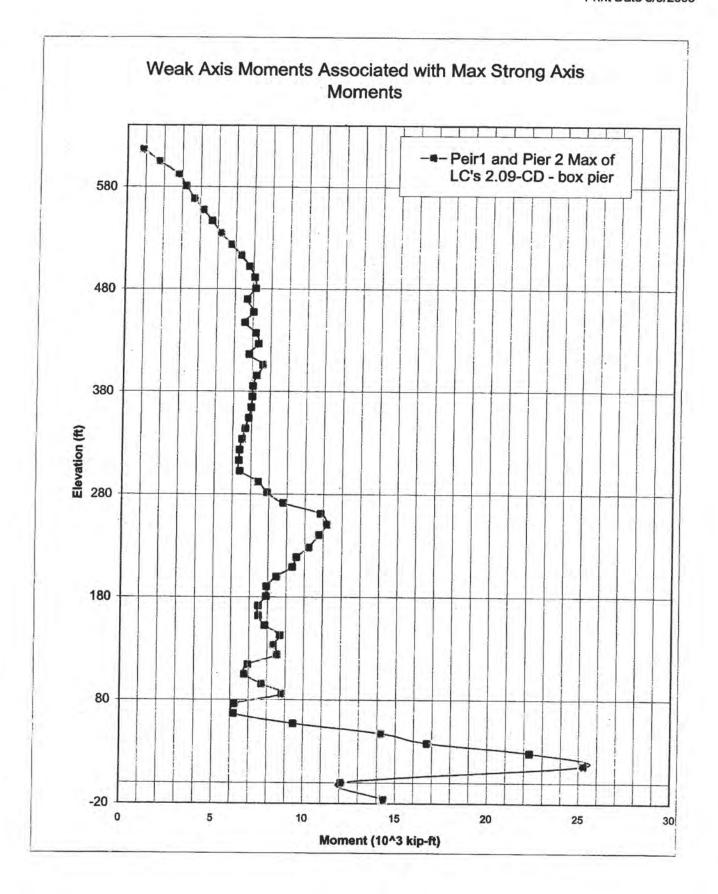




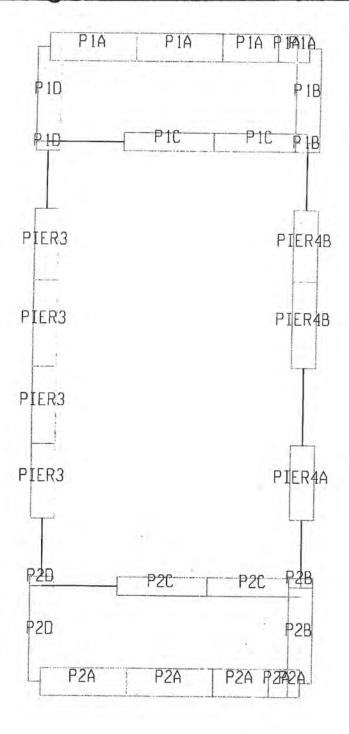


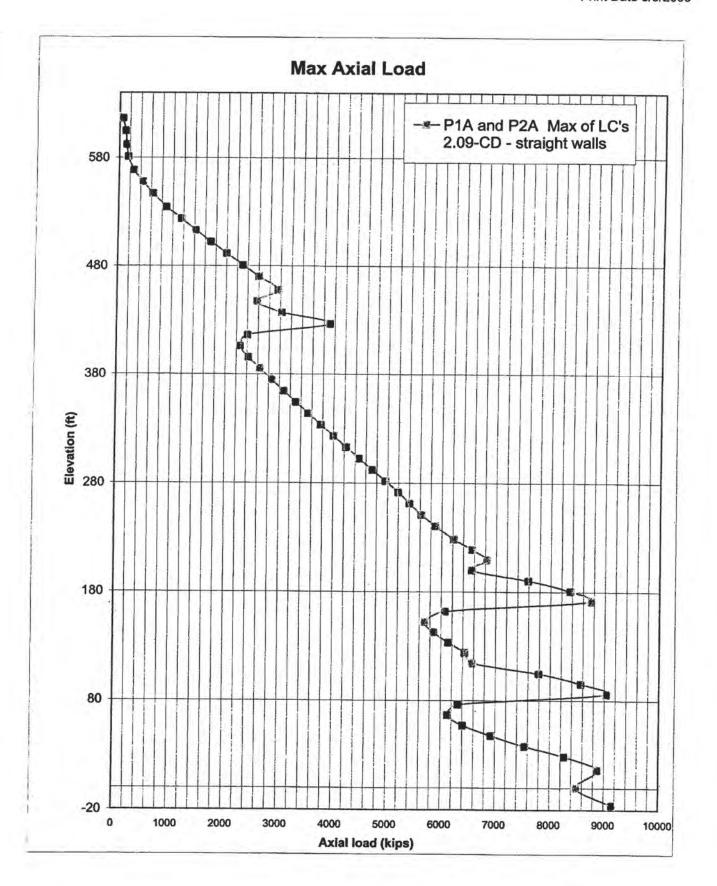


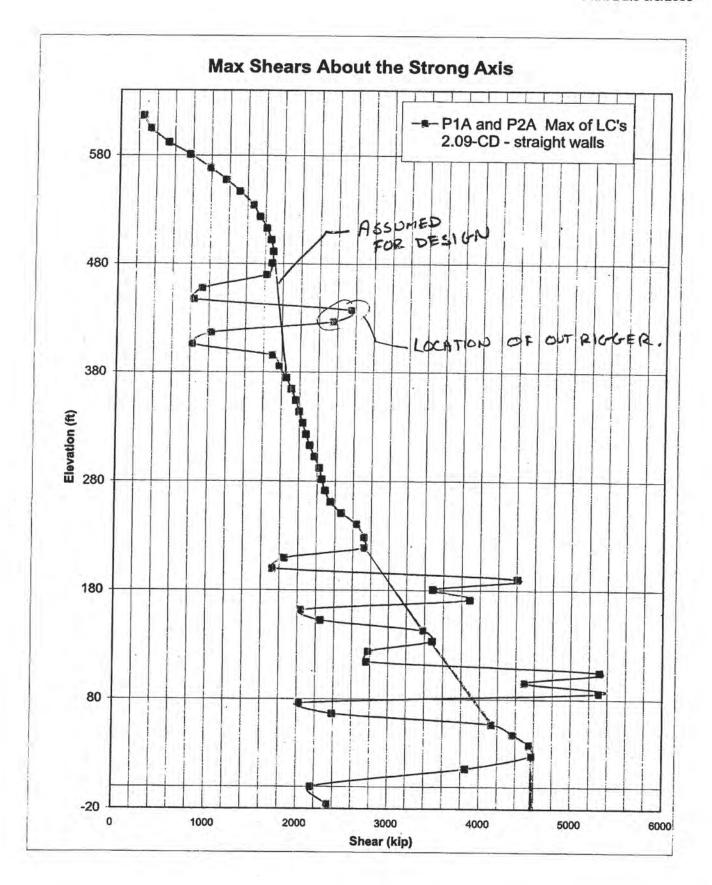


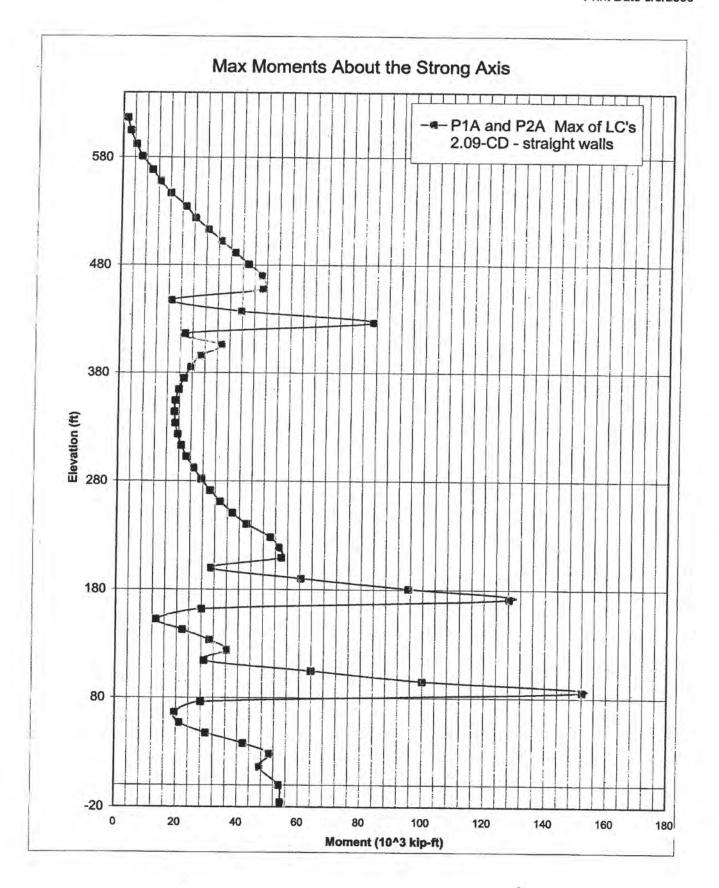


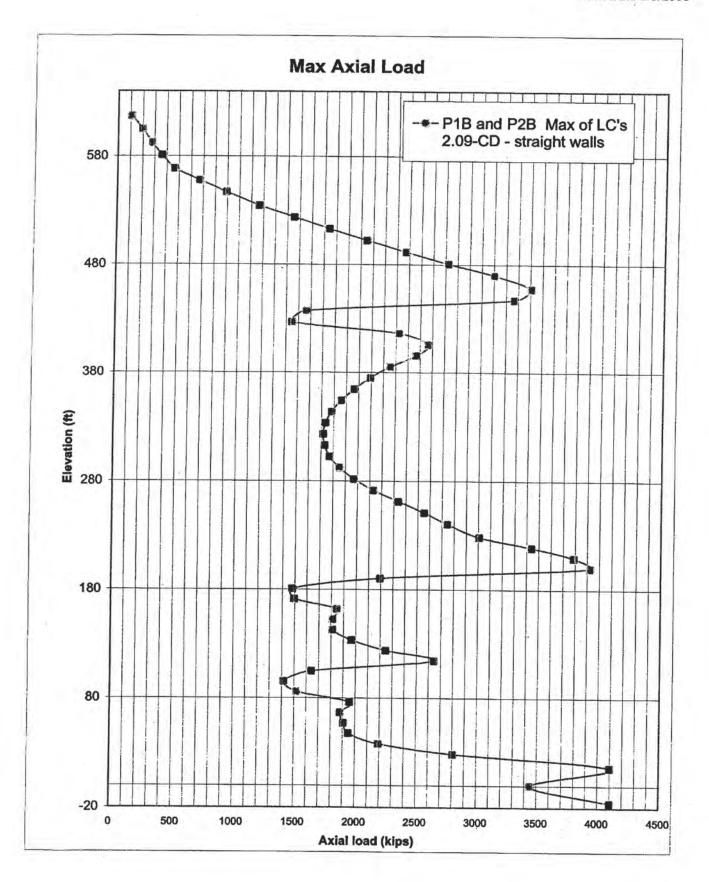
# PIER DEFINITION FOR SHEAR AND BE STEEL DESIGN

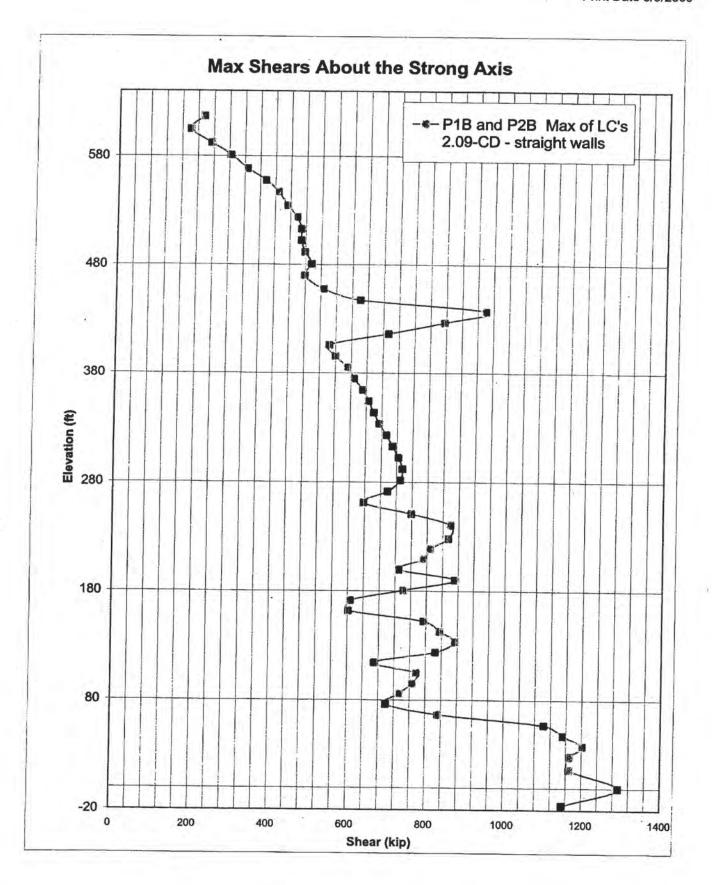


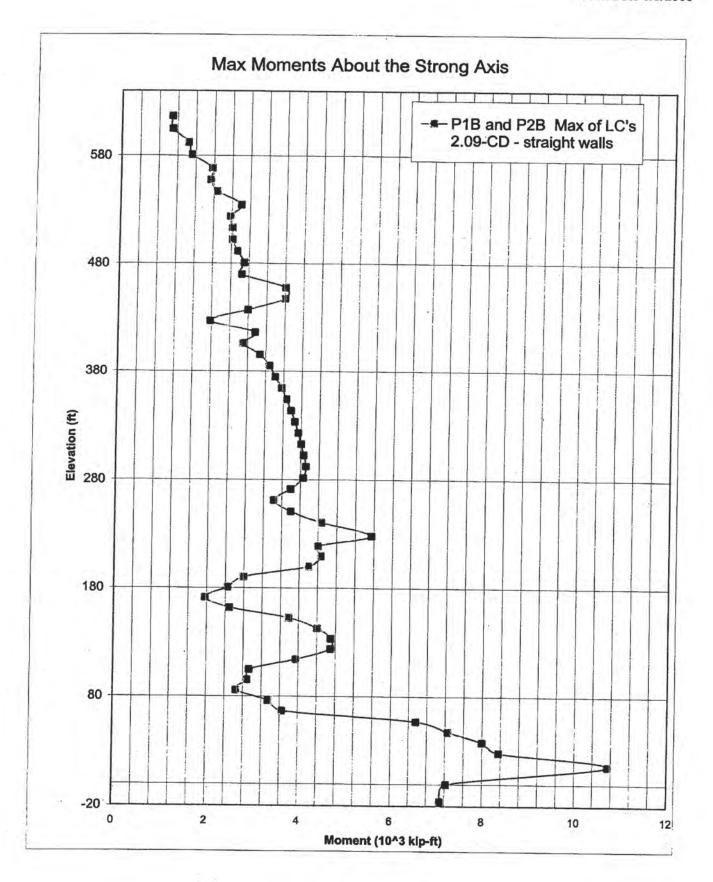


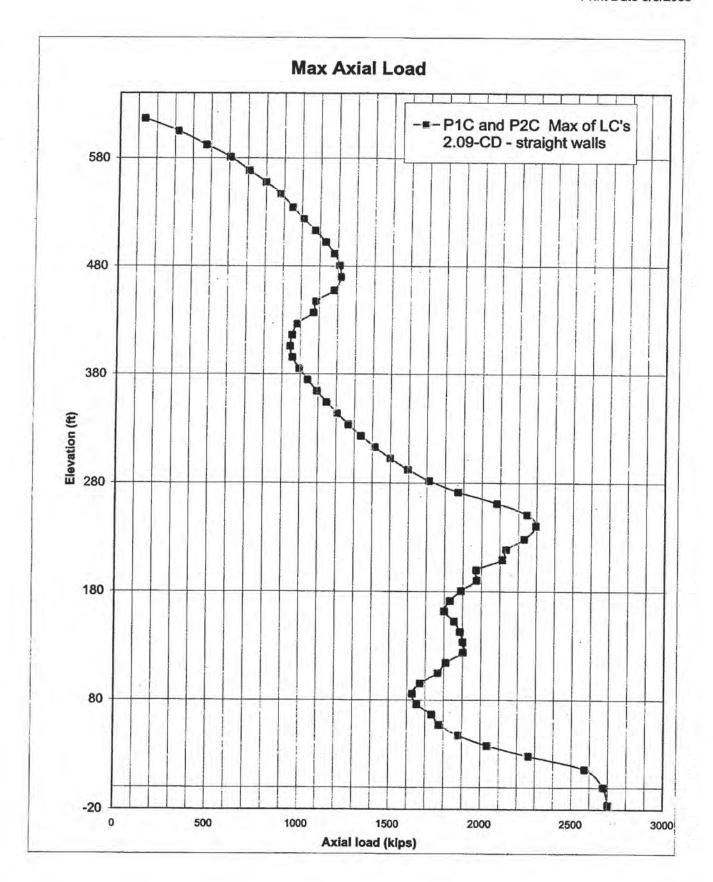


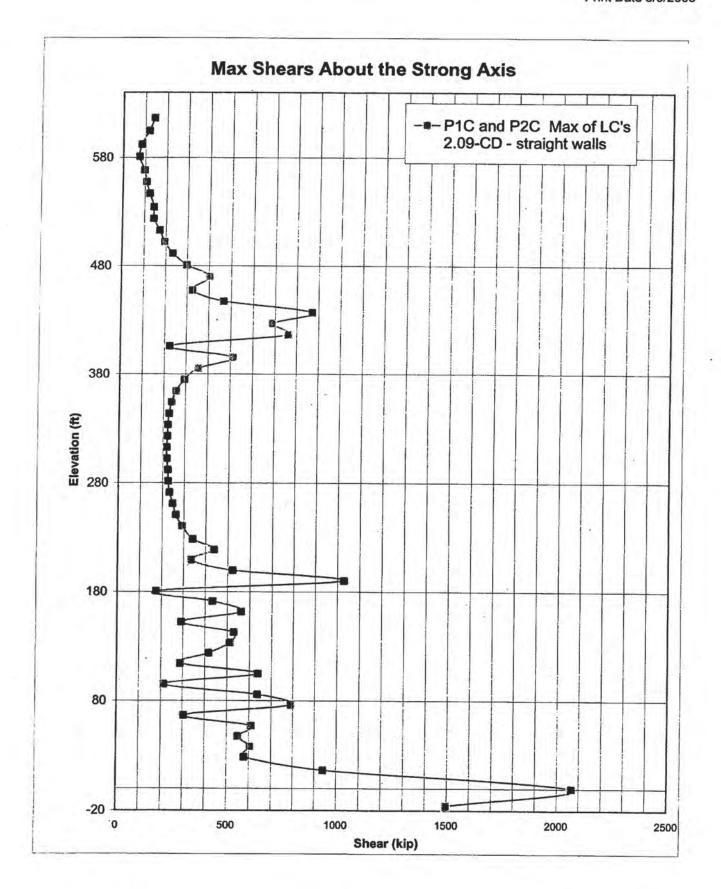


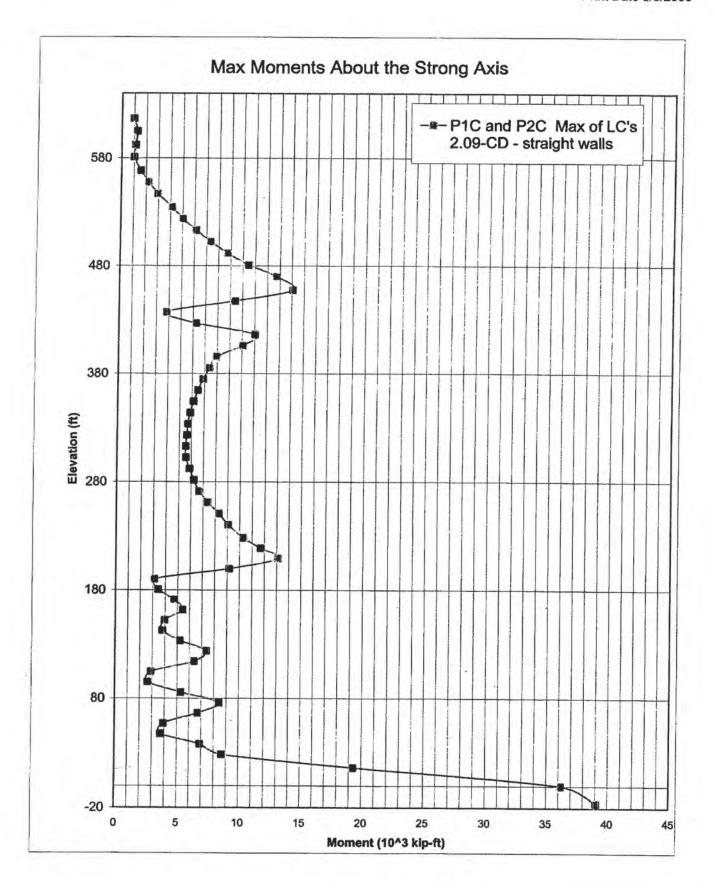


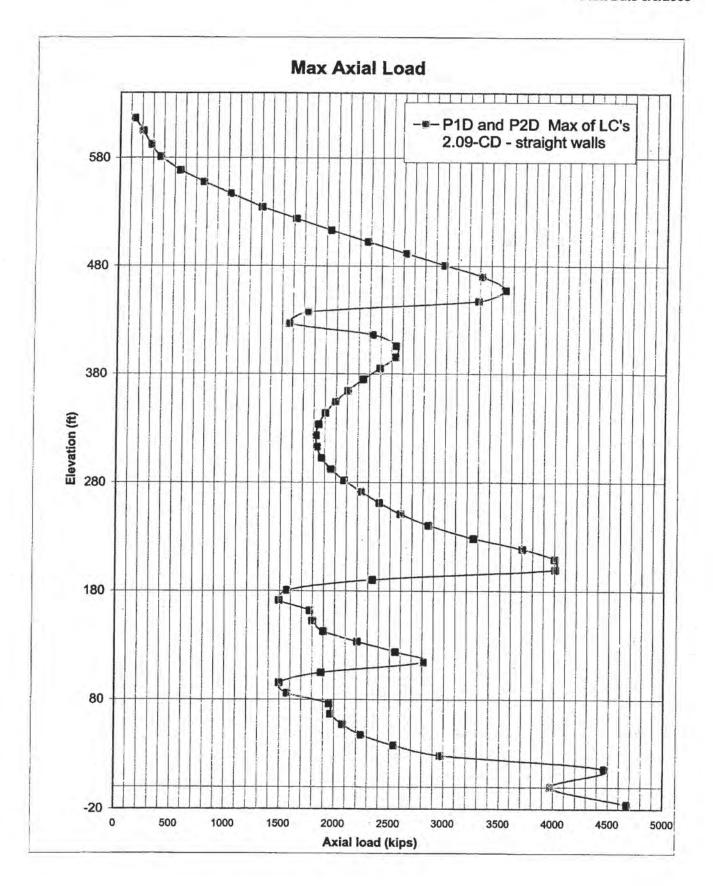


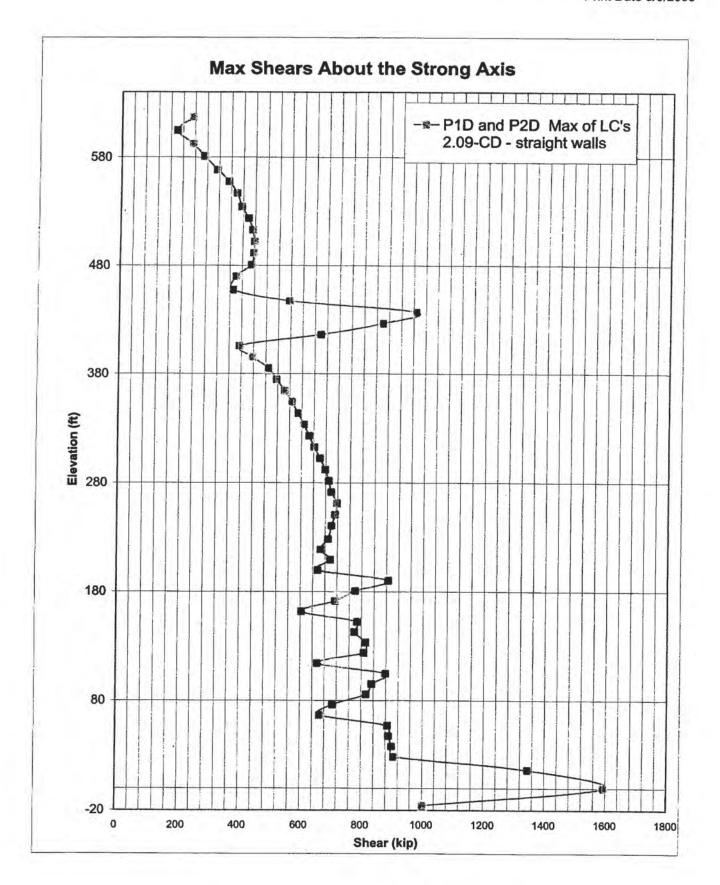


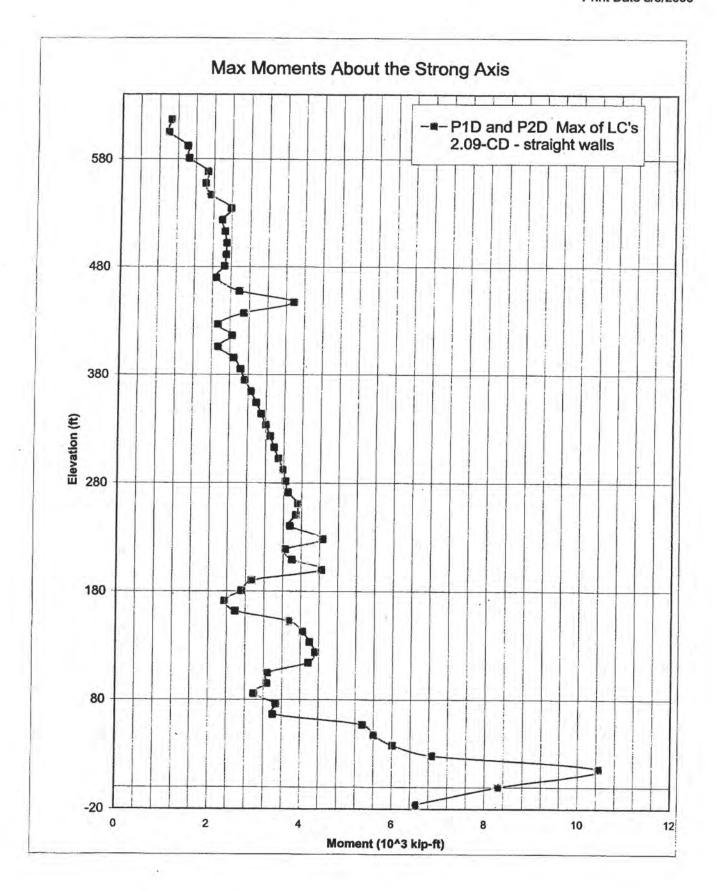












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	61	PIER2	E30X100Y5 MAX	Тор		191	76		-			553			
	60	PIER2	E30X100Y5 MAX	Top		185	90					1101			
	59	PIER1	E30X100Y5 MAX	Top		184	155					2031	1273		
	58	PIER1	E30X100Y5 MAX	Top	904	292	226	5721	6652	10644		2756			
	57	PIER1	E30X100Y5 MAX	Top	1195	400	285	6753	7532	12720		3525			
	56	PIER2	E30X100Y5 MAX	Top	1516	476	343	7653	7797	16465		3460	2148	4976	
	55	PIER2	E30X100Y5 MAX	Top	1862	550	389	8503	8176	19804		4162	2580	6024	
	54	PIER2	E30X100Y5 MAX	Top	2237	626	429	9175	8426	23439		4929	3053	- TT-	
	53	PIER2	E30X100Y5 MAX	Top	2639	671	467	10002	8038	27850		5630	3485	8269	
	52	PIER2	E30X100Y5 MAX	Top	3058	717	493	10532	8114	31962		6333	3916		
		PIER2	E30X100Y5 MAX	Top	3497	754	525	10902	8186	36531		7036	4348	10533	851
	50	PIER2	E30X100Y5 MAX	Top	3951	789	539	11146	8190	41490		7740	4780	11691	828
		PIER2	E30X100Y5 MAX	Top		813	583	11197	8239	46835		8443	5211	12863	791
		PIER2	E30X100Y5 MAX	Top		848	580	10808	8226	52653		9147	5643	14046	744
		PIER2	E30X100Y5 MAX	Top	5393	590	628	10518	8347	58811		9966	6141	15358	748
		PIER2	E30X100Y5 MAX	Top	5847	560	751	12346	8993	61295		10678	6578	16525	731
		PIER2	E30X100Y5 MAX	Top		723	1010	12982	12220	36494		11395	7018	16314	2100
		PIER2	E30X100Y5 MAX	Top	5078	898	896	12043	9789	39113		12113	7459	17191	2380
		PIER2	E30X100Y5 MAX	Top		603		12324		35623		12825	7896	17454	3266
	117	PIER2	E30X100Y5 MAX	Top	3609	527			11098			13513	8317	17121	4709
	4.4	PIER2	E30X100Y5 MAX	Тор	3962	971	658	12636	11372	48172		14201	8739	18163	4777
		PIER2	E30X100Y5 MAX	Top	4367	922			11253			14889	9161	19255	4794
		PIER2	E30X100Y5 MAX	Top	4790	932	638	14999	11289	41221		15577	9583	20366	4793
		PIER2	E30X100Y5 MAX	Top	5226	942			11398	ステマクス		16265	10005	21491	4778
		PIER2	E30X100Y5 MAX	Top	5674	956	646	16211	11511	36488		16953	10427	22627	4752
	270	PIER2	E30X100Y5 MAX	Top	6132	969			11631			17641	10848	23773	4717
		PIER2	E30X100Y5 MAX	Top	6597	984	657	17090	11760	33581		18329	11270	24926	4674
	,	PIER2	E30X100Y5 MAX	Top	7068	1001			11892			19017	11692	26085	4624
		PIER2	E30X100Y5 MAX	Top	7545	1021			12044			19705	12114	27250	4569
			E30X100Y5 MAX	Top	8027	1051			12142			20393	12536	28420	4509
			E30X100Y5 MAX	Top	8509	1069			12342			21081	12958	29589	4449
	22.00		E30X100Y5 MAX	Top	8991	1065			12772			21769	13379	30760	4389
		PIER2	E30X100Y5 MAX	Top	9472	1058	586	18161	13472	36319		22457	13801	31929	4329
		PIER2	E30X100Y5 MAX	Top	9949	1118			14687			23145	14223	33094	4274
			E30X100Y5 MAX		10423	1203			18386			23833	14645	34256	4222
			E30X100Y5 MAX		10919	1407			18814	With COS E		24520	15066	35439	4147
	-		E30X100Y5 MAX		11416	1505			17820			25279	15533	36695	4116
			E30X100Y5 MAX		11905	1585			16853			25984	15959	37889	4054
			E30X100Y5 MAX		12383	1128			16782	2 1 2 2 2 2		26640	16360	39023	3977
			E30X100Y5 MAX		12825				13389			27319	16776	40145	3951
			E30X100Y5 MAX						16520			28005	17196	40183	5018
			E30X100Y5 MAX					2.000	14618			28690	17616	41029	5277
			E30X100Y5 MAX						13040			29375	18037		5305
			E30X100Y5 MAX	Top	11864	1008	900	11549	11882	40812		30054	18453		12.00
			E30X100Y5 MAX	Top	10355	1200	997	10432	131/8	28464		30710	18854		
			E30X100Y5 MAX	Top	10744	2016	1000	19125	14515	21286		31366	19256		
			E30X100Y5 MAX	Top	11168	2016	1002	19439	14249	23695		32022	72 O V 75 V 1	43190	8489
			E30X100Y5 MAX E30X100Y5 MAX		11615							32677		44292	
					12053							33357			
			E30X100Y5 MAX		12100							34042	20895		
		and the same of	E30X100Y5 MAX		12389							34727	21315		
			E30X100Y5 MAX		12773							35413		48186	
			E30X100Y5 MAX		11939							36092		48032	
			E30X100Y5 MAX	Top	10493	1341	10/2	15/94	7271	27726		36748	22553		
			E30X100Y5 MAX	Top	10805	2288	1268	22780	16015	28166		37404		48208	
			E30X100Y5 MAX	Tob	11149	2326	1282	25195	20092	33226		38059	23356		
			E30X100Y5 MAX		11478							38715	23757		
		Accessed to the second	E30X100Y5 MAX		11772							39371		51143	
			E30X100Y5 MAX		12012							40126	24623		
22.5			E30X100Y5 MAX		14842							41097	25217		
ROUNI	DP	IER1	E30X100Y5 MAX	Bottc	16335	1952	1660	16459	38568	88297		42123	25845	58457	9510

Level	Pier ID	Load Combo	Loc	(kips)	V2 (kips)		(kip-ft)	1. 1	(kip-ft)	1.42DL+0.5LL (kips)	0.9*DL (kips)	Pucomp (kips)	P <sub>utens</sub> (kips)
61 1	PIER2	E30X100Y5 MAX	Top	190	191	76		1964		553			
	PIER1	E30X100Y5 MAX	Тор	407	179	90		3745		1101			
	PIER1	E30X100Y5 MAX	Тор	642	184	155	4634	5519		2031			
	PIER2	E30X100Y5 MAX	Тор	904	286	225	5502	6690		2756			
	PIER2	E30X100Y5 MAX	Тор	1195	388	289	6490	7536		3525			
	PIER2	E30X100Y5 MAX	Тор	1516	476	343	7653	7797		3460		4976	
	PIER2	E30X100Y5 MAX	Тор	1862	550	389	8503						
0.7	PIER2	E30X100Y5 MAX						8176		4162			
			Тор	2237	626	429	9175	8426		4929		7166	Service of the Control
1000	PIER2	E30X100Y5 MAX	Тор	2639	671	467	10002	8038		5630		8269	V 10.00
15.00	PIER2	E30X100Y5 MAX	Тор	3058	717	493	10532	8114		6333		9391	
	PIER2	E30X100Y5 MAX	Тор	3497	754	525	10902	8186	100000	7036		10533	77.0
	PIER2	E30X100Y5 MAX	Тор	3951	789	539	11146	8190		7740		11691	
	PIER2	E30X100Y5 MAX	Bottom		813	583	11253	8762		8443		12863	
	PIER2	E30X100Y5 MAX	Bottom		848	580	10868	9403	57071	9147	5643	14046	744
	PIER2	E30X100Y5 MAX	Bottom		590	628	10596	11042		9966	6141	15358	748
46 F	PIER2	E30X100Y5 MAX	Top	5847	560	751	12346	8993	61295	10678	6578	16525	731
45 F	PIER2	E30X100Y5 MAX	Top	4918	723	1010	12982	12220	36494	11395	7018	16314	2100
44 F	PIER2	E30X100Y5 MAX	Тор	5078	898	896	12043	9789	39113	12113	7459	17191	2380
43 F	IER2	E30X100Y5 MAX	Top	4630	603	802	12324	9329	35623	12825	7896	17454	3266
42 F	PIER2	E30X100Y5 MAX	Top	3609	527	622	11801	11098	47971	13513	8317	17121	4709
41 F	IER2	E30X100Y5 MAX	Top	3962	971	658	12636	11372	48172	14201	8739	18163	
	PIER1	E30X100Y5 MAX	Bottom	4347	939	645	14450	11449	36526	14889	9161	19235	
	IER1	E30X100Y5 MAX	Bottom	4762	943	629	15201	11824	34588	15577	9583	20338	
	IER1	E30X100Y5 MAX	Bottom	5189	948	200	15837	12195	33239	16265	10005	21453	
	IER1	E30X100Y5 MAX	Bottom	5625	955	636	16346	12518	32371	16953	10427	22578	
	IER1	E30X100Y5 MAX	Bottom	6069	962	640	16767	12822	31917	17641	10848	23710	
	IER1	E30X100Y5 MAX	Bottom	6519	971	642	17128	13093				the second second	
100									31840	18329	11270	24847	4752
	IER1	E30X100Y5 MAX	Bottom	6972	981	645	17433	13360	32133	19017	11692	25989	4720
	IER1	E30X100Y5 MAX	Bottom	7428	994	645	17744	13559	32564	19705	12114	27133	4686
	IER1	E30X100Y5 MAX	Bottom	7886	1019	641	17589	13713	33253	20393	12536	28279	4650
	IER2	E30X100Y5 MAX	Bottom	8509	1069	661	17989	14111	32172	21081	12958	29589	4449
	IER2	E30X100Y5 MAX	Bottom	8991	1065	649	18051	14635	34158	21769	13379	30760	4389
29 P		E30X100Y5 MAX	Bottom	9472	1058	586	18224	15216	37010	22457	13801	31929	4329
28 P	IER1	E30X100Y5 MAX	Bottom	9678	1173	448	18998	17166	44013	23145	14223	32823	4545
27 P	IER2	E30X100Y5 MAX	Top	10423	1203	710	18978	18386	43307	23833	14645	34256	4222
26 P	IER2	E30X100Y5 MAX	Top	10919	1407	933	20152	18814	45295	24520	15066	35439	4147
25 P	IER1	E30X100Y5 MAX	Top	11105	1440	944	19664	17843	47934	25279	15533	36384	4427
24 P	IER2	E30X100Y5 MAX	Bottom	11905	1585	918	18739	17865	64883	25984	15959	37889	4054
23 P	IER1	E30X100Y5 MAX	Bottom	12068	1110	979	16293	18718	67846	26640	16360	38708	4291
22 P	IER2	E30X100Y5 MAX	Top	12825	687	1050	12051	13389	65461	27319	16776	40145	3951
21 P		E30X100Y5 MAX	Top	12179	1743	1309	10578	16520	43043	28005	17196	40183	5018
20 P		E30X100Y5 MAX	Top	12339	1875	1142	11851	14618	44152	28690	17616	41029	5277
19 P		E30X100Y5 MAX	Тор	12731	2016	1024	10501	13040	53333	29375	18037	42106	5305
12.47	San March 1971	E30X100Y5 MAX	Тор	11864			11549	11882	40812	30054	18453	41919	
		E30X100Y5 MAX	Bottom				16529	15318	20920				
		E30X100Y5 MAX	Bottom		1981		19717	15819		30710	18854	40810	8755
		THE RESERVE AND ADDRESS OF THE PARTY OF THE							24990	31366	19256	41875	
		E30X100Y5 MAX	Bottom				20046	17209	35766	32022	19657	42962	
		E30X100Y5 MAX		11400			17772	18557	46970	32677	20058	44077	10000
		E30X100Y5 MAX		11827				10357	41073	33357	20475	45184	
		E30X100Y5 MAX	Тор	11884			9728	12333	32181	34042	20895	45926	9011
11 P		E30X100Y5 MAX		12182			9629	14024	60657	34727	21315	46909	9133
10 P	IER1	E30X100Y5 MAX	Bottom	12584	2674	1287	10790	16890	79932	35413	21735	47997	9151
9 P	IER1	E30X100Y5 MAX	Bottom	10509	952	1213	11831	14323	29539	36092	22151	46601	
8 PI	ER1	E30X100Y5 MAX	Bottom	10308				21713	29730	36748	22553		
		E30X100Y5 MAX	Bottom		2336			26505	32728	37404	22954		
		E30X100Y5 MAX		11024				31808	43003	38059	23356		
		E30X100Y5 MAX	Bottom		2434				59608	38715	23757		
		E30X100Y5 MAX											
				11694					79348	39371	24158		
		E30X100Y5 MAX	ALEXANDER OF THE PARTY OF THE P	11946				56701		40126	24623		
		E30X100Y5 MAX	Тор	12820					71611	41097	25217		
12 25 31 4) F31	ER1	E30X100Y5 MAX	Hollom	16335	1952	1660	16459	38568	88297	42123	25845	58457	9510

Level	Pier ID	Load Combo	Loc	P	(a) 4 1 1	1200	6.600 4.0	Section 12.1		1.42DL+0.	5LL	0.9*DL	Pucomp	Putens
- 04	DIEDA	F400VF00V LAAV	Datte							(kips)		(kips)	(kips)	(kips)
100	PIER1	E100X530Y MAX	Bottom	75	243	47					553			
	PIER1	E100X530Y MAX E100X530Y MAX	Тор	160 251	327 496	56 66					1101			532
	PIER2	E100X530Y MAX	Bottom	336	753		10507				2031	1273	2282	
	PIER2	E100X530Y MAX	Bottom	441	990		12216				2756		3092	
	PIER2	E100X530Y MAX	Bottom	556	1168		14002				3525 3460		3966 4016	1748
	PIER2	E100X530Y MAX	Bottom	679	1320		15246				4162		4841	1900
	PIER2	E100X530Y MAX	Bottom	812			16131				4929	3053	5741	2241
	PIER2	E100X530Y MAX	Bottom	953	1533	-0.75	17088				5630	3485	6583	2531
	PIER2	E100X530Y MAX	Bottom	1101	1621		17567				6333	3916	7434	2815
	PIER2	E100X530Y MAX	Bottom	1254	1678		17656		99044		7036	4348	8291	3093
7.0	PIER2	E100X530Y MAX	Bottom		1729		17330		113832		7740	4780	9153	3366
17.7	PIER2	E100X530Y MAX	Bottom	1577	1762		16401	2404	129284		8443	5211	10020	3635
	PIER1	E100X530Y MAX	Bottom	1768	1850		15107		145326		9147	5643	10915	3875
	PIER1	E100X530Y MAX	Bottom	1936	999		11665		153167		9966	6141	11902	4205
	PIER1	E100X530Y MAX	Тор	2090	934		16351	4.355	153893		10678	6578	12768	4487
27	PIER1	E100X530Y MAX	Bottom	1698	1683		16387	7171	88487		11395	7018	13093	5320
	PIER1	E100X530Y MAX	Bottom	1843	1672		16529	7336	98496		12113	7459	13956	5616
	PIER1	E100X530Y MAX	Bottom	1381	1479		16612	6827	91993		12825	7896	14205	6515
	PIER1	E100X530Y MAX	Top	1368	748		13016		113899		13513	8317	14881	6949
	PIER1	E100X530Y MAX	Top	1487	1985		15574		116293		14201	8739	15688	7252
	PIER1	E100X530Y MAX	Top	1622	1862		19601		105934		14889	9161	16510	7539
	PIER1	E100X530Y MAX	Top	1761	1879		21623	7037	97638		15577	9583	17338	7822
38	PIER1	E100X530Y MAX	Top	1904	1890	219	23195	6986	90363		16265	10005	18169	8101
	PIER1	E100X530Y MAX	Тор	2049	1913		24405	6857	84135		16953	10427	19002	8377
36	PIER1	E100X530Y MAX	Top	2196	1935		25391	6685	78997		17641	10848	19837	8653
35	PIER1	E100X530Y MAX	Top	2343	1962		26232	6515	75061		18329	11270	20671	8928
	PIER1	E100X530Y MAX	Top	2489	1994		26957	6391	72433		19017	11692	21506	9203
	PIER1	E100X530Y MAX	Top	2635	2029	Action I	27648	6353	71244		19705	12114	22340	9479
32	PIER1	E100X530Y MAX	Top	2780	2076	241	27817	6406	71127		20393	12536	23173	9756
31	PIER1	E100X530Y MAX	Bottom	2920	2131		28529	7408	73323		21081	12958	24001	
30	PIER1	E100X530Y MAX	Bottom	3056	2155		28862	7916	78731		21769	13379	24825	
29	PIER1	E100X530Y MAX	Bottom	3185	2191	265	29081	8802	85691		22457	13801	25642	
28	PIER1	E100X530Y MAX	Bottom	3330	2256	304	29376		94646		23145	14223	26475	
27	PIER1	E100X530Y MAX	Bottom	3488	2396	254	30084	11196	102985		23833	14645	27321	0.2.2.402
26	PIER1	E100X530Y MAX	Bottom	3640	2581				116607		24520	15066	28160	
25	PIER1	E100X530Y MAX	Bottom	3789	2732				136928		25279	15533	29068	
24	PIER1	E100X530Y MAX	Bottom	3932	2927	372	26856	9556	154224	- 1	25984	15959	29916	
23	PIER2	E100X530Y MAX	Bottom	4413	1935	365	21091	9323	164334	. 3	26640	16360	31053	
22	PIER2	E100X530Y MAX	Top	4556	990	369	16500	8464	159459	3	27319	16776	31875	
21	PIER1	E100X530Y MAX	Bottom	3970	3515	450	14752	7935	115103		28005	17196	31974	13226
20	PIER1	E100X530Y MAX	Bottom	4072	3424	394	17058	7917	132188		28690	17616	32762	
19	PIER1	E100X530Y MAX	Bottom	4180	3491	382	13878	7486	149905	102	29375	18037	33555	13857
18	PIER1	E100X530Y MAX	Top	3914	1933		15774	7503	99172		30054	18453	33969	14539
17	PIER1	E100X530Y MAX	Тор	3459	2357	389	22180	7851	58647	1.3	30710	18854	34170	
16	PIER1	E100X530Y MAX	Bottom	3563	3760	415	29714	8695	52049	1.3	31366	19256	34929	15693
15	PIER2	E100X530Y MAX	Bottom	4171	3792	435	29525	8318	73697	3	32022	19657	36192	15486
14	PIER2	E100X530Y.MAX	Bottom	4321	2924	386	25362	8528	95486	3	32677	20058	36998	15738
13	PIER2	E100X530Y MAX	Тор		2312	412	14932	6949	91958	3	33357	20475	37827	16005
12	PIER2	E100X530Y MAX	Bottom	4560	4629	490	12640	6771	101915	3	34042	20895	38603	16334
11	PIER2	E100X530Y MAX	Bottom	4685	4336	469	10462	7714	126461	3	34727	21315	39412	16630
10	PIER2	E100X530Y MAX	Bottom	4816	4505	470	13276	8829	155693	3	35413	21735	40228	16919
9	PIER2	E100X530Y MAX	Top	4582	1822	461	15798	6233	94807	3	36092		40674	17570
8	PIER1	E100X530Y MAX	Top	3573	2435	438	22478	6205	77813	3	36748		40321	
71	PIER1	E100X530Y MAX	Top	3668	4515	517	36567	9463	65675	3	37404		41072	
61	PIER2	E100X530Y MAX	Bottom	4391	4494	507	38313	14219	70678		88059		42451	
5 1	PIER2	E100X530Y MAX	Bottom	4500	4925	525	39525	16738	102883		8715		43215	
		E100X530Y MAX	Bottom	3978	4838	613	41946	22307	130903		39371		43349	
31	PIER1	E100X530Y MAX	Bottom		4832	544	43351	25264	182035		0126		44180	
21	PIER1	E100X530Y MAX	Bottom	4995	3678			12105			1097		46092	
POLL	PIER1	E100X530Y MAX	Bottom	5518	4056			14369			2123		47641	

4069-20050509-2.09-CD-Wall Design - Pier1 and Pier2 SummaryM3+Grav

8.1.1-27

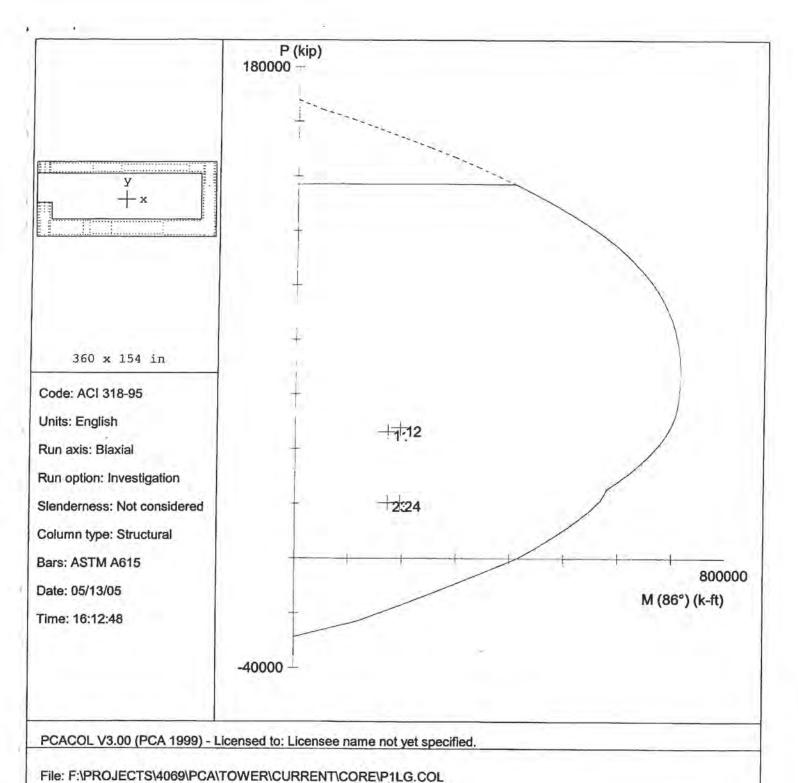
## MAY OF LEVEL BI -> LEVEL 4

Print Date: 5/13/2005

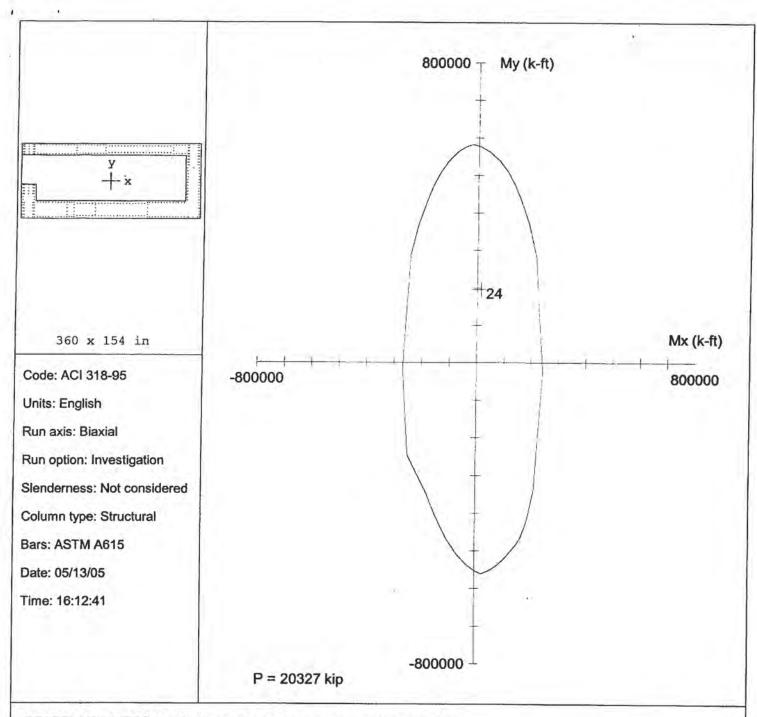
PCA INPUT PILG FILE

			1000	1.42DL+0						label	pca label
	Putens	Pucomp	0.9*DL	.5LL	МЗ	M2	P	Load Combo	Level Pier ID	sion)	(comp.)
	12386		24158	39371	64140	32509	11772	E30X100Y5 MAX	4 PIER2	13	1
*****	12611	52138	24623	40126	78497	41314	12012	E30X100Y5 MAX	3 PIER2	14	2
MAX P	10375	55938	25217	41097	78893	28261	14842	E30X100Y5 MAX	2 PIER1	15	3
	9510	58457	25845	42123	88297	38568	16335	E30X100Y5 MAX	GROUND PIER1	16	4
	12464	51065	24158	39371	79348	46002	11694	E30X100Y5 MAX	4 PIER1	17	5
	12677	52072	24623	40126	101248	56701	11946	E30X100Y5 MAX	3 PIER1	18	6
MAX M2	12396	53917	25217	41097	71611	45223	12820	E30X100Y5 MAX	2 PIER1	19	7
	9510	58457	25845	42123	88297	38568	16335	E30X100Y5 MAX	GROUND PIER1	20	8
	20181	43349	24158	39371	130903	22307	3978	E100X530Y MAX	4 PIER1	21	9
	20569	44180	24623	40126	182035	25264	4054	E100X530Y MAX	3 PIER1	22	10
МАХМЗ		46092	25217	41097	172685	12105	4995	E100X530Y MAX	2 PIER1	23	11
	C # 3 40 - 17 % No.	47641	25845	42123	195399	14369	5518	E100X530Y MAX	GROUND PIER1	(24)	12

PCA LABEL 24 GOVERNS DESIGN)



#### Project: Column: Engineer: fc = 10 ksi fy = 75 ksiAg = 25446 in^2 366 bars Ec = 5000 ksi Es = 29000 ksi As = 426.48 in^2 Rho = 1.68% fc = 8.5 ksi e\_rup = Infinity Xo = 11.63 inIx = 8.45602e+007 in^4 e\_u = 0.003 in/in Yo = -8.27 inly = 3.34865e+008 in^4 Clear cover = N/A 8.1.1-29 Beta1 = 0.65 Clear spacing = 0.97 in



### PCACOL V3.00 (PCA 1999) - Licensed to: Licensee name not yet specified.

File: F:\PROJECTS\4069\PCA\TOWER\CURRENT\CORE\P1LG.COL

Project:

Column:

Engineer:

fc = 10 ksi

fy = 75 ksi

Ag = 25446 in^2

366 bars

Ec = 5000 ksi

Es = 29000 ksi

As = 426.48 in^2

Rho = 1.68%

fc = 8.5 ksi

e\_rup = Infinity

Xo = 11.63 in

lx = 8.45602e+007 in^4

e\_u = 0.003 in/in

Yo = -8.27 in

ly = 3.34865e+008 in^4

Beta1 = 0.65

Clear spacing = 0.97 in

Clear cover = N/A 8.1.1-30

Confinement Tied

phi(a) = 0.8 phi(b) = 0.9 phi(c) = 0.7

#### General Information:

File Name: F:\PROJECTS\4069\PCA\TOWER\CURRENT\CORE\P1LG.COL

Project:

Column:

Code: ACI 318-95 Engineer: Units: English

Run Option: Investigation

Run Axis: Biaxial

Slenderness: Not considered Column Type: Structural

Material Properties:

f'c = 10 ksi= 5000 ksi = 8.5 ksi Ec

fc

Ultimate strain = 0.003 in/in

Beta1 = 0.65

fy = 75 ksiEs = 29000 ksi

Rupture strain = Infinity

### Section:

------

No.	Y (in)	Y (in)	No.	X (in)	Y (in)	No.	X (in)	Y (in)
1 4	-180.0 180.0	77.0 77.0	2	-180.0	-77.0	3	180.0	-77.0
No.	Y (in)	Y (in)	No.	X (in)	Y (in)	No.	X (in)	Y (in)
1 4	-179.9 -150.0	53.0 -41.0	2 5	-179.9 150.0	-7.0 -41.0	3 6	-150.0 150.0	-7.0 53.0

Gross section area, Ag = 25446 in^2

Ix = 8.45602e+007 in^4 Xo = 11.6293 in

 $Iy = 3.34865e+008 in^4$ 

Yo = -8.27093 in

#### Reinforcement:

Rebar Database: ASTM A615

S	ize	Diam (in)	Area (in^2	)	Size	Diam (	in)	Area	(in^2)	S	ze	Diam	(in)	Area	(in^2)
-				-											
#	3	0.38	0.1	1	4 4	0	.50		0.20	#	5		0.63		0.31
#	6	0.75	0.4	4	# 7	0	.88		0.60	#	8		1.00		0.79
#	9	1.13	1.0	0	# 10	1	.27		1.27	#	11		1.41		1.56
#	14	1.69	2.2	5 1	18	2	.26		4.00						

Confinement: Tied; #3 ties with #10 bars, #4 with larger bars. phi(a) = 0.8, phi(b) = 0.9, phi(c) = 0.7

Pattern: Irregular

Total steel area, As = 426.48 in^2 at 1.68%

Area in^2	X (in)	Y (in)	Area in^2	X (in)	Y (in)	Area in^2	X (in)	Y (in)
1.00	-93.2	-73.8	1.00	-93.2	-44.1	1.00	-99.2	-73.8
1.00	-99.2	-44.1	1.00	-105.2	-73.8	1.00	-105.2	-44.1
1.00	-110.5	-73.8	1.00	-110.5	-44.1	1.00	-116.5	-73.8
1.00	-116.5	-44.1	1.00	-124.5	-73.8	1.00	-124.5	-44.1
1.00	-130.5	-73.8	1.00	-130.5	-44.1	1.00	-135.2	-73.8
1.00	-135.2	-44.1	1.00	-141.2	-73.8	1.00	-141.2	-44.1
1.00	-147.2	-73.8	1.00	-147.2	-44.1	1.00	67.3	-71.8
1.00	67.3	-46.1	1.00	61.6	-71.8	1.00	61.6	-46.1
1.00	55.6	-71.8	1.00	55.6	-46.1	1.00	49.6	-71.8
1.00	49.6	-46.1	1.00	43.6	-71.8	1.00	43.6	-46.1
1.00	37.6	-71.8	1.00	37.6	-46.1	1.00	31.0	-71.8
1.00	31.0	-46.1	1.00	25.0	-71.8	1.00	25.0	-46.1
1.00	19.0	-71.8	1.00	19.0	-46.1	1.00	13.0	-71.8
1.00	13.0	-46.1	1.00	6.3	-71.8	1.00	6.3	-46.1

		22 2	3-15-5						
1.00	0.3	-71.8	1.00	0.3	-46.1	1.00	-5.7	-71.8	
1.00	-5.7	-46.1	1.00	-12.1	-71.8				
				-12.1	-/1.0				
1.00	-18.1	-71.8	1.00	-18.1	-46.1	1.00	-24.1	-71.8	
1.00	-24.1	-46.1	1.00	176.7	21.1				
				1/0./	21.1	1.00	153.6	21.1	
1.00		15.1	1.00	154.2	15.1 2.8 -2.9	1.00	176.9	9.1	
1.00	154.2	9.1	1.00	176.7	2 0				
	134.2	3.1			2.0	1.00	154.0		
1.00	176.7	-2.9	1.00	154.0	-2.9	1.00	176.4	-8.9	
1.00	153.6	-8.9	1.00	176.4					
						1.00	153.6		
1.00	176.4	-20.9	1.00	153.6	-20.9	1.00	176.4	-26.9	
1.00	153.6	-26.9	1.00						
			1.00	1/0.4		1.00	153.6		
1.00	176.4	-38.9	1.00	153.6	~38.9	1.00	-4.4	58.2	
1.00	-4.4	71.9	1.00	1.6					
						1.00	1.6		
1.00	7.6	58.2	1.00	7.6	71.9	1.00	13.6	58.2	
1.00	13.6	71.9	1.00					71 0	
						1.00	19.6	71.9	
1.00	26.1	58.2	1.00	26.1	71.9	1.00	32.1	58.2	
1.00	32.1	71.9	1.00	38.6	58.2	1.00			
		50.0						71.9	
1.00	44.6	58.2	1.00	44.6	71.9	1.00	50.6	58.2	
1.00	50.6	71.9	1.00	57.1	58.2	1.00			
1.00	63.6	EO 0			50.2		31.1		
		58.2	1.00	63.6	71.9	1.00	69.6	58.2	
1.00	69.6	71.9	1.00	75.6	58.2	1.00	75.6	71.9	
1.00	81.6	58.2		01 6					
			1.00		71.9	1.00	87.6	58.2	
1.00	87.6	71.9	1.00	94.1	58.2	1.00	94.1	71.9	
1.00	100.1	58.2							
			1.00	100.1	71.9	1.00		58.2	
1.00	106.1	71.9	1.00	112.6	58.2	1.00	112.6	71.9	
1.00	118.6	58.2		118.6	71.9				
						1.00		55.8	
1.00	-111.1	74.1	1.00	-117.6	55.8	1.00	-117.7	74.1	
1.00	-123.6	55.8	1.00		74.1				
						1.00	-129.6	55.8	
1.00	-129.7	74.1	1.00	-135.6	55.8	1.00	-135.7	74.1	
1.00	-141.6	55.8	1.00	-141.7	74.1	1.00			
		74.1	1.00					55.8	
1.00	-147.7	74.1	1.00	-104.6	55.8	1.00	-104.7	74.1	
1.00	-98.6	55.8	1.00	-98.7	74.1	1.00	-02 6		
1.00	-92.7		1 00	20.7		1.00	-52.0	55.8	
1.00	-32.1	74.1		-86.6	55.8	1.00	-86.7	74.1	
1.00	-80.6	55.8	1.00	-80.7	74.1		-74.6	55.8	
	-74.7			1.05 0					
			1.27	-165.0	55.2	1.27	-165.0	59.2	
1.27	-153.7	59.2	1.27	-156.0	59.2	1.27	-173.9	59.2	
1.27	-176.1	59.2							
				-153.7	70.5	1.27	-156.0	70.5	
1.27	-165.0	70.5	1.27	-173.9	70.5	1.27	-176.1	70.5	
1.27	-153.7	65.1	1.27						
				-156.0	65.1	1.27	-165.0	65.1	
1.27	-173.9	65.1	1.27	-176.1	65.1	1.27	-153.7	55.1	
1.27	-156.0	55.1		-173.9					
1 07		33.1			55.1		-176.1		
1.27	-153.7	74.1	1.27	-156.0	74.1	1.27	-165.0	74.1	
1.27	-173.9	74.1	1.27	-176.1	74.1	1.27			
							-68.6	56.3	
1.27	-68.7	73.6	1.27	-62.8	56.3	1.27	-62.9	73.6	
1.27	-57.0	56.3	1.27	-57.1	73.6	1.27			
1.27	-51.3	72 6						56.3	
			1.27		56.3	1.27	-45.5	73.6	
1.27	-39.7	56.3	1.27	-39.7	73.6	1.27	-33.9	56.3	
1.27	-33.9	73.6	1.27		56.3				
				-28.1	56.3	1.27	-28.2	73.6	
1.27	-22.3	56.3	1.27	-22.4	73 6	1.27	-16.1	56.3	
1.27	-16.2	73.6	1.27		62.1				
						1.27	-11.3	67.8	
1.27	-11.3		1.27	-11.4	73.7	1.27	-68.7	67.8	
1.27	-68.7	62.5	1.27	176.7	27.1	1.27			
						1.21	153.6	27.1	
1.27		27.1	1.27	153.7	33.1	1.27	156.0	33.1	
1.27	173.9	33.1	1.27	176.5	33 1	1.27	153.7		
	150 0	20.1		170.0	33.1	1.21		39.1	
1.27	156.0	39,1	1.27	173.9	39.1	1.27	176.5	39.1	
1.27	153.7	39.1 45.1	1.27		45.1	1.27			
1.27							173.9	45.1	
	176.5	45.1	1.27	153.7	51.1	1.27	156.0	51.1	
1.27	173.9	51.1 56.4	1.27	176.5	51.1	1.27	153.7		
1.27	156 0	56 4				1.2/		56.4	
	156.0	30.4	1.27	173.9	56.4	1.27	176.5	56.4	
1.27	153.7	62.4	1.27	156.0		1.27	173.9		
1.27						1.21	173.9	62.4	
	176.5	62.4	1.27	153.7	68.4	1.27	156.0	68.4	
1.27	173.9	68.4	1.27	176.5	68.4	1.27		73.9	
1.27	156.0	73.9							
	20010	13.3	1.27	165.0	13.9	1.27	173.9	73.6	
1.27	176.5	73.6	1.27	147.4	56.2	1.27	147.6	73.6	
1.27	141.6	56.2							
			1.27	141.8	73.6	1.27	135.8	56.2	
1.27	136.0	73.6	1.27	130.0	56.2	1.27	- 15 at a 7 18	73.6	
1.27	124.4	67.8	1.27	124.4	62 1	1.27	104 4		
2.27	*****	01.0	1.21	174.4	62.1	1.21	124.4	56.2	

124.6 73.3 73.3 83.3 89.3 101.3 107.3 118.6 124.6 135.2 141.2	-44.1 -73.8 -44.1 -73.8 -44.1 -73.8 -44.1	1.27 1.27 1.27 1.27 1.27 1.27 1.27	101.3 112.6	-69.4 -48.7 -73.8 -44.1 -73.8 -44.1	1.27 1.27 1.27 1.27 1.27 1.27	73.3 73.3 77.3 89.3 95.3	-62.7 -73.8 -44.1 -73.8 -44.1
73.3 83.3 89.3 101.3 107.3 118.6 124.6 135.2	-44.1 -73.8 -44.1 -73.8 -44.1 -73.8 -44.1	1.27 1.27 1.27 1.27 1.27 1.27	77.3 83.3 95.3 101.3 112.6	-48.7 -73.8 -44.1 -73.8	1.27 1.27 1.27 1.27	73.3 77.3 89.3	-73.8 -44.1 -73.8
83.3 89.3 101.3 107.3 118.6 124.6 135.2	-73.8 -44.1 -73.8 -44.1 -73.8 -44.1	1.27 1.27 1.27 1.27 1.27	83.3 95.3 101.3 112.6	-73.8 -44.1 -73.8	1.27 1.27 1.27	77.3 89.3	-44.1 -73.8
89.3 101.3 107.3 118.6 124.6 135.2	-44.1 -73.8 -44.1 -73.8 -44.1	1.27 1.27 1.27 1.27	83.3 95.3 101.3 112.6	-44.1 -73.8	1.27 1.27	89.3	-73.8
101.3 107.3 118.6 124.6 135.2	-73.8 -44.1 -73.8 -44.1	1.27 1.27 1.27	101.3 112.6		1.27		
107.3 118.6 124.6 135.2	-44.1 -73.8 -44.1	1.27 1.27 1.27	101.3 112.6			20.0	
118.6 124.6 135.2	-44.1 -73.8 -44.1	1.27 1.27	112.6			107.3	-73.8
124.6 135.2	-44.1	1.27		-73.8	1.27	112.6	-44.1
135.2	-44.1		118.6	-44.1	1.27	124.6	-73.8
		1.27		-73.8	1.27	130.6	-44.1
1/1 2	-73.8	1.27		-44.1	1.27	141.2	-73.8
141.2	-44.1	1.27		-73.9	1.27	156.3	-73.9
165.2	-73.9						-73.9
176.8	-69.1	1.27					-56.0
176.8	-49.3						-73.8
147.2	-44.1						-44.1
-77.9	-73.8	1.27					-69.4
-87.2	-62.7						-48.7
-73.3	-69.4						-55.3
-73.3	-48.7						-44.1
-87.2	-73.8	1.27					-69.4
-30.8	-62.7	1.27		-55.3			-48.7
-30.9	-71.8		-30.9	-46.1			-71.8
-36.9	-46.1	1.27	-42.9	-71.8			-46.1
-49.2	-71.8	1.27					-71.8
-55.2	-46.1	1.27	-61.2				-46.1
-67.2	-71.8	1.27	-67.2	-46.1			-69.1
173.6	-62.2	1.27	-173.6	-56.0			-49.3
173.5	-44.9	1.27	-156.2				-62.2
156.2	-56.0	1.27					-44.9
153.1	-69.1	1.27	-153.1				-56.0
153.1	-49.3	1.27	-153.0				-73.9
156.2	-73.9	1.27	-165.1				-73.9
176.7	-73.9	1.27	-176.7				-62.2
176.7	-56.0	1.27					-44.9
156.0	-26.9						-26.9
156.0	-20.9						-20.9
156.0	-14.9						-14.9
156.0	-8.9						-8.9
176.4	-8.9	1.27					-14.9
153.6	-14.9		-176.4				-20.9
176.4	-26.9	1.27					-32.9
153.6	-32.9	1.27	-176.4				-38.9
	176.8 176.8 176.8 147.2 -77.9 -87.2 -73.3 -73.3 -87.2 -30.8 -30.9 -49.2 -55.2 -67.2 173.5 156.2 173.5 156.2 175.1 156.2 176.7 156.0 156.0 176.4 153.6 176.4	176.8       -69.1         176.8       -49.3         147.2       -44.1         -77.9       -73.8         -87.2       -62.7         -73.3       -48.7         -87.2       -73.8         -30.8       -62.7         -30.9       -71.8         -36.9       -46.1         -49.2       -71.8         -55.2       -46.1         -67.2       -71.8         173.6       -62.2         173.5       -44.9         156.2       -56.0         153.1       -49.3         156.2       -73.9         176.7       -56.0         156.0       -26.9         156.0       -26.9         156.0       -8.9         176.4       -8.9         176.4       -8.9         176.4       -26.9	176.8       -69.1       1.27         176.8       -49.3       1.27         147.2       -44.1       1.27         -77.9       -73.8       1.27         -87.2       -62.7       1.27         -73.3       -69.4       1.27         -73.3       -48.7       1.27         -87.2       -73.8       1.27         -87.2       -73.8       1.27         -30.8       -62.7       1.27         -30.9       -71.8       1.27         -36.9       -46.1       1.27         -49.2       -71.8       1.27         -55.2       -46.1       1.27         -67.2       -71.8       1.27         173.6       -62.2       1.27         173.5       -44.9       1.27         155.2       -56.0       1.27         153.1       -49.3       1.27         155.2       -73.9       1.27         176.7       -56.0       1.27         176.7       -56.0       1.27         156.0       -26.9       1.27         156.0       -26.9       1.27         156.0       -8.9       1.27	176.8       -69.1       1.27       176.8         176.8       -49.3       1.27       176.2         147.2       -44.1       1.27       -82.5         -77.9       -73.8       1.27       -77.9         -87.2       -62.7       1.27       -87.2         -73.3       -69.4       1.27       -73.3         -73.3       -48.7       1.27       -73.3         -87.2       -73.8       1.27       -87.2         -30.8       -62.7       1.27       -30.8         -30.9       -71.8       1.27       -30.9         -36.9       -46.1       1.27       -42.9         -49.2       -71.8       1.27       -61.2         -67.2       -71.8       1.27       -61.2         -67.2       -71.8       1.27       -156.2         173.6       -62.2       1.27       -156.2         173.5       -44.9       1.27       -156.2         153.1       -49.3       1.27       -153.1         153.1       -49.3       1.27       -165.1         176.7       -73.9       1.27       -165.0         156.0       -26.9       1.27       -165.0 <td>176.8       -69.1       1.27       176.8       -62.2         176.8       -49.3       1.27       176.2       -44.9         147.2       -44.1       1.27       -82.5       -73.8         -77.9       -73.8       1.27       -77.9       -44.1         -87.2       -62.7       1.27       -87.2       -55.3         -73.3       -69.4       1.27       -73.3       -62.7         -73.3       -48.7       1.27       -73.3       -73.8         -87.2       -73.8       1.27       -87.2       -44.1         -30.8       -62.7       1.27       -30.8       -55.3         -30.9       -71.8       1.27       -30.9       -46.1         -36.9       -46.1       1.27       -42.9       -71.8         -49.2       -71.8       1.27       -42.9       -71.8         -49.2       -71.8       1.27       -61.2       -71.8         -55.2       -46.1       1.27       -61.2       -71.8         -55.2       -46.1       1.27       -156.2       -69.1         173.6       -62.2       1.27       -156.2       -69.1         156.2       -56.0       1.2</td> <td>165.2       -73.9       1.27       173.6       -73.9       1.27         176.8       -69.1       1.27       176.8       -62.2       1.27         176.8       -49.3       1.27       176.2       -44.9       1.27         147.2       -44.1       1.27       -82.5       -73.8       1.27         -77.9       -73.8       1.27       -77.9       -44.1       1.27         -87.2       -62.7       1.27       -87.2       -55.3       1.27         -73.3       -69.4       1.27       -73.3       -62.7       1.27         -73.3       -69.4       1.27       -73.3       -62.7       1.27         -87.2       -73.8       1.27       -87.2       -44.1       1.27         -87.2       -73.8       1.27       -87.2       -44.1       1.27         -87.2       -73.8       1.27       -87.2       -44.1       1.27         -87.2       -73.8       1.27       -87.2       -44.1       1.27         -30.9       -71.8       1.27       -87.2       -44.1       1.27         -30.9       -71.8       1.27       -49.2       -46.1       1.27         -49.2</td> <td>165.2       -73.9       1.27       173.6       -73.9       1.27       176.8         176.8       -69.1       1.27       176.8       -62.2       1.27       176.8         176.8       -49.3       1.27       176.2       -44.9       1.27       147.2         147.2       -44.1       1.27       -82.5       -73.8       1.27       -82.5         -77.9       -73.8       1.27       -87.2       -55.3       1.27       -87.2         -87.2       -62.7       1.27       -73.3       -62.7       1.27       -73.3         -73.3       -69.4       1.27       -73.3       -62.7       1.27       -73.3         -73.3       -69.4       1.27       -73.3       -62.7       1.27       -73.3         -73.3       -69.4       1.27       -73.3       -62.7       1.27       -73.3         -87.2       -73.8       1.27       -87.2       -44.1       1.27       -30.8         -30.8       -62.7       1.27       -30.8       -55.3       1.27       -30.8         -30.9       -71.8       1.27       -42.9       -71.8       1.27       -42.9         -49.2       -46.1       1.27&lt;</td>	176.8       -69.1       1.27       176.8       -62.2         176.8       -49.3       1.27       176.2       -44.9         147.2       -44.1       1.27       -82.5       -73.8         -77.9       -73.8       1.27       -77.9       -44.1         -87.2       -62.7       1.27       -87.2       -55.3         -73.3       -69.4       1.27       -73.3       -62.7         -73.3       -48.7       1.27       -73.3       -73.8         -87.2       -73.8       1.27       -87.2       -44.1         -30.8       -62.7       1.27       -30.8       -55.3         -30.9       -71.8       1.27       -30.9       -46.1         -36.9       -46.1       1.27       -42.9       -71.8         -49.2       -71.8       1.27       -42.9       -71.8         -49.2       -71.8       1.27       -61.2       -71.8         -55.2       -46.1       1.27       -61.2       -71.8         -55.2       -46.1       1.27       -156.2       -69.1         173.6       -62.2       1.27       -156.2       -69.1         156.2       -56.0       1.2	165.2       -73.9       1.27       173.6       -73.9       1.27         176.8       -69.1       1.27       176.8       -62.2       1.27         176.8       -49.3       1.27       176.2       -44.9       1.27         147.2       -44.1       1.27       -82.5       -73.8       1.27         -77.9       -73.8       1.27       -77.9       -44.1       1.27         -87.2       -62.7       1.27       -87.2       -55.3       1.27         -73.3       -69.4       1.27       -73.3       -62.7       1.27         -73.3       -69.4       1.27       -73.3       -62.7       1.27         -87.2       -73.8       1.27       -87.2       -44.1       1.27         -87.2       -73.8       1.27       -87.2       -44.1       1.27         -87.2       -73.8       1.27       -87.2       -44.1       1.27         -87.2       -73.8       1.27       -87.2       -44.1       1.27         -30.9       -71.8       1.27       -87.2       -44.1       1.27         -30.9       -71.8       1.27       -49.2       -46.1       1.27         -49.2	165.2       -73.9       1.27       173.6       -73.9       1.27       176.8         176.8       -69.1       1.27       176.8       -62.2       1.27       176.8         176.8       -49.3       1.27       176.2       -44.9       1.27       147.2         147.2       -44.1       1.27       -82.5       -73.8       1.27       -82.5         -77.9       -73.8       1.27       -87.2       -55.3       1.27       -87.2         -87.2       -62.7       1.27       -73.3       -62.7       1.27       -73.3         -73.3       -69.4       1.27       -73.3       -62.7       1.27       -73.3         -73.3       -69.4       1.27       -73.3       -62.7       1.27       -73.3         -73.3       -69.4       1.27       -73.3       -62.7       1.27       -73.3         -87.2       -73.8       1.27       -87.2       -44.1       1.27       -30.8         -30.8       -62.7       1.27       -30.8       -55.3       1.27       -30.8         -30.9       -71.8       1.27       -42.9       -71.8       1.27       -42.9         -49.2       -46.1       1.27<

Factored Loads and Moments with Corresponding Capacities: (see user's manual for notation)

				and the last of th		
No.	Pu kip	Mux k-ft	Muy k-ft	fMnx k-ft	fMny k-ft	fMn/Mu
1	51143.0	32509.0	64140.0	227118.7	447420.8	6.978
2	52138.0	41314.0	78497.0	231662.7	441476.9	5.620
3	55938.0	28261.0	78893.0	188615.5	527537.9	6.685
4	58457.0	38568.0	88297.0	213650.4	488337.4	5.532
5	51065.0	46002.0	79348.0	241748.9	418324.3	5.268
6	52072.0	56701.0	101248.0	239064.4	426657.3	4.215
7	53917.0	45223.0	71611.0	253718.5	401866.1	5.611
8	58457.0	38568.0	88297.0	213650.4	488337.4	5.532
9	43349.0	22307.0	130903.0	104603.0	620200.7	4.736
10	44180.0	25264.0	182035.0	89485.8	642137.2	3.528
11	46092.0	12105.0	172685.0	48544.3	692504.7	4.010
12	47641.0	14369.0	195399.0	51488.6	694072.8	3.552
13	12386.0	32509.0	64140.0	177850.0	351098.3	5.473
14	12611.0	41314.0	78497.0	180727.8	344070.1	4.381
15	10375.0	28261.0	78893.0	145472.6	406751.2	5.155
16	9510.0	38568.0	88297.0	161501.1	370735.3	4.197
17	12464.0	46002.0	79348.0	186557.6	321696.8	4.055

18	12677.0	56701.0	101248.0	185016.8	329853.9	3,259
19	12396.0	45223.0	71611.0	190942.9	301852.0	4.217
20	9510.0	38568.0	88297.0	161501.1	370735.3	4.197
21	20181.0	22307.0	130903.0	89622.7	527367.2	4.028
22	20569.0	25264.0	182035.0	74980.7	541393.7	2.974
23	20222.0	12105.0	172685.0	39433.6	562619.3	3.258
24	20327.0	14369.0	195399.0	41583.5	562013.0	2.876

<sup>\*\*\*</sup> Program completed as requested! \*\*\*

6																																																						P18 & P76	1000	-		
pund	M	4	*	cn	0	7	On sc	10	=	12	13	4	zA.	16	17	6	6	20 5	21	20	2 5	2 6	20	27	8	8	8	31	K	23	£	8	4 2	2	8	8		3 2	2	6	8	0	5 6	8	51	52	20 .	2 E	84 8	57	28	60	8	2	owny			
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10000 60 0.80	000 00 0.60	10000 80 0.60	8	0000 80 0,60	000 80 0.60	000 00 0.00	000 80 0.50	08.0 08 000	08,0 08 000	8	8	8	8	8	8 8	8 1	200 90 0 000	8 8	3 8	8 8	8	8000 80 0,80	8	8	8	8	8	8	8	8	8 1	8 8	8000 80 0.60	8	g	2 2	8 8	8	8	8	7000 00 0.00	8 8	88	8	7000 80 0.60	7000 60 0,60	8 8	090 09 0007	8 8	8	7000 60 0,00	7000 60 0.60	7000 60 0.60	No of the court				
1150	1294	118	100	1202	1151	i k	78	Z.	767	777	8	20	874	23	794	38	2	743	871	198	808	855	869	759	638	698	730	736	724	700	200	174	846	2	8	500	044	500	536	2	2	507	100	478	460	460		406	376	327	284	235	120	all				
4530	4530	4530	4530	4530	4530	450	1530	4530	4530	4530	4530	4530	Ses a	1000	2000	1630	1000	9550	4030	4530	4530	4530	4530	4530	4530	4530	4630	4530	4630	1620	1	500	4530	4530	4530	4530	4630	4530	4530	4530	4530	4050	4530	4530	4530	4630	4530	4630	4530			4530			4			356190
4530	4530	4630	4530	4530	4530	4500	4630	4530	4530	4530	4530	4530	200	OCC P	OBC.	1000	4000	100	4052	4062	4062	4052	4052	4062	4062	4062	4052	4052	4053	4052	2007	2002	4062	4062	4062	Santa Oran	3790	3790	3790	3790	3790	06/6	3790	3790	3790	3790	DEJC.	3790	3790	3790	3790	3790	3/80	Ripe	470)			Reinforcement of Wall
2	25	9	2	2	29	29	29	Q.	OX.	2	29	2 9	25	25	29	2 5	S S	QX.	2	OX	ě	e	Q	ð	ě	8	2	2	2	25	25	200	OK	Q.	2 5	2 5	2	QX	Q	29	200	2	R	06	9	2 9	29	2	ş	Q	Q:	29	2 9		Grab-	section	Check size of	Wall
				Ŋ	10,0	100	2	9				20	20	v	7	v	17	-	-	488	400	406	100																													8 2		-	*			
0.0027	DOMA:	D.CO.O.	O COUNTY	00000	0.0037	0.0025	0.0025	0.0025	20000	2000	Cana	Como.o	COUNTY.	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0028	0.0025	0.0026	0.0025	00005	0.005	0.0000	0,000	0.0025	0.0025	0.0025	0.0025	0.000	0.0026	0.0025	0.0025	0.0025	0000	0.0026	0.0025	0.0026	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0005	0.0025	ı	Ì			
450	1.00	100	1.00	4.00	1.98	1.50	1.56	158	150	160	1.58	1.50	1,58	1.58	1.58	1.58	1.20	1.20	120	1.20	1.20	1.20	120	2	120	120	100	200	1	2 20	1.20	1.20	1.20	120	1.20	120	1.20	128	120		120	1.20	1.20	126	3 5	1.28	120	1.20	120	120	120	120	1.20		7	ridding.	Alea of	
	13.6	125	10.0	1	10.4	18.0	18.0	18.0	0.00	10,0	18.0	0.0	18.0	18.0	18.0	18.0	18.0	16.0	16.0	18.0	16.0	18.0	180	180	10.0	9.0	10,0	10,0	16.0	16,0	16.0	16.0	16.0	100	76.0	16,0	16.0	18.0	18.0	10.0	16.0	18.0	18.0	18.0	16.0	16.0	16.0	16.0	16,0	18.0	180	16.0	16.0		pasmber			
	40	0.0	4.0	4.0	40	40	40	4 4		4.0	4.0	*6	4.0	4.0	4.0	4.0	12.0	12.0	12.0	12.0	12.0	120	120	130	0.24	12,0	12.0	12.0	120	12.0	120	120	120	120	120	12.0	12.0	120	120	12.0	120	120	120	120	120	120	12.0	120	120	120	120	12.0	12.0	Ť	provided	Spacing		Check design
0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0000	2000	0.013	0.013	0.013	0,013	0.013	0.013	0.013	0.003	0.003	0.003	0.003	0.003	0.003	0,000	0.000	0.003	0.003	0,003	0.003	0.003	0.003	0.003	0.003	0.003	0.000	0.003	0.003	0.003	0.000	0.003	0.003	0.003	0,003	0.000	0.000	0.000							0.003			1			gn
148	485	4486	1485	4486	4486	48	1	200	1400	4405																																				1864						188	10	kips	V .W			
4485	4485	438	488	4485	4485	4485	4485	4486	4485	4465	4485	4485	Alles	485	485	4485	1812	1716	1718	1716	1716	1718	1718	1718	1718	1718	1716	1718	1718	1718	1716	1716	2710	1716	1710	1684	1881	M001	1054	1884	1664	1664	1691	1004	1684	1664	1884	1664	1000	18	1664	1664	1684		TOAcp*wort(Fa)	At A to utu a	*	
0.48	0.43	243	0.46	0.43	0.41	031	020	0.28	0.29	0.25	0.31	0.32	0.31	0.29	0.23	0.23	0.00	0 5	071	0.79	0.00	0.83	0,74	0.62	0.88	0.71	0.71	0.70	99.0	0.67	0.85	000	0.01	0.59	0.57	0.56	2 6 6	0.00	0.94	0.62	0.53	0.46	0.48	0.47	0.47	0.48	0.43	041	0.00	0.28	0.23	0.18	0.22		VAN			
					4.07																																												Ì			8.14			Pepidore	Overstrength		

Plert and Pierz							Sheer P	alsforwment c	at of Well	L	1			Chack dealgo	agla			- 1	
Walito	Story	Width	Length	7.3		> 2	₹'s	V	Check size of section V	₹.	į	Area of attent wellthin specing	Spacing required		Į	, v. v.	10Ac		VAV
PIC & P2C	25 03	22	248	M -	0.60	142	5962	4080	š	88	0.0025	1,28		1	1.		Mps 24	т	0.10
	80	2	248		0.60	8	5052	4980		989	0,0025	85	18.0	120	0000	2484	2484		0.06
	82	24	248	7000 60	0.60	74	5962	4890		288	0.0026	130					77.		00
	25	24	248	7000	090	8	5952	A980		989	0.0025	1.20					200		800
	8 8	2 2	246	7000 80	6.60	90	5962	4980		989	0.0026	1.20	Ė				245		3 6
	3 3	38	248	7000 40	0.00	25	20807	4960		8 5	0.0025	120					24		0
	2	75	248	7000 80	080	19	5062	4060		989	0.0025	120					248		9.4
	25	24	248	7000 60	0.60	170	5962	4960		200	0.0000	250	18.0				246		0.10
	10	24	248	7000 60	0.60	4	5952	4680		586	0.0005	130					246		5
	8	75	245	7000 000	0,60	22	5962	4880		988	0.0006	130	200				248		5
	9	54	248	7000 60	0.60	283	2989	4960		508	0.0025	120	ĺ				24		5
	\$	24	248	2000 60	0.60	408	5862	4060		598	0.0025	120					78		N I
	4	2	248	7000 80	0,80	326	5862	4980		200	0.0025	120	18.0				240		50
	9	2	248	2000 60	090	468	5062	4880		596	0.0025	120	5	ľ			940		
	9 :	77	248	000	0,00	97.4	5962	4960		288	0.0025	1.20	í				246		2 2
	\$ :	* :	248	2000 000	0.60	989	5952	4980		989	0.0025	1.20	Ĺ				346		9 3
	3 5		9	000/	0.60	2	5962	4980		989	0.0025	1.20					248		2.40
	2 :		200	7000 80	0.60	77	2989	4980		809	0,00025	120					348		1
		4 7	248		38	210	2000	4980		698	0,0026	1.20					248		
	2 2	5 7	249	8000		9 9	7000	725		659	0.0025	1.20					255		6
	3 29	1 2	248	8000 60	8 8	9 5	2080	5524		62	0.0025	120	18.0				190		2.19
	31	34	248	3 5	080	200	2000	9250		8	0.0025	120					255		5
	98	24	248	8	900	332	5063	200		200	0.0025	8					200		0.16
	18	24	248	8	080	32	2007	1000		200	0.0025	128					265		5
	*	24	248	8	0.60	2	299	703		836	07000	N.					255		55
	83	24	248	8	0.60	218	5862	500		200	0.000	1					285		24
	R	24	248	8	0.60	219	5962	5224		830	0.0005	130					265		2
	31	24	248	8	0.60	22	5952	5324		639	0.0026	1.20					8		77
	8	24	248	8	0.00	528	5862	5324		639	0.0005	1 20					255		0.15
	20	24	248	8	0.00	233	5862	6324		83	0.0025	1 20					280		25
	58	24	248	용	00'0	248	5965	5324		639	0.0025	120					S S		0.15
	27	2	248	8	0.60	283	6852	MOD9		639	0.0025	120					a s		9
	58	24	248	8	0.60	284	59652	5324		630	0.0025	120					8 1		
	g :	2	248	8	0.60	ž	6952	5324		629	0.0025	130	Ĵ				98		
	3 5	\$ 2	240	8	0.00	98	2989	6224		830	0.0025	120	Ĝ				956		3 6
	3 5	5 2	040	3 1		3 1	7000	5324		3	0.0025	120					350		2
	3 5	36	248	38		700	7000	9324		620	0.0026	1.20					282		3
	8	374		3 8		3	2000	6700		200	0.0025	120	Ì				285		187
	9	24	248	8	080	123	5067	2080			0000	1.20	18.0				267		11
	180	24		8	0.60	2	K06.2	5060			0.0025	1,58	18.0		0.01		989		3.12
	11	24	248	10000 90	0.80	282	2005	5060		244	0.000	1.00	18.0	4.0	0.01		596		1.16
	16	24	248	8	0.60	620	6067	6908		244	D'MED	907	18,0	4.0	0.01		585		900
	15	54	248	8	080	512	5067	6062		. 7	0000	100	18.0	4.0	0.01		596		1,15
	7	24	248	10000 80	0.80	418	5062	5967		714	0.0000	8 1	18.0	4.0	0.01		989		7
	13	24	248	8	0.60	289	5952	5962		71.4	0.000	2 1	18.0	4.0	0.01	7068	296		1,12
	12	24	248	8	09.0	83	2989	5962	ð	714	90000	25	48.0	200	100	- 1	280		80
	=	24	248	8	0.60	215	5962	5995	ð	714	0,0025	1.58	18.0	40	100	7060	986		0.18
	10	24	248	10000 80	0.60	27	5962	2989		714	0.0025	1.58	180	99	000	70	Caro		9
	20 6	8 :	967	10000 90	080	781	5962	5962		7.	0.0025	1,58	18.0	4.0	0.016		707		9 5
		**	240	10000 50	080	8	2985	2902		714	0.0025	1.58	18.0	40	0.018		205		18
			248	2000	200	3 5	2090	2000		74	0.0025	1.58	18,0	4.0	0.04	17	285		1
	un	24	24R 3	1000 60	0.60	900	2002	7000		*	0,0025	1,58	18.0	40	0.01	7	5965		16
	*	77	248	09 0000	0.80	819	6967	5967		11.4	dans.	2 5	18.0	9	0.01	7058	5965		137
	10	72	248 1	10000 60	090	I	5862	2985	50	147	0 0000	9 9	10.0	4.0	0.018	1	585		116
	*	24	248	10000 80	O RIG	-	COEN	BACK			U. 5000	1.00	10.0	40	200	1000	2005		36
					200	CAND.	7000	7000		714	00000	1.53	10.4	A.A.	0.00	1	Name of the last		Ī

4069-20050500-2 09-CD-Weil Designs - Blast and Blast

	6																																																	P10 & P20	CINELA			Pierri and Pierz	By: NJR
l	Parison	2	cu		. 0	n a	7	80 1	0 2	1	12	ä	ż	ch a		7 0	100	20	21	13	2 2	2 6	8	27	28	8 8	3	K	21 1	8 3	8	37	8 8	6	61	38	ż	8	4	8	49	50	2	2	r	2 2	57	58	50	2	Auto				NUR
	5	8	8	8	8	8	8	8 8	8 8	8 8	30	8	8	88	8 8	8 8	30	8	30	8	8 8	88	8	8	88	8 8	30	90	88	3 8	8	8	8 8	8 8	8	8 8	90	8	8 8	8	8	8 8	8 8	8	88	38	8	8	8 8	8					N
ľ	7	151 100	151 10	151 10	151 10	161 10	151 10	151 10	101 101	151 10	151 10	151 10	151 10	151 10	151 10	151 10		Œ	-	151 8	23		7		5 0			-	161	C	151 8	151 8	151	151 8	151	151	151	151	151	151	161	151	151	151	61	2 25	5	5	5 5	151	in				
ı	10000 0000	00.00 00 000	0000 80 0.8	P'0 09 0000	00 00 Q.E	30 08 000	3.0 00 000	000 00 00	8000	000 80 0.80	8	0000 80 0.0	0000 60 0.0	0000 60 000	8 8	8 8	000 00 0.00	8	8000 80 0,80	8	8000 00 000	8 8	8	8	8000 80 0	8 8	8	8	88	38	8	8000 80 0.80	8 8	8	8	8 8	8	2 8	8 8	8	8	88	8	8	8 8	8	8	8	7000 80 0.60	7000 60 0	pal Nai				
ľ		Ī	30 1346	907	900	398	988	808	Ÿ.	Ä		8	80	200	700	600			Ī.	85		8 8		ĵ.	60 718		Ĩ,			Ē	ij	60 567		60 488	õ			988			430	80 438	1		36 36						Nos				
ľ	_		Г							_		-	4530	4030	4530	4530	_	Ť		-	-	4530		-			_	_		_	-	_	-	_	-	-	_	-	-	-	-	-		_	453				3 5	453	5.7			Share	
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2		OK :	2	읒	QX	OX.	2	2 9	9	무	2	2 9	2 9	2 9	2	NO	OK.	S	2	2 5	200	000	Q.	23	200	2	Q	29	29	ě	OK.	25	2 9	ě	25	Š	OX.	9 9	2	OK.	25	22	OX.	Q.	2 0	OK.	00	99	2	ī,	V (V.4)	section	Charak size of	EP.W.	
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200	2	3 6	100	180	18.0	18.0	10,0	18.0	18.0	18.0	100	0.81	18.0	18.0	18.0	18.0	18.0	460	10.0	16.0	16.0	16.0	16.0	15.0	18.0	16.0	18.0	10.0	18.0	18.0	16.0	100	16.0	18.0	18.0	18.0	18.0	10.0	18.0	16.0	16.0	16,0	16.0	16.0	18,0	18.0	16.0	18.0	16,0	16.0	beniupen	_	Ĭ		
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	から	3.33	4.05	4.98	5.04	5.08	6.73	8.35	540	5,10	6,63	5.56	6.51	5.77	574	240	2.33	1.94	2.02	2.48	2.58	240	241	2.39	2.46	2.53	2,60	2.67	274	282	1.03	3.16	300	3,81	4.22	2.52	172	2.99	4.49	3,87	3.83	3.80	90	42	4.36	5.30	6.14	7.14	2.18		W.A.A.	Owerstrength			

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not for we	Areas of added legs	è							I																	0.80	080	0.60	0.60	09'0	0.60	080	080	090	0.60	090	980	080	0.60	080	0.00	080	200	080	0.60	0.60	000	0.60	0.50	0.60	0.60	0.60	0.00	0.60	0.60
Confinement in width	Legs in addition to outer hoop						1				1	I		I		1			T							*		,	4	*	9		4 4	4		*		,	*	*	-	-	,	*	+	*			*	7	1		*	4	7
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	% of Length		100	E/J	rig	Bu	rigg.	0/0	Na Na	E .	2	e/o	2	1/8	n/a	E/U	9,0	n/s	2	2	2	0/4	ejsu	n/a	alu.	0.31	0.34	0.34	0.31	0.31	0.31	50	0.34	0.31	0.31	0.31	0.31	031	0.31	031	0.31	0.51	200	0.31	0.31	0.31	500	0.34	0.31	EO	0.34	0.31	0.31	0.31	0.31
۱	forg thought to though	5	nia	IVE.	nje	n/a	n/s	1/8	Dig.	ne	n/a	200	o/o	s/s	nia	nie	N/B	103	orie o	963	nju	nia	nya	rva	nie	100	6.3	5.7	5.7	5.7	5.7	27	2.7	\$7	5.7	5.7	5.7	27	5.7	5.7	5.7	37	5.7	6.7	5.7	5.7	100	5.7	5.7	5.7	5.7	57	5.7	5.7	5.7
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	owing	T	eju	n/a	nia	e de	n/a	rya	100	N/a	Na Na	No.	n/a	9,0	n/a	n/a	Na.	n/o	aria a	n/a	ago.	ri/s	ule	n/a	nla	× ×	5 8	ĕ	OK	¥	×	¥ 8	5 8	ŏ	OK	OK	88	3 30	ž	×	X	88	200	NO.	OK	×	58	5 8	30	30	*	58	ě,	OK	36
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	s (spacing) Asi		1	_			1		1	1	1	1		-				1	1											2	*	1		4	4	*	*			4	4			4	4 2	4	1	4 6	4	4 3	4	4 4	4	4 2	4
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1	B.E. <sup>2</sup> Lange	9								1	1	1														900	90,	900	300	108	108	306	8	8 8	108	108	80	9 8	108	108	108	8	100	18	106	108	108	100	108	108	808	108	108	108	508
	28		19/8	apu	n's	n/a	e/u	2	ē	2	2	2	2	2	2/0	n/a	ę.	ē.	N/A	1	1	N/A	178	n/a	Ø.	0.51	0.5	0.50	0.51	0.51	0.52	3	7,0	950	0.57	0.58	0.59	8 6	0.70	0.71	2970	0.67	0.87	0.89	0,70	0.73	0.75	0.22	0.72	0.73	0.75	0.78	0.50	0.80	0.824
	Length of B.E. Redd	-	4	н	н	4	4	-	4	+	A Dia	4	+	+	+	H	-	D'a	+	2	+	-	-	s ola	1 1/0	-	+	3 5	33	53	X	+	+	+	90		-	+	8	80	8	8	8 6	18	B	19	8	3 6	8	19 6	23	2 %	10	18	2
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	3	1	0.23	0.38	0.42	0.37	0.37	0.37	0.38	200	0.45	0.43	200	0.64	0.70	0.92	0.36	0.45	200	0.83	0.33	0.29	0.27	0.28	0.25	620	0.04	950	0.27	0.30	0.32	80	800	0.30	0.46	0.48	990	0.00	0.67	0.89	0.36	410	200	0.40	0.24	0.30	0.63	8 %	0.18	0.18	0.24	0.37	0.40	0.75	0.81
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	a	Page 1	ŀ	28854			7805	2000	2000	2007	OP 1	2892	2692	2695	2895	2833	682	8	City of	2000	26.06	3,600	No.	DEA	- Shen	Sugar	2000	200	2000	3 6	2000	2000	200	- Speller	2000	3000	WIGH	CONCE	3083	30600	3080	3080	3090	3850	38000	2000		Section .			Service Control	18506	30500	38506	38505	38800	38506	38506	38509	30000	200
	Conclusion		NOT REOD	NOT REO'D	NOT REQUE	VOT REOD	NOT REOD	NOT DECOR	STATE OF THE PARTY	OF SCHOOL	S NECES	NOI REGED	NOT RECYD	PROVIDEBLE	AUVIDE B.E	COVIDE B.F	CONDE B.E	PROVIDE B.E.	PROVIDEBLE	DAVIDE D.C	PROVIDERE	NATURE & C	UNIDER	RROVINGRE	T & SCINC	DROWING R.E.	CULTER B	ANALOG BE	NAME OF THE PARTY	DECEMBER OF	WATER OF	O COLUMN	TANK BE	Canal a contract	GOOWINGE	CVOFRE	CAMPERE	OVIDEBE	OVIDEBE	OVIDEBE	OVIDE B.E.	OVIDEBE	OVIDE B.E.	OVIDE B.E.	OVIDE 8.E.	CANDOBE	OVIDE BE	CANDED C	OWNER BY	CHARLE BY	TANK DE	CAMPERE	DATE BE	OVIDEBE	DVIDE B.E.	CANDE B.E.	OVIDE B.E.	PROVIDE B.E.	OVIDE B.E.	PHONING GE	OMIDE R P
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	MANA		0.41	0.49	0.30	0.33	0.48	0.61	0.44	0.48	200		0.61	0.63	0.43	0.40	040	000	0.40	0.10	0.25	0.35	0.44	0.43	0.43	0.43	0.40	0.40	0.49	670	200	0.43	0.43	0.43	0.43	0.42	0.30	0.39	0.48	0.38	0.40	0.43	0.17	90.00	070	10.00	0.43	270	1970	0.43	1 12 0	0.30	0.20	0.40	0.35	0.48	0.51	0.44	0.47	3	0.44
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New Auto	1	ľ	1			UNB.	TATE	TUE	n/a	N/B	n/a	nis	nya	0/3	n/a	u/a	ryla	FriB	0.48	0.46	0.40	0.40	0.40	0.68	0.55	350	O ACC	250	0.55	0.55	0.56	0.56	0.55	0.55	0.55	550	800	990	0.55	0.55	0.55	0.68	0.68	0.00	0.88	988	0.68	99'0	0.66	0,65	0.68	0.66	0.68	0.668	0.68	0.59	0.68	0.68	
BE' LANGE																			10	100	10	6	3 6	2	5 67	G	20		G	150	160	57	57	22	le e	15	200	57	16	57	57	25	150	100	6	15	10	29	25	57	25	257	25	à	n te	15	57	25	
38	Infin	ľ	9			96	8	n/a	n/a	n/a	1/3	Na	ova.	nya	NS.	aju.	NI.	e p	0.31	5	200	200	300	35.0	95.0	800	20	35	25.0	10.00	95.0	0.38	0.40	0.41	0.43	17.0	0 44	0.45	940	0.46	0.46	0.51	0.51	200	8890	250	0.54	0.54	0.55	0.55	980	950	150	20	8 9	190	0.62	0.63	
of B.E. Regid		1	+	4	1	4	4	-	_			Н	ш		n'e	-	198	4	4	100	4	1	1	1	1	1	1	8	R	8	8	41		3	4	4	8 5	1	1	帮		45	2	77	17	29	9	9	48	99	99	47	47	9 9	9 9	315	52	53	
0	t	1000		2000			9 6 6	4 0.05	4 0.06	4 0.05	4 0.07	4 0.08	60'0 5	4 0.09	000 9	8 8.10	4 0.11	4 0.17	0.12	4 0.12	2 0	200	200	1013	0.44	27.0	0.45	0.18	6 1 0.15	0.17	0.17	0.18	0.19	020	1021	200	0.00	0.23	0.23	0.23	0.24	0.19	0.19	380	1024	120	0.21	0.22	н	0.22	0.22	023	+	+	0.76	+	-	0.27	ì
a.	Vine	-8	-	4-	-	-	200	in the	南	3563	3541	3541	3541	35414	3541	3541	3541	3841	200	200	2000	26.14	4747	4047	40.47	4047	4547	ADA74	4047	40474	40474	40474	40474	40474	40474	40474	40474	4000	40474	40474	40474	50592	50582	50697	50692	50502	50692	50592	50592	50502	50692	50592	2500	SOUGH	20000	50582		50592	
Donolusion		NOT BEOD	MOT BEATT	10000	MOT BEAT	STATE OF THE PERSON	DE MENT	NOT RECOD	MOT REOTO	NOT REOLD	NOT REOD	NOT REOD	NOT REQYD	NOT REQTD	NOT REOD	NOT RECTD	NOT REOD	NOT RECTO	KOVIDE B.E	OWNER BE	ALL AND THE BELL	SAMPLE DE	ROVINERE	ACVIDE B.F.	SOVIDE R.F.	SOVIDE B.F.	ROVIDE B.F.	SOVIDE B.F.	ACVIDE B.E.	SOVIDE B.E.	NOVIDE B.E.	<b>SOVIDE B.E.</b>	<b>WINDE B.E.</b>	NOVIDE B.E.	COVIDE B.E.	COVIDE SE	MOVIDE BLE	COVIDERE	PROVIDE B.E. 40474	<b>JOVIDE B.E.</b>	NOVIDE B.E.	OVIDE B.E.	CAMPE BE	COVIDERE	SOMOE B.F	OWDE B.E.	DVIDE B.E.	OWDEBE	OVIDE B.E.	OVIDE B.E.	OVIDE B.E.	OVIDEBLE	WINE BLE	CAMPERE	PROVIDERE	PROVIDE B.E.	PROVIDE B.E.	OVIDEBLE	
9	-	Ì	T	Ì	Ť	t	1	4	П	1		1	П					J	2 0	1	0	6	a	15.	D	d	L	L		L	Ш			ч	П	II.	t	L	П	Ш		1	Т		1		11						T	I					
VAA-weets))		0.08	0.23	0.43	210	200	200	0.16	0.17	0.19	920	023	0.25	0.27	0.32	0,40	0.38	0.40	0,76	0.73	0.44	D. A.B.	0.38	0.36	0.34	0.33	0.33	0.33	0.33	0.33	0.34	0.36	0.39	0.42	0.39	00.00	0.43	0.45	0.40	0.63	150	0.00	000	0.31	0.40	90	0.36	0.45	0.71	920	0.72	0.66	100	0.73	0.74	0.81	0.75	192	
MAL		950	0.36	070	250	0.00	B	77.77	0.87	8	188	1.14	120	125	124	121	1,41	0.74	0.23	0.50	1.78	687	0.75	0.77	0.77	0.75	0.73	0.72	0.71	0,89	0.68	0.67	190	990	270	0.04	0.83	0.82	1.04	650	0.13	0.40	030	880	0.34	0.40	0,50	0.48	0.15	0.27	0.28	R	800	0.26	0.37	0.37	050	0.63	-
Symmetry 1=sym 0=un-sym			177	-				-	-	1	-	-	-	-	-		-	-	1	-	-		-	1	-	-	1.	1	1	1	4	1	-	-	-			-	-	-	-	-		-	-	+	1.	_	-	-	-	1	-	-	Ļ	,	-		
Sheart	u	872	248	248	245	246	37.0	200	200	8	97	248	248	248	248	248	2	267	8 9	248	265	748	248	248	248	376	248	248	246	248	248	240	248	248	200	348	192	248	248	248	548	992	246	248	245	246	548	348	248	248	248	057	990	100	545	248	248	949	
1	319	2555	5852	5055	5985	5052	5060	2000	7097		Ž,	2652	2862	2882	75.5	7000			2000		Call Call				5962				_		_		- 4			2050		10	29952			1	10		5962		2962	1	_	Ш	200		1	1	25052				
PANES	1	400	5107	80	0033	500	500	1	900	3	8	100	570	8/8	8	COUNT	N S	8 5	38	107	111	118	106	333	116	121	127	120	130	143	148	3	191	8 8	107	88	182	35	187	88	8	3 3	3 18	77	100	2	23	8	98	87	88	2 8	5	8	12	9	N	8	8
~	1	256	238	362	962	250	25.2	000	2000	2 5	2 5	700	200	d	1	100	200	200	650	3	200	120	250	352	362	352	352 (	255	200	62	254	250	2 2	70	4 6	20	55	62	622	95	316	200	25	52 0	52 0	62 0	52 0	N N	200	2	7 5	100	0	0	32 0	52 0	25	200	
-		130	118	841 3	74. 5	E.	811	271	3	200	200	2110	200	10/	101	200	DAY O	3 6	337	9 198	156	227 5	205 R	190 5	181 58	1771 8	178 59	176 55	177 56	177 58	181 58	182 56	200	24012	25 17	210 58	229 59	240 59	215 59	8 8	B 1	0 00	100	184 59	238 59	65 952	13	8 8	5	200	200	200	65	25	(43 59	194	9		
3,	Pot.	754	965	859	624	1000	1294	7751	1000	0.300	200	2003	10000	2000	4040	2000	1000	18081	2572	4415	4043	3312	3169	3007	2867	2752	2000	2002	2562	数実	2543	2012	2007	80.5	200	3906	3912	40B6	4944	80	200	2246	128	2229	1692	1830	2345	7007	100	1000	ALC: N	7470	2402	2311	3408	3685	8308	2000	
a'	200	287	920	1030	1353	1672	1745	2010	2224	25.17	Depart of	300	2000	3694	3 6	27.14	1	4181	4290	4458	4634	4835	2906	5296	5537	5779	8024	6274	66330	8795	7070	900	2000	New	875.4	1688	9138	9232	1835	2500	9070	7500	6000	0242	0453	0648	9280	777	2	/01	200	1774	1993	1972	2811	3016	2020	000	
7.420L+0.5L	sdry	151	301	200	753	963	3	1197	1300	98.50	200	1020	7756	PILIS.	0000	2000	2012	3112	3306	3503	3691	3879	4086	4254	4442	4630	4818	5006	5154	6382	6670	90,00	6434	6333	6209	1899	6904	7097	7278 9381 4844 215 5852 0.	70.40	2007	8023	8209	8366 1	856711	874811	8825	STATE OF	Outpe 1	1 0056	DE54 1	10001	1021811	10395 12	10574 13	107531	109001	1071	The second second
a"	200	130	349	475	909	109	800	588	5	1,0mg	7201	211	1180	7000	1212	4465	1087	1201	982	855	375	292	8	1643	1000	1149	9021	1268	1336	1413	1500	100	4874	280	2962	1887	2234	35136	2115	216	000	831	900	1854	988	208	276	4700	4872	10000	1656	7.57	122	581	2037	SHS	9/4	070	
2	55	10	0	12	н	_	-	+	+-	+	+-	4-	+	-	1	-	+	+	+	Н	Н	-	_	_	_	_	_	-	_	$\overline{}$	201	-	+				75 2		2	+	C.X	+		7.5	7	+	+	2 ×	+	т	75 16		75 17			75 22	_	_	7
2	-1	-	+	+	-	-	H	-	+	₽	+	+	+	+	+	+	000	000	000	000	000	000	000	000	000	000	000	00	000	8	000	38	900	98	88	000	9000	000	9000	DOM:	10000	-	Н	10000	100001	+	2000	+	troom	+	1	10000		Ц	-	10000	+	4	
Floor	1		1		1		H	r		t	t	t	t	t	+	t	t	t	H				+	+	+	+	Ì	1	†	1	+	t	t	t	-			+	1	t	Ť	t	Н	10	2	2 4	200	100	1	100	101	10	9	0	10	2	1	+	
æ		9	Ö	ń	8	10	8	ag	N.	16	12	in	a.		9	1	1	45	77	*	45	7	8	P	28	6	8	8	5	3	PI	18	8	K	23	R	10	24	2 2	3 2	8	13	40	4	10	2	2	2	+	10	a	60	-	CD	0		3 6	Greened	

8.1.1-41 DODSONNOC00000434

4089-20050509-2.09-CD-Wall Design - Piort and Pier2

8.1.1-42

Width of BE hoop h.= 27.9 in Portmetor of wall = 308 in.

5/13/2805

Avea of added legs A	'n	i sal				L	Г	17	П	$\neg$	,				)	+	t	۰	1		H			-	Ŧ	-	-	7	+	+	+	F	H	7	Ŧ	-	1	-	+	7	-	7	7	-	Ŧ	-	H	7	7	+	-	7
on Area of	Н	S	O. C.	100	2	a de	E	E.	nia	н	242	+	+	-	+	2,42	242	242	2.42	2.42	242	242	242	297	243	2.42	2.42	2.42	242	2 42	2.42	2.42	2.42	2.42	076	242	242	242	242	2.42	242	242	242	2.42	242	242	2.42	242	242	242	2.42	79.7
5890	E			1							0.60	0.60	09'0	070	0.60	090	90	090	0.80	0.80	0.60	09'0	0.60	36	100	0.60	0.50	0.60	200	200	090	090	0.60	0.60	200	0.60	090	0.60	0.60	0.60	0.60	0.50	0.50	080	0.50	09'0	080	0.60	080	0.80	090	0.00
Lagain addison to cuter hoop											0	2 (5	1 10	n	8	0	,	(*)	60	3	.,	6	3	2		3	3	10	nin	2 10	9 10	3	n	ene	9 60	es	E)	0	90		67	6	60	es e		0	2	0	10	6	9 4	-
10 11	'n			Ī	İ	Ī					500	033	0.31	0.31	0.31	0.31	0.31	0.31	0,31	0.31	0.31	0.31	0.31	0.50	100	0.31	0.51	0.31	0.31	12.0	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.31	0.33	0.31	0.31	0.31	031	0.34	0.31	0.31	0.31	0.31	0.31	0.31	0.31	1000
	N.	alla a	21	apo	ş	- Par	뵅		100	ę.	0.94	0.0	950	0.94	100	100	100	100	250	107	1.07	1.07	107	187	107	1.07	1.07	1,07	100	1.07	101	1.07	1.07	1.07	107	107	178	3	3 3	130	7.7	7,	3	1	100	27	3.34	3	N.	4	-	*
% of Length		- e/u	No.	a/a	ne	n/a	n/a	nia	and a	no.	0.45	0.48	0.48	0.48	0.48	0.48	0.48	0.48	0.48	0.48	0.48	0.48	0.48	970	0.48	0.48	0.48	0.48	0.48	0.48	0.48	0.48	0.48	0.48	0.48	0.48	0.45	0.48	0.48	0.48	0.48	0,48	0.43	0.48	0.46	0.48	0.45	0.48	0.48	048	9 68	04.0
long specing of legs	g	1/8	ale of	n/a	ale.	nja	nia	rija	n/a	nya	5.6	5.6	3.5	5.5	26	90	2.6	5.6	5.6	5.5	5.6	5.6	200	970	25	275	9.6	2.6	94	2.8	976	5,6	5.6	2.6	9.5	5.6	5.6	9.6	8.8	5.6	5.6	5.6	2.6	9.0	3.5	5.6	5.6	979	5.6	9.6	200	2.0
org bars alowed	g.	n/a	a de	1/4	ola	rafa	nia	piz	ngs.	ula 1	0.667	0.667	1,667	0.067	0.667	0.007	0.667	7997	0.867	0.667	0.667	0.667	0.607	0,667	1990	0.657	0.967	0.667	0.007	0.067	0,667	0,967	299'0	0.567	0.667	0.667	19970	0.667	0.667	0.657	0.067	0.867	0.667	0.657	0.667	0.667	0.667	0.567	0.667	0.567	0.667	Market .
guye		n/a	Dia.	ale a	100	nota	mia	nça .	riva	+	+	+	-	1	+	t	t			+	-	+	t	+	+	H	+	+	t	t	t	Н	+	XX	H	H	H	+	+	t	H	1	1	t	t	4	+	+	+	t	t	1
-	who	n/a	+	-	H	H	-			+	+	+	-	122	+	+	+	-	-	-	+	22	+	200	100	22	-	+	+	+	H	H	H	+	ł	Н	+	+	+	H		+	1	+	-			2	2	2	100	
*	Z	2	2 4	-		-	e .	0	N.	2	-	1	1.5	17	1	1	-	7	1.2	122	122	1.2		1		1.2	1.2	1.2	122	1	1.2	12	12	122	12	122	122	122	122	122	1.2	123	12	120	122	1.22	122	1.2	12	12	1	7
guipeds) u		4	-		1					1	9 4	4 4	8	4	4	9 7	9 40	4	8	9	4	4	4 .	9 9	1 4	8	4	*	0 0	4	*	4	31.4	2	4	3 4	*	4 .	7	*	*	7	4	*	1	*	*	4	1	4	1	1
legs Ash	Æ		+	-	+	+	H		-		13 0.38	13 03	13 0.38		13 03	200	13 10.38	-		15 038			-	13 0.36	13 03	13 0.38			18 0 38			13 0.3		13 0.36		13 0.38	13 0.38		13 0.38					13 0 25		13 0.38	13 0.30	13 0.8	3 0.3	3 0 3	13 0 38	10.00
3.	uju.	E/U	E ope	200	n/a	nia.	50	n/a	D/B	ula	0.61	0.61	1970	0.81	0.61	190	190	190	П	Н	7	1	0.70	0.00	0.00	H		+	0.00	t	t	Н	1	0.70	t	Н	Н	Ħ	986	t	Н	1	+	+	+		1	1	1	Ť	0.68	1
B.E.*	s		t	İ	t			-		1	22	2 22	22	23	2	2 5	2 12	73	7.3	7.3	7.3	2	21	2 12	1	73	2	2	24.5	13 5	E	g	73	22	202	73	+	2	32	22	12	1	1	2 %	+	Н	73	2	2	+	73	1
	u/u		No.	2	4	eye.	rije.	r c	1/2	n/a	0,19	020	0.21	0.22	021	0.10	0.20	0.21	0.21	020	0.22	0.22	270	200	0.73	0,23	0.23	0.24	200	95.0	19	0.27	92.0	0.00	0.31	0.27	0.29	620	28.0	08.0	0.31	32	333	150	0.31	0.32	0.33	23	1,34	8	0.50	1000
Length of B.E. Req'd	E	e/u	7	_	1	1	1	4		4	25		K	92	R	27	24		52	23	23	1	3 5	35	77	24	24	Q t	d K	22	27	182	30	100			22	2	т	19	32	1	1	28	t	П		7	+	1	33	1
5		904	000	0.03	0.04	0.05	0.06	90.0	600	0.11	0.13	0.16	0.18	6.19	0.19	200	0.17	0.18	0.18	0.18	91.0	0.16	0.00	910	0.18	0.17	0.17	000	0.10	0.30	0.21	0.22	0.24	920	0.28	0.23	0.16	0.17	0.18	0.18	0.20	0.21	220	9 9 9	0.19	120	120	220	220	623	080	1 230
a.	kips	25052	75050	28054	28054	15688	MAN	288	30304	N I	2005	2000	26954	26954	10000	SORRY.	26854	2007	5992	30804	30104	30904	3000	Sec.	30804	30804	30804	30804	30904	30004	30604	30804	SORON	30904	30604	30604	38505	39205	38605	38505	30505	38505	9000	20000	38505	38506	900	38505	98802	5000	38505	270
Conclusion		SEGO.	RECTO	RECO	REOD	REGO	RECTO	REGIO	RECOD	REGO	OC B.E.	DE B.E.	DE B.E.	10E B.E.	UE BE	DE RE	DEBE	DEBE	DEBE	DE B.E.	DE B.E.	DE B.E.	DE BLE	MERE E	EEB F	DE B.E.	DE B.E.	THE B.E.	DEBE	DEBE	DEB.E.	DE B.E.	DE B.E.	PROVIDE BE 30804	DE B.E.	DEBE	DESE	DEBE	DEBE	DEBE	DE B.E.	DE B.E.	DEBE.	DEBE	DE B.E	DE B.E.	DEBE	DEBE	UE B.E.	TEST PER	PROVIDERE	1
		2	NON	MO	NO	NO	NO	NO	ON	CN	580	PRO	PRO	PRO	SOC.	000	PROV	PROV	PROV	PRO	PRO	5	5 8	PROV	PROV	PROV	PROV	PROV	PROV	PROV	PROV	VCBR	DEG.	2000	PROV	PROV	PROV	000	PROV	PROV	PROV	PROP	2000	PROV	PROV	PROV	PROV	500	N COL	NCHO O	PROM	
VJA. TOTAL	U	I.		и	П	П	Ш	Ш	- 1	-	ш	U	П	M	ŀ	П	П	П	П	П	П	П	П	1	ı	85	1	Т		И	П	173	1			Ш	П		П	П				Ь			1	1	1	£	287	١
W/W.		9	0.38	0.33	6.48	0.41	0.41	0.48	200	170	3.45	177	98	0.54	22.5	020	24	141	345	42	7.62	747	200	175	745	77	75	260	9	3	(43	(42	121	0.30	133	61	199	97	170	44	41	7	100	34	52	40	1	9 9	3 0	200	180	
Symmetry 1-sym G-un-sym.			-			1	-						+				1	1	_	,					-	0 1									1 0	1 0	0	3 6	0	0	0		3 0	10	0	0	0 0	36	3 0	-	0	ľ
Shear L. Co.	<u> </u>	121	19	151	151	151	15	15	100	200	250	51	51	151	51	191	151	151	.9	121	101	101	1	191	151	151	101	15	21	15	191	121	151	1	35	150	100	2 2	-						9		1				-	
- 65 - 27		-	ы			ы	230	200	000	3 9	38	98	000	4530		1		4530			1	1	1	4630 1			1	98	4530 1	30		1	1	1.	2	8	839	30		30 151	4530 151	45.90	L	4530 15	30	41	-1	181	4	10		ı
PANA		0,000												0.164 4												0.141	9 2							0.233 46		7	+	+	Н	-	-	-	-	-		-	-+-	4-	4-	+-	6 4530	•
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/, (S)	E 15	200	84 453	96 453	97 453	37 453	453	2 2	200	13 453	1	453	8	3 2	453	453	453	123	S S	100	400	100	65.453	453	4534	53	40.5	4500	4530	3 4530	2 4530	453	403	2730 573 4530	9 4530	653	A PER	6530	6 4530	7 4530	4530	8 6530	9	0530	4530	653	4530	4530	4630	4530	1348 4530	Townson In
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4 2	9 5	8	182	125	20	1/4	010	98	250	277	125	116	5/8	188	19	202	17754	26	20	177	18	93	040	18	153	1631	2 28	100	31	10	8 2	3 2	20	93.1	90	2 8	33.8	92	03	3	3 59	12	12	T	2	8 5	7	20	3	*	0	20
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# **DESIMONE**

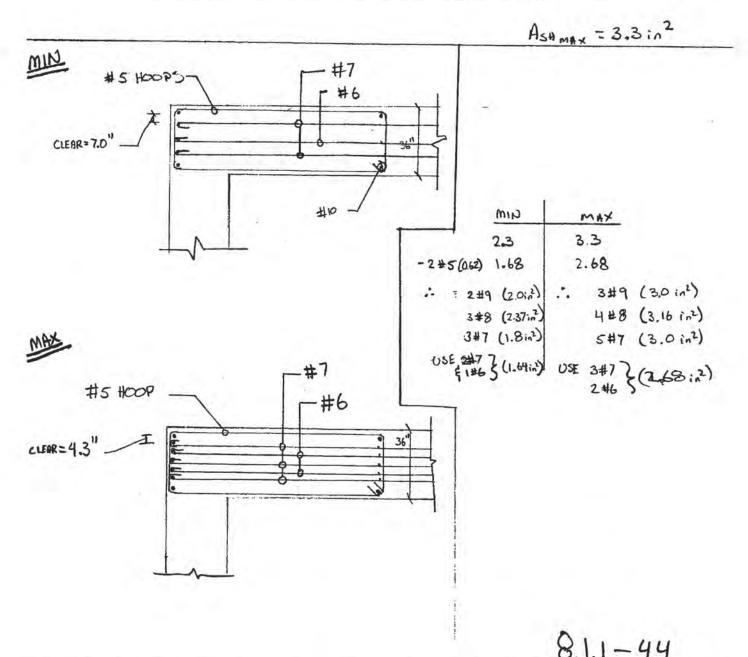
3-2-6	301	
Project	301 MISSION	PageOf
Project No.	14	Date 1 7/05
	T PIER LOWER 10 LEVELS	By NIR Ch'kd
TIES = GR 75		(ETABS NEW RIGS 1.01)
for Vert s	= 4" DETERMINE Ash req	through 36" WIDTH
	Ash = 0.09 S	he fic/(fy.1000)
	= 0.09 (4)(	(33,9) (10,000)/(75,000)
	Ashrea 1.63 in	1 (75,000)
	SHEAR	
	OPTION 1) Vu = 4267 kips	
	\$ Vn = Aev (2.0 Ja	1'e + Pn fx)
	4267k = (0.6)(12960 i	2)(2.0 J10000 + Pn (75,0001)
	Pn = 0.0046	5
4	@ 411 O.C. VERT	=> Asy = Pn(Ac1) = 0.00465(36")(4")
		Asvrey = 0.64 in2
4"	(OPTON 2)	(228 3,01101
	Vexpected = wv mo	VUSTATIC LOAD) VUSTATICIOAD (UPLYSM)
	= (5/3)(2	
	= 3.33	. Vu
		(3462 kips)
	= 11258	
	Set ØVn=Vexpected	(d=0.85)
	11528k = (0.6	35)(12960)(2.0\10,000 + /n (75,0001)
	Asures = 0.	01128(36")(4")
	= 1.	625 in2 Asureg = 1.625 in2

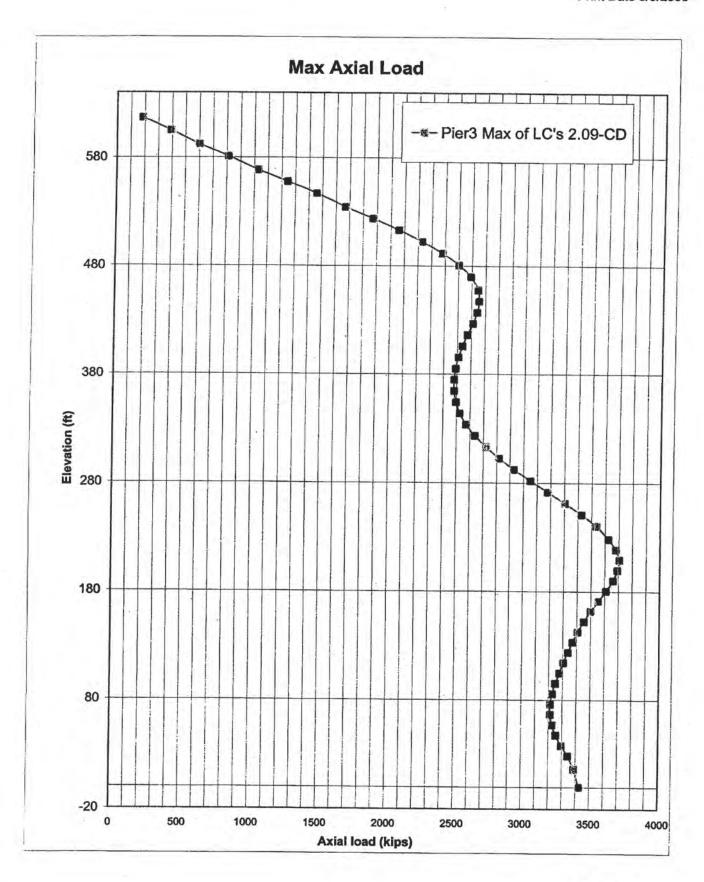
8.1.1-43

## DESIMONE

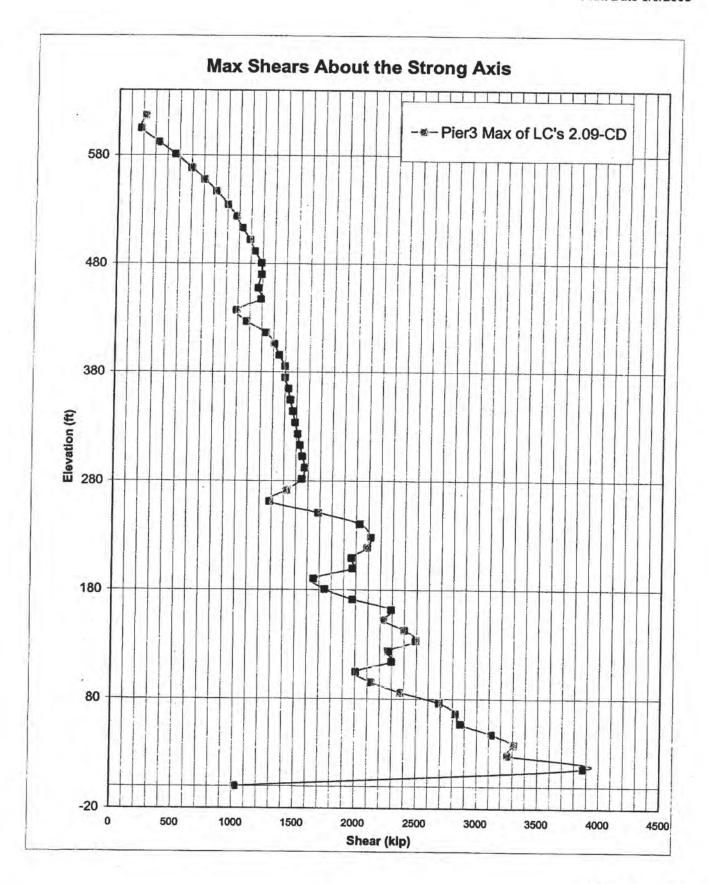
Project _	301	Page Of	107
Project No.	40690	Date 1/7/05	
Item _		By NJR Ch'kd	

MAX (Governteed NO SHEAR FAMOLECAPACITY DESIGN) = 1.63 in + 1.625 in

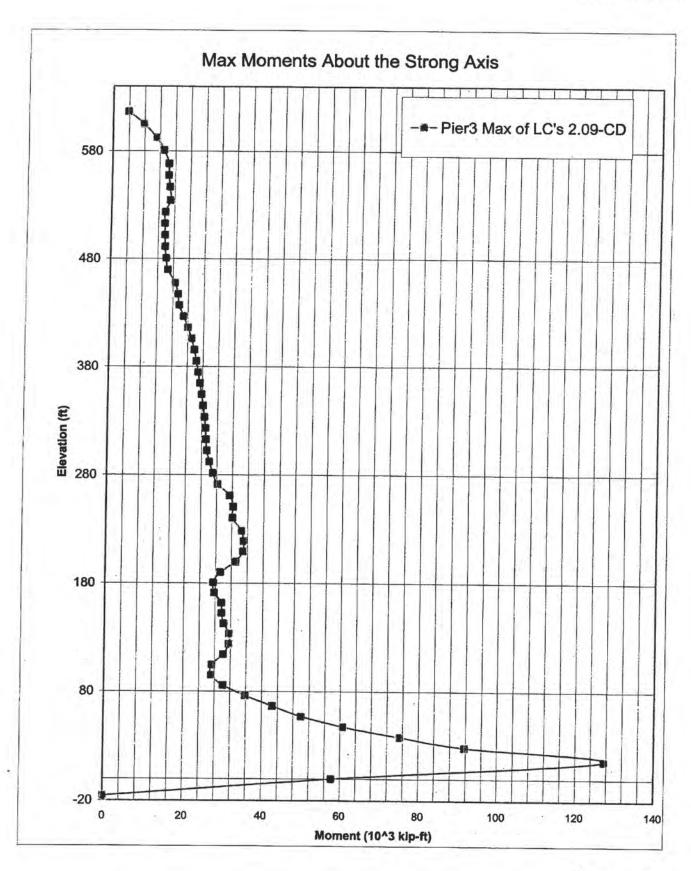




8.1.1-45



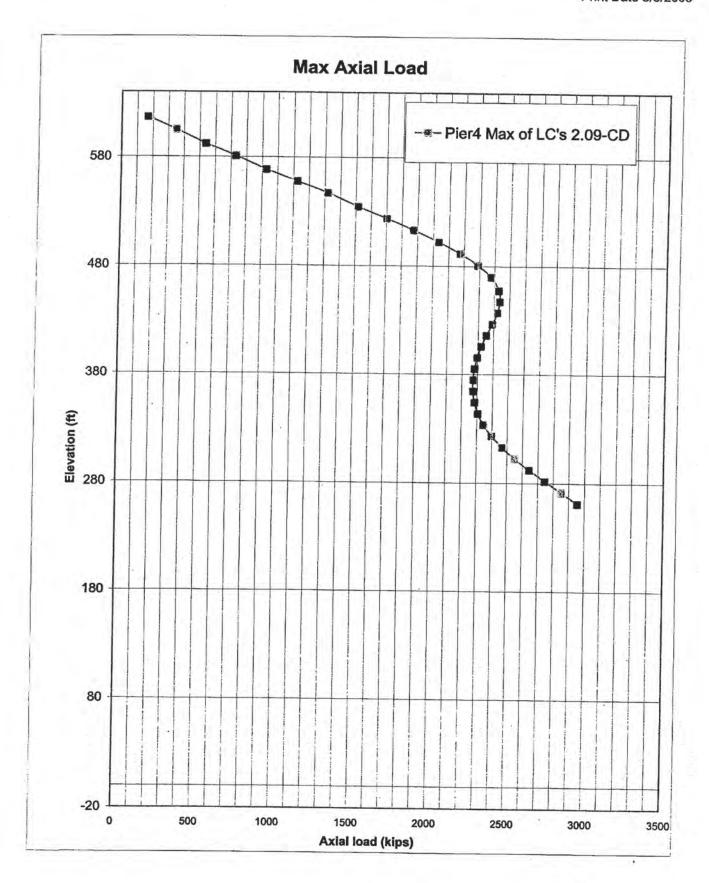
8.1.1-46



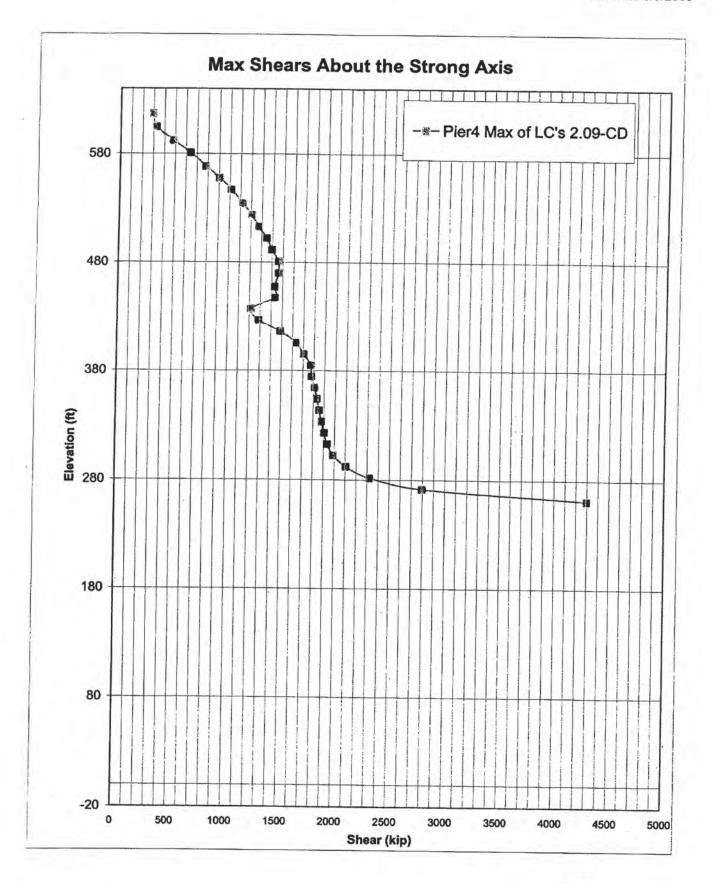
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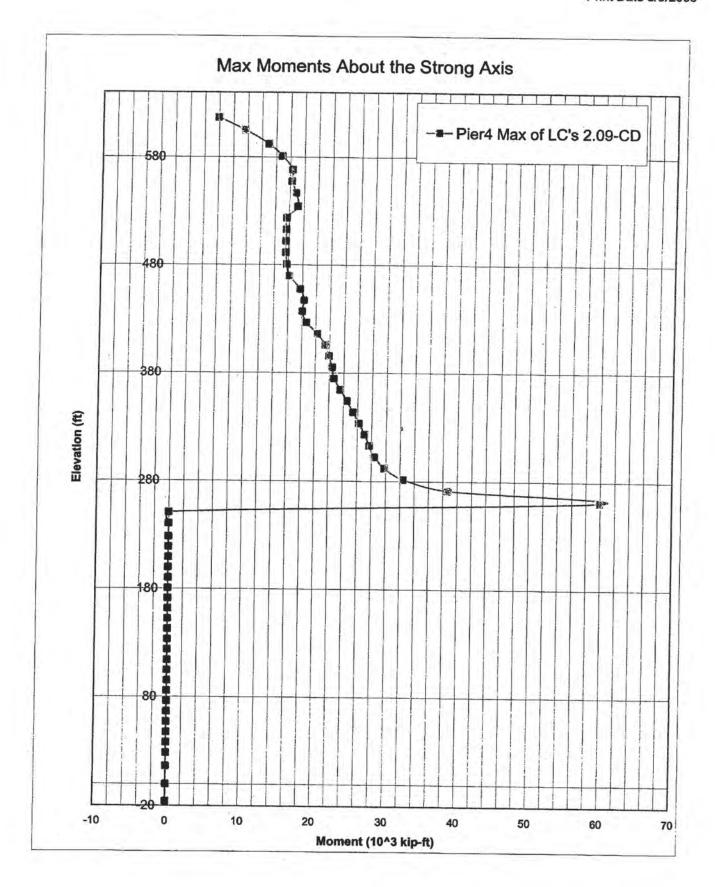
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Mak tips abs tipm group Live Load (40% corridor, 60% units)	Pier3 (3A & 1B)	Flow Game	61 Cap	So Mes	58 170			55 179	8 8	52 170	51 175	50 175	45 17	47 Mach	55 170	4 199	2 170	4 190	40 Typ	8 18 18 18 18 18 18 18 18 18 18 18 18 18	10 10	88 to 150	25	as Ing	31 175	88	28 70	27 770	8 10		22 22	27 72	20 170	25 ST ST ST ST ST ST ST ST ST ST ST ST ST	17 79	16 799	1 P	13 179	12 179	10 Ive	a Tyre	100	6 130	5 Typ	84	2 Moch	Printer	
Uhe Load	Pier3 (		L						_																																						Ground	

81.1-48

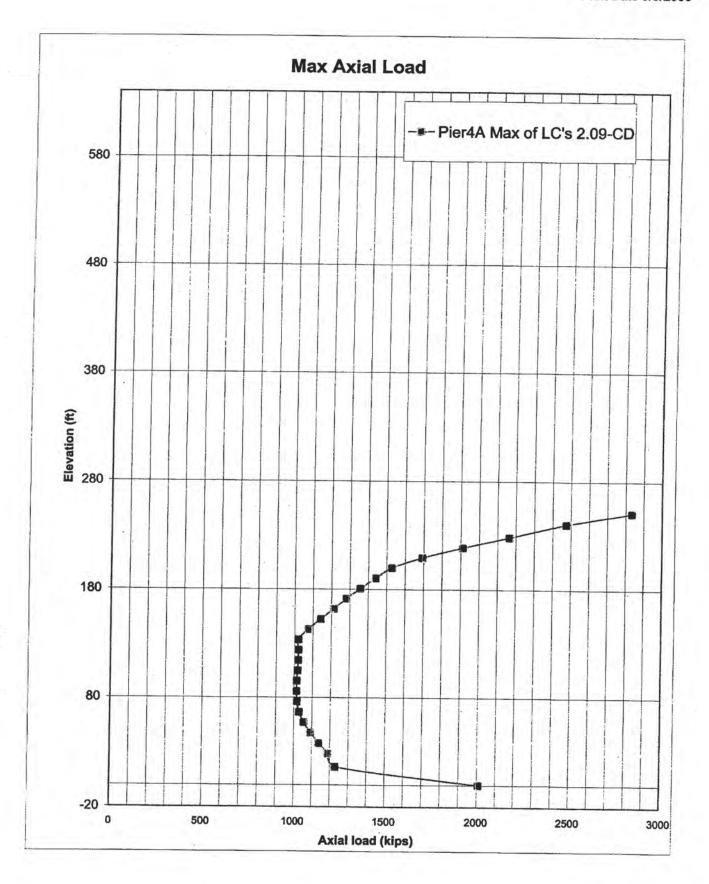


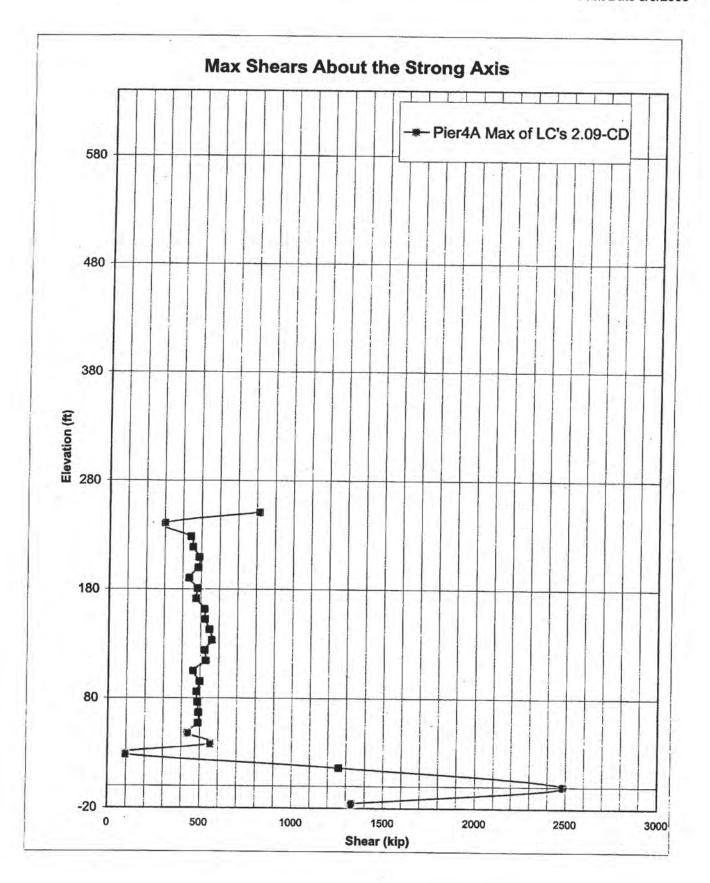
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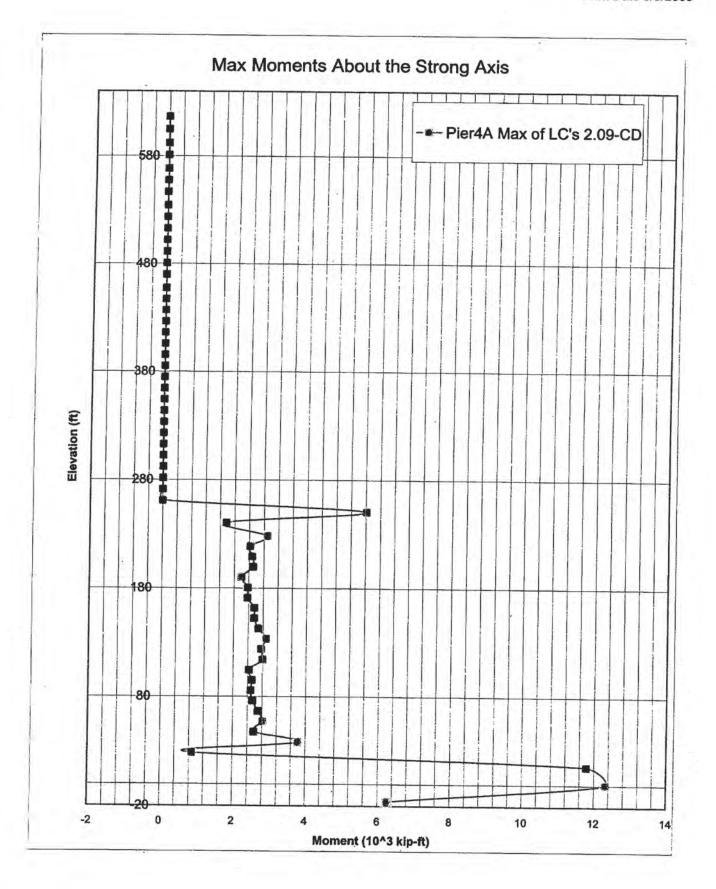


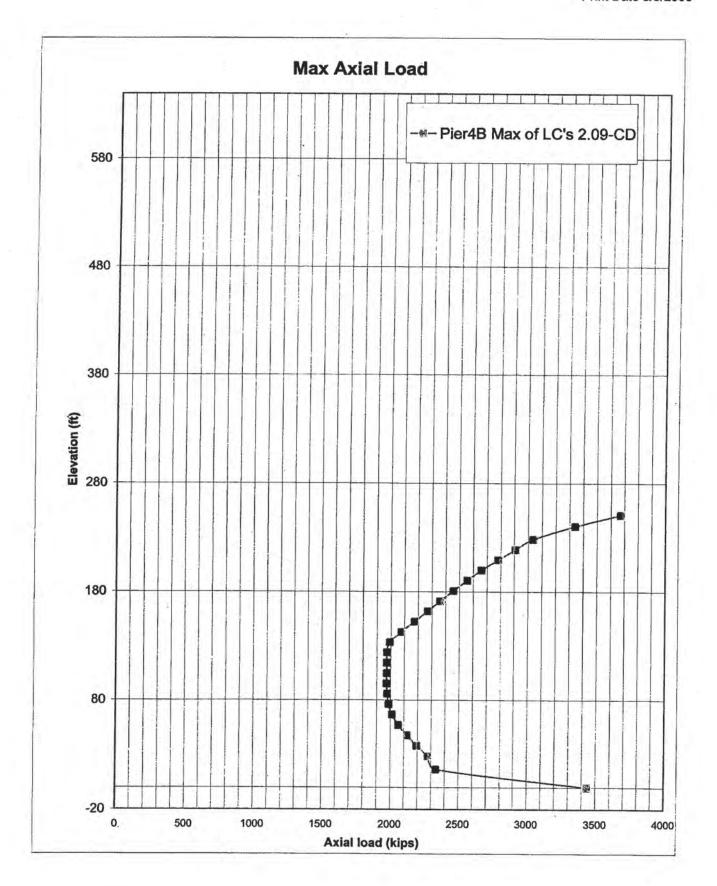
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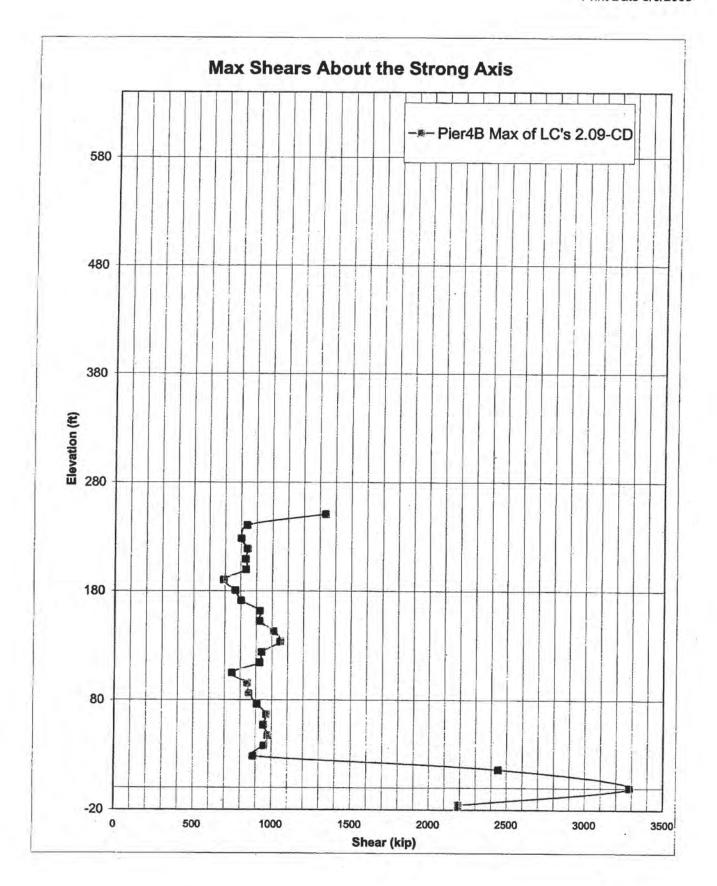




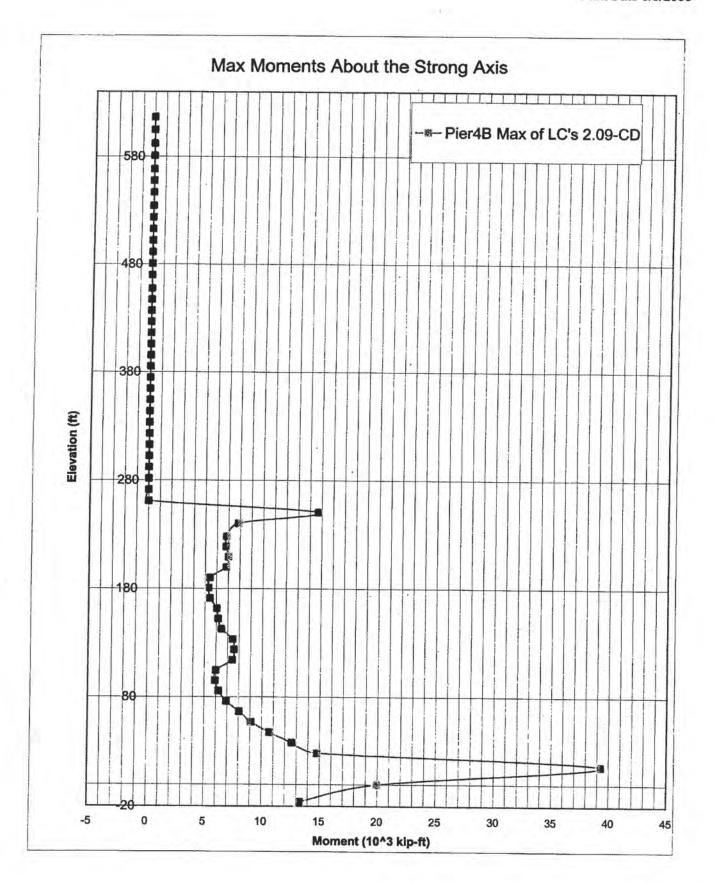
8.1.1-51 DODSONNOC00000446







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100		LL Wi	138	34	161	3	2.0	150	151	118	E	136	136	136	136	158	130	130	200	130	38	130	品	2	130	969	130	130	130	130	136	130	8	6.	8 8	8	8	8. 8	8 8	8 8	8	8 8	88	8	8	8.	8.8	28	8	8	8 8	115	157	149
Deficience Miles	8	Die 10	0	0	0	0 0	0 0		0	0	0	0	0	0	0	0	0	0	0 0	0 6	0	0	9	9	0	0 0		0		0 0	0 0	0	0	0	5 0	0	0	0 0	9 0	0	0	p 4	9 6	0	0	0	00	0 0	0	0	0 4	0	0	0
Ontringer C		No. of Lot	0	0	0	0 0		9	0	0	0	0	6	0	0	0	0	0	0 0	9 0	0	0	0	0	0	9 0	0	0	0	0 0	9 0	0	0	0	0	0	0	0.0	a e	0	6	0 0	9 0	0	0	0	00	0 0	0	0	0 0	0	0	0
Ontrience Or		Tengs.	0		0	0 0	9 0	0	0	0	9	0	0	0	0	0		0 1	0 0	9 9	0	0	0		0 (	0 0		0	0	0	0 0	0	0	0	9 60	0	0	0 0	0 0	0	0	0 0	0 0		0	0	00	9 0	0	0	0 0	0	0	0
Beam On				60	00	00 0		a des	ier	100		M	**	66		00	oin i	90 (	10 to	0 00	00	80	100				. 100	100	<b>*</b>	<b>10</b>		-	и			Þ	10				2		1		1	1					- 1	6		6
	Nav.	N. M.		1590			1590	0.654	0.654	0.654	0.654	0.654	0.654	0.654	0.654	0.654	9590	5850	0,654	0.654		0.654	1996	0.654	0,654	9590	1590	0.654	1590	9000	0.654	9990	0.654	1620	0.501	0.591	1650	0.591	0.50	0.591	0.591	1551	0.50	1650	0.591	1650	1650	0.591	1650	1650	0.591	0.391	0.591	0.591
Total Steel Link Bears	Total Lancett Land Stratut	Angels likel persons		12.6	12.6	17.6	12.6	12.6	12.6	12.6	126	126	12.6	12.6	12.6	126	12.6	12.6	17.5	12.6	12.6	12.6	12.6	12.6	12.6	944	12.6	12.5	12.6	12.6	12.6	12.6	12.6	126	12.6	12.6	126	12.6	12.6	12.5	12.6	12.6	12.6	12.6	12.6	12.6	126	126	12.6	12.6	125	12.6	12.6	125
Ĭ.	7		8	20	8	2 3	3	x	35	35	3	31.	8	3	3	0	30 3			. 35	.75	*					7	×	T 2		. *	*	*						٠											į.				E.
Flower Red	10		243	E	502	108	128	128	128	178	173	128	128	128	128	46.5 10	128	87	125	128	9 871	123	128		871	128 6	128 6	128 6	128 6	178	123 6	128 6	9 871	971	46.5 10	128 6	128 6	871	128	178	621	28 6	128	128 64	128 64	128 64	2 2	120	128 64	123 64	28 82	128 64	65 100	87.5 100
1		10.00	280	280	88	107	3627	4643	5889	6875	7691	\$706				-	13786	16891	16831	17849	5988	18861	96800	21912	2567	54966	97.65	16661	1000	5700	1055	1,000	3107	4164			6292	me.	140	2439	3476	6164	7859	1624	1998	9697					5505	57992	91 6200	900
	Tree A. Tree A.	10 th	280	8	9101	1016	1016	9101	1016	1016	1016	9101	9101	9101	1016	9101	9101	9101	1016						9010								1037			1087			-	-		1007		40	1037 4						2 440		200	2
8	TAN T	84. ft	286	573	500	3590	3590	4596	2005	8099	7614	8620	3625	10033	11637	12643	13649	15667	999	17672	87.981	19684	0640	2500	1200	4713	6145	67.25	06/12	0200	30748					36888	5764	9960	9660	2022	60049	62103	6130	1217	4818M	69211	51264	ď	Ξ	2888	15			
	SA T	far ng. ft ng. ft	286	288	98	900	9001	9001	1000	900	9001	9001	9001	900	900	900	900	inos.	900	-											1006					1027 3		1027 3	E	С.	1000			2	ũ,	1027 69	0.0		Ü	2 200	16	\$ 120	027 38	UZI SI
	T drawn		404	40	\$ 5	3	40	404	404	404	404	4	404	40	ğ	ğ.	200	1	404	404	40	8	5	5 5	1	404	404	104	5 7	104	4	*	200	101	303	308	303	303	303	£:	100	303	303	903	303	500	900	要	308	9 10	303	303	908	200
	Wide L	N.	2,	2	2 5	8	8	8	96	R	30	R :	8	30	8 :	8.1	8.5	2 5	10	26	30	9 1	3 1	2 2	2.3	R	R	2 :	2 8	2 2	30	2	2 5	2 5	R	2	2 2	2 8	25	R	2 5	2 9	30	30	30	R 8	2 8	R	R	8, 9	30	8	8 8	R
	Elevation	n.	1.123	618.7	2000	581.0	566.5	557.7	547.0	5345	523.7	513.0	3052	491.5	400.7	aran a	2012	635.9	428.5	416.2	405.8	395.5	227.00	2 796	354.2	343.8	333.5	3232	3075	282.2	281.8	2715	2012	240.5	228.3	218.6	100.5	1903	180.8	1713	167.8	142.8	133.3	123.8	114.3	0.50	868	76.3	65.8	57.3	38.3	28.8	000	3 44
	Fr. Ht. El	2	11.0	11.8	31.3	125	10.8	10.8	12.5	10.3	10.8	10.8	10.6	900	200	125	10.5	10.3	10.3	10.3	103	103	200			L		10.3			10.3		10.3		8.5	9 6	n e	47	8.5	8.5	2 4	9	92	9.5	n e	0 0	1 47	9.5	65	or or	100	122	15.8	-
6		Usage	Cap .	O Mech	A Ten	7 198	5 Typ	5 Typ	4 190	3 100	di l	200	130	200	dir.	Meca	Tun S	Te	3 139	Typ.	199	1	2	70	The	170	Typ.	200	100	2	17.5	L'A	52	1	Mech	di,	4.5	100	Typ	di.	1,12	Tre	al.	S.	2	100	170	100	di.	200	Lyn	102	Most	-
riera (4A & 415)		Floor	φ	(B) W	i il	6	in .	ad i	6	16	id i	in S	5	4	4.5	* 4	14	7	43	42	4	3 6	3 5	23	8	R	8	P 8	1 24	8	8	19 5	7 %	10	24	2 23	4 5	8	139	200	12	15	14	2	77	C.	6	eo.	1	9 40	*	10 0	4	
Piera		1																																																			Ground	-

#### 8.1.2 Coupling Beams

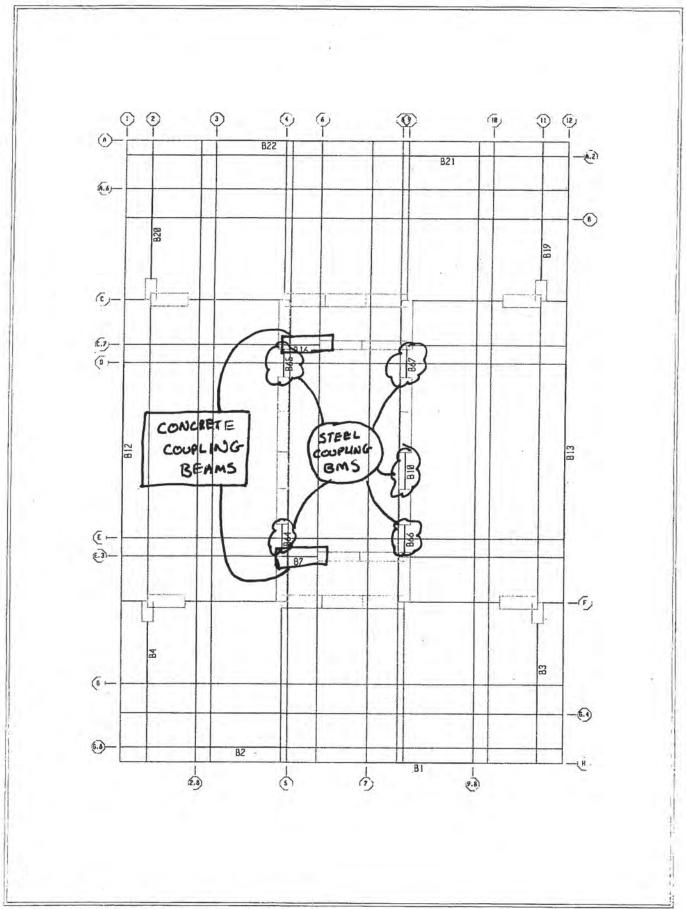
**Steel Coupling Beams** – Design forces were obtained by considering the seismic RSA analysis forces from ETABS combined with the gravity loads imposed by tributary area/PT reaction.

The 2002 AISC Seismic Provisions Part II, Section 16 contains requirements for Special Reinforced Concrete Shear Walls Composite with Structural Steel Elements (C-SRCW.) The steel coupling beams must meet requirements similar to that of the link portion of an Eccentrically Braced Frame (EBF.)

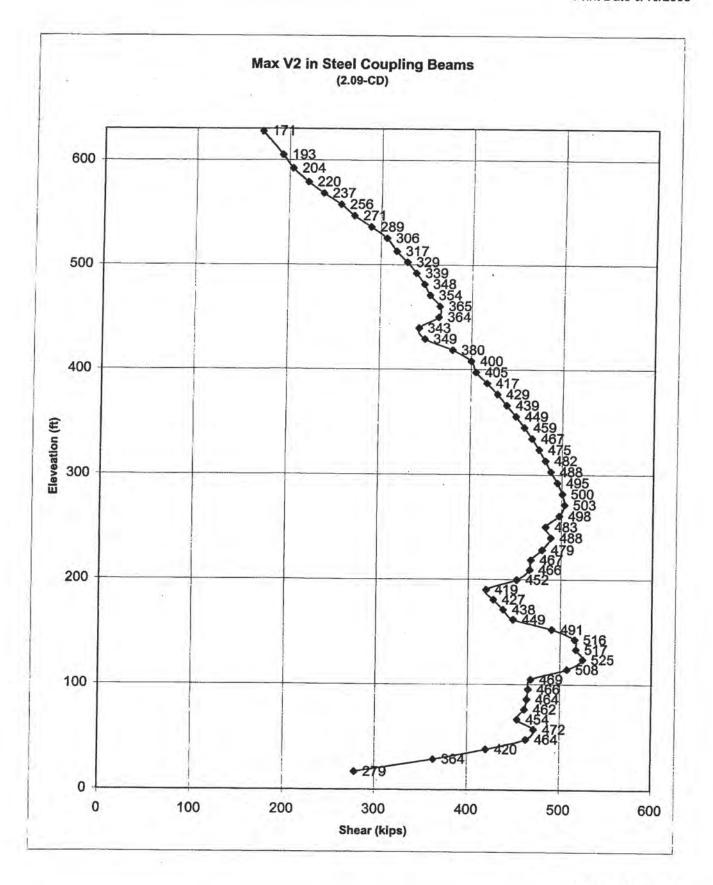
The AISC commentary cites research done by Harries, Shahrooz, et al. In EERI Earthquake Spectra, Volume 16, Number 4, November 2000, Harries, Gong, and Shahrooz provide guidance for the design of the embedded region and the mechanism of load transfer.

Requirements and provisions set forth in the above references were used in the design.

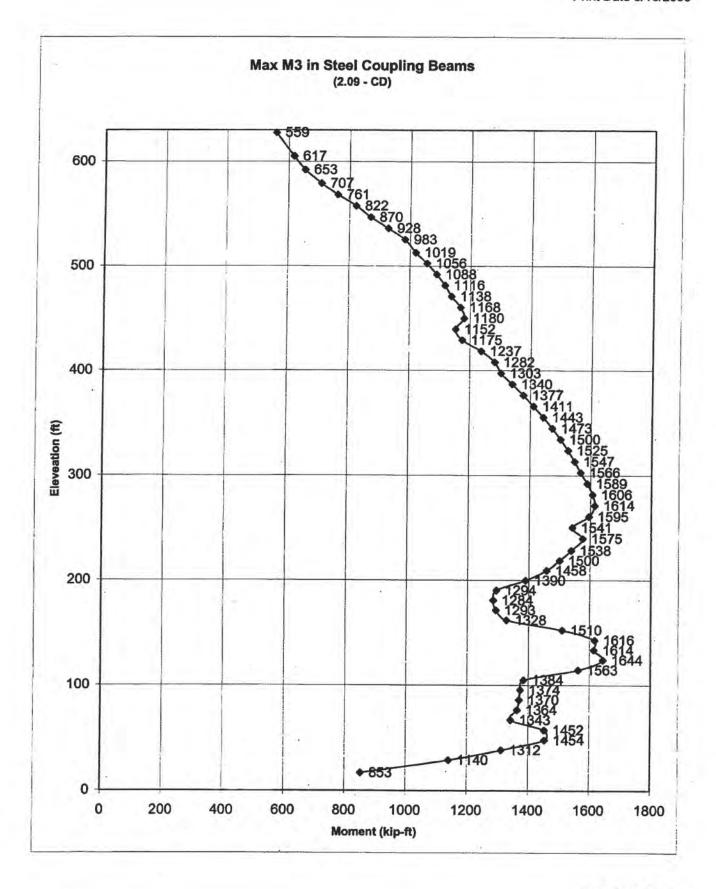
Concrete Coupling Beams – Beams on line E.3 and C.7 are designed as a SMRF conventionally reinforced beam. Some special conditions for openings occur in the core, and will be addressed at the building permit submittal.



ETABS v8.5.0 - File: 4069-20050509-2.09-CD - May 16,2005 19:16 Plan View - 24 - Elevation 228.25 - Kip-ft Units



V2 plot Chart 1 8.1.2-3



M3 Plot Chart 1 8.1.2-4

## **DESIMONE**

Project 301 MISSION

\_ Of\_\_\_\_\_

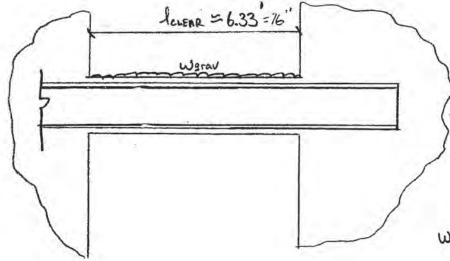
Project No.

STEEL LINK BEAMS

Date 1/27/05

Item

By NIR Ch'kd



CURRENT BEAM ASSUMPTION :

### GRAVITY PT LOAD

DL = 138 PSF

LL = \$60% 40 PSF

40% 100 PSF

\$64 PSF

TRIB AREA MAX = 545 Ft2

White = [.42(133 PSF) + 0.5(64 PSF)](545 F12)
= 19.01 K/f+

VSEISMIC MAX = 502 K

Vu = 1.40 + 0.5 L + E = 60.2 k + SOZ k

Vu= 562k

ΦVn= (0.90)(0.6)(50ksi)(Aω) = Vu=562k

.. Aw reg = 20.8 in2 where Aw = d-tw

W12x252 d: 154 to=1.4 Au=21.5in

CHECK MOMENT: Try W14 x 283 2x = 542 in3

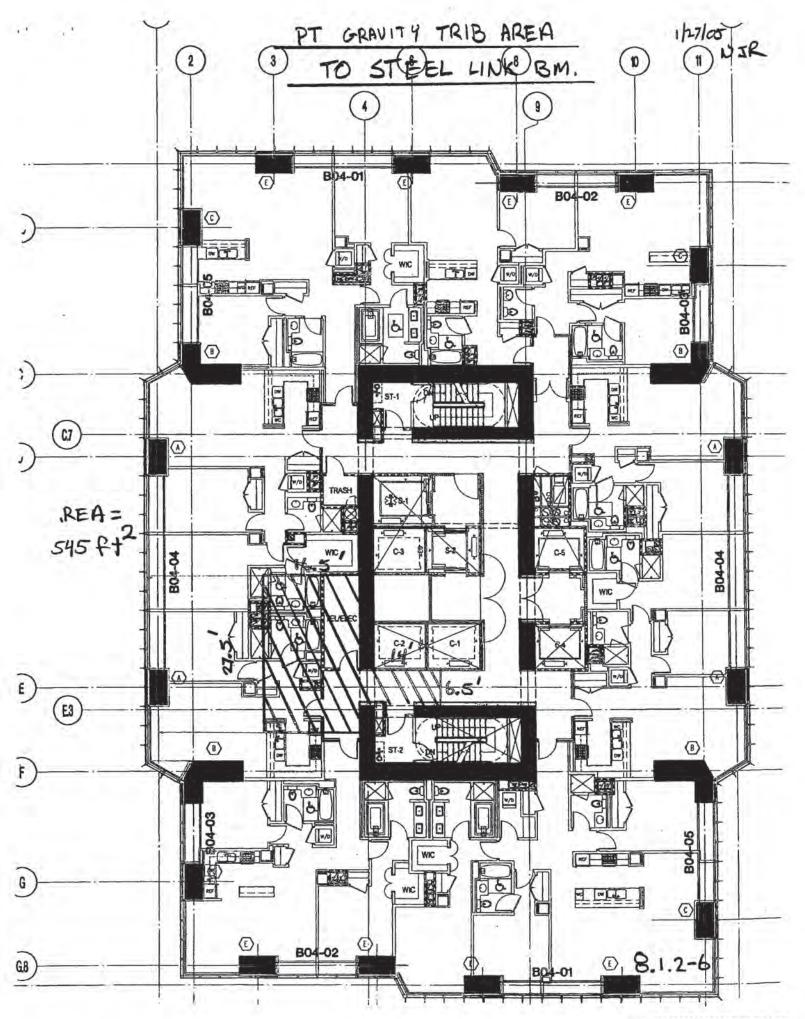
Mu = (1901 WH) (6.33)2 + 1661 k-ft

φMn = (0.9) (50 ksi) (542 in3) = 2032 K-ft

00 USE W14×31)

8.1.2-5

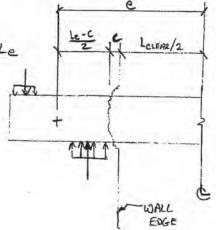
NG



## DeSIMONE CONSULTING ENGINEERS, P.L.L.C.

DETERMINE EMBEDMENT REQ.

MARCAKIS AND MITCHELL (1980) PROPOSES THE FOLLOWING
BASED ON RIGID BODY MOTION OF THE EMBEDMENT LENGTH Le



$$2 = \frac{\frac{1}{2} + c + \frac{1}{2}}{2}$$

$$= \frac{74^{"} + 1.5" + \frac{1}{2} + \frac{1}{2}}{2}$$

$$e = \frac{1}{2} + \frac{$$

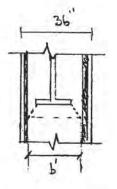
$$\frac{4}{4} - \frac{1}{4} = 8EARING WIDTH = 16SSER OF \begin{cases}
\text{CORE WIDTH} \\
2.5 \text{ bf}
\end{cases}$$

$$= 36' - 2c - 2\phi_{\text{STIRUP}} - 2(\frac{1}{2}\phi_{\text{#14bar}})$$

$$= 36'' - 2(1.5) - 2(0.625) - 2(\frac{1}{2}(1.693))$$

$$= 30.06'' < 2.5(16.6) = 41.5$$

$$b' = 30.1''$$



## PLUGGING Q, 3, AND A into () AND SOLVING FOR Le YIELDS:

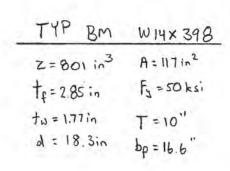
920 kips = 
$$\frac{0.85 (0.70)(12 \text{ ksi})(20.1")(\text{Le}-1.5")}{1 + \frac{3.6(\frac{1}{2}\text{Le} + 37.75")}{(\text{Le}-1.5")}}$$

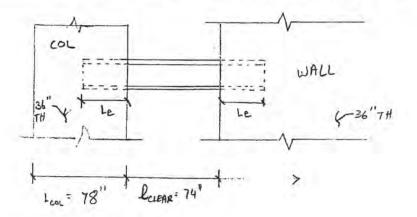
### DeSIMONE CONSULTING ENGINEERS, P.L.L.C.

PROJECT 301 MISSION PAGE OF DATE 6/28/04

DOE JOB# 40698

ITEM STEEL COUPLING BM IN E-W DIR BY NIR CH'KD





### CONCRETE DATA

(c) CLEAR COVER = 1.5" FOR WALL AND COL  
#5 STIRUPS 
$$\phi = 0.625$$
  $\phi = 0.70$   
 $\phi = 1.693$  ;  $f' = 12$  ksi  
PLASTIC MOMENT (Mp) AND WEB PLASTIC SHAR STRENGTH (Vs.)

effective link length (e)

$$L = l_{CLEAR} + 2(C)$$

$$= 74'' + 2(1.5'')$$

$$L = 77 lo$$

DETERMINE IF "SHEAR" OR "FLOWER" CRITICAL

$$\frac{M_{P}}{V_{SP}} = \frac{40,050 \text{ k} - \text{in}}{669 \text{ k}} = 59.86 \text{ in}$$

$$L = 1.28 M_{P}$$

$$V_{SP}$$

Since L & 1.6 Mp LINK IS "SHEAR" CRITICAL

LINK	BM	MARK	<b>LB2</b>			
Se	eismic (Y/N)	y	1.0	2002	2 500	1.5.0
	Mu =	500	k-ft	Mu=	6,000	k-in
	Width, b =	24	in			
Tota	al depth, h =	21	in			
CI	ear Cover =	1.5	in			
Sti	rrup size # =	5		Stirrup $\phi =$	0.625	in
	$f_c =$	10	ksi	$\beta_1 =$	0.65	
	$\mathbf{f_y} =$	75	ksi	$\rho_{\text{max}} =$	2.5%	§1921.3.2
	$\phi =$	0.90		$\rho_{\min} =$	0.4%	§1921.3.2

	§1921.5.1		Requir	ed for 1 layer ste	eel		Requir	ed for 2 layer	steel
	MIN COL h			Clear Spacing	Marc av. N			Clear Spacing for	
Bar	(in)	No.	As	for 1 row	ρ limits check	No.	As	2 rows	p limits check
3	9.4	45.5	5.00	> smin NG	OK	47.42	5.22	> smin NG	OK
4	12.5	25.1	5.02	> smin NG	OK	26.29	5.26	1.09	OK
5	15.6	16.3	5.04	> smin NG	ok	17.09	5.30	1.91	OK
6	18.8	11.5	5.06	1.06	ок	12.14	5.34	3.00	OK
7	21.9	8.5	5.08	1.65	ок	8.97	5.38	4.54	OK
8	25.0	6.5	5.10	2.44	ок	6.87	5.43	6.70	OK
9	28.2	5.1	5.12	3.39	ок	5.47	5.47	9.60	OK
10	31.8	4.0	5.14	4.80	ок	4.35	5.52	14.46	OK
11	35.3	3.3	5.16	6.53	ок	3.57	5.58	21.88	OK
14	42.3	2.3	5.21	12.05	ОК	2.53	5.68	66.91	OK
18	56.4	1.3	5.30	51.58	ОК	1.48	5.91	error	OK

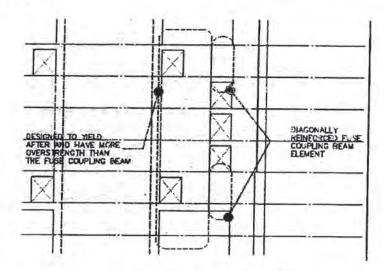
### Check above beam with specific reinforcing

Bar size # =	10	d =	17.11	
No. of bars =	6.00	a =	2.80	
No. of rows =	2	$\phi Mn =$	8077	k-in
Diameter =	1.270		673	k-ft
As total =	7.62			
P	1.86%	$Mu/\phi Mn =$	0.74	should be less than 1.0 to be OK
		Clear Spacing	7.97	in
		ρ limits check	OK	

$$\begin{array}{ccc} \text{fy for overstrength} = & 1.25 \\ a = & 3.50 \\ \text{Mpr=} & 10969 & \text{k-in} \\ & 914 & \text{k-ft} \end{array}$$

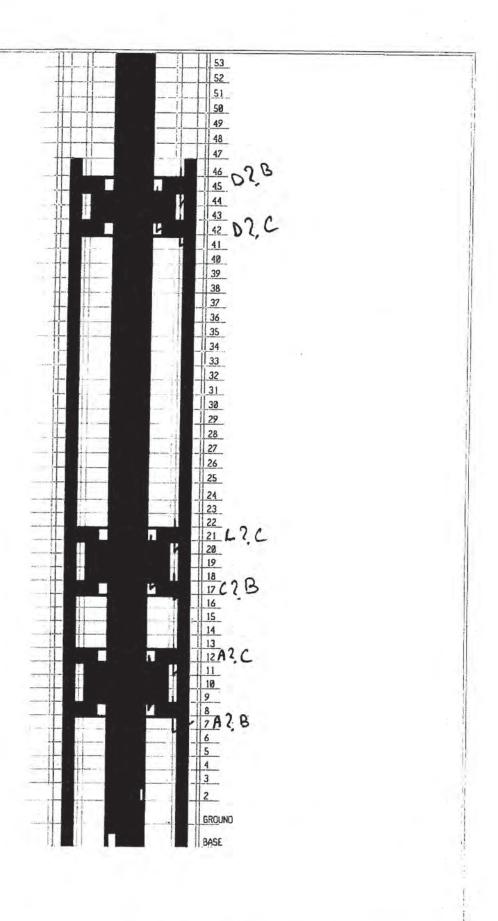
### 8.1.3 Outrigger and Outrigger Column

The outrigger shape has two distinct parts. The portion directly attached to the shear wall core (left side on the below picture) and the floor high sections attached to the outrigger column (on the right.) The part of the shear wall attached to the column is designed as a diagonally reinforced fuse coupling beam element. The outrigger is geometrically proportioned to yield first in this element. The other portion, attached to the core is designed to resist the capacity of the fuse coupling beams.



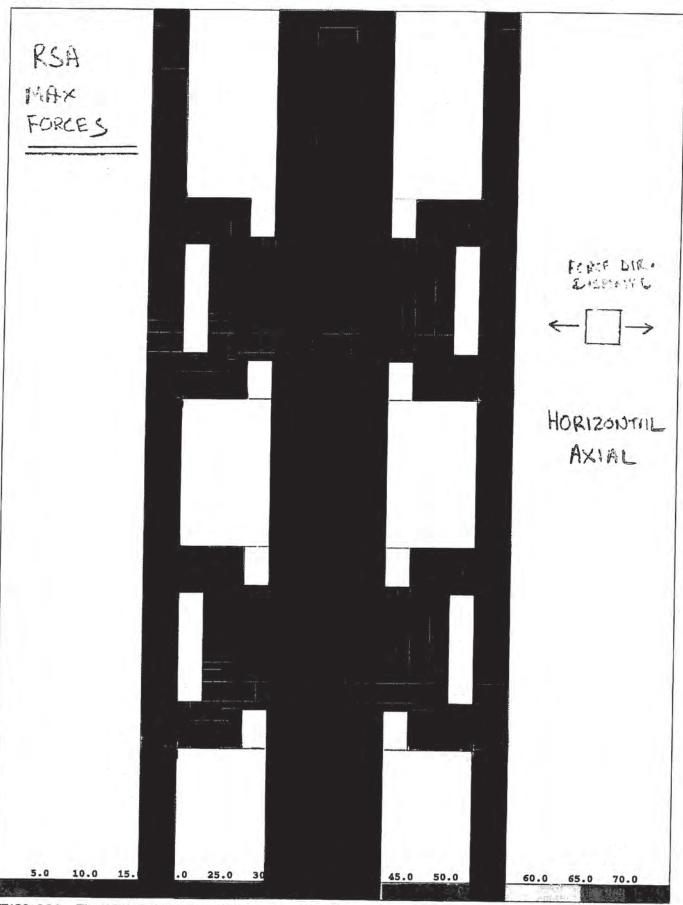
The outrigger column is designed for the capacity of the system. Outrigger column axial force from the code based design earthquake (with orthogonal effects, i.e. 100%+30%, and with torsion) is only 35% of that produced by the outrigger system. Since this outrigger column is part of the dual system as well as the primary system, it was designed for the conservative load case of  $2.8^{\circ}P_{\rm E}$  instead of the less conservative required code level force. Torsion on the L-shape column was also checked, at the request of the peer reviewer Middlebrook + Louie of San Francisco.

Design force Method	Axial (kips)
Code Design Force ( $P_E$ ) from ETABS =	9,200
Over-strength/System Capacity (Use Smaller of)	
1) 2.8 * P <sub>E</sub> =	2.8*(9,200) = 25,800
2) V <sub>pr</sub> of Outrigger Beams * 6 Locations=	6*(6,000) =36,000



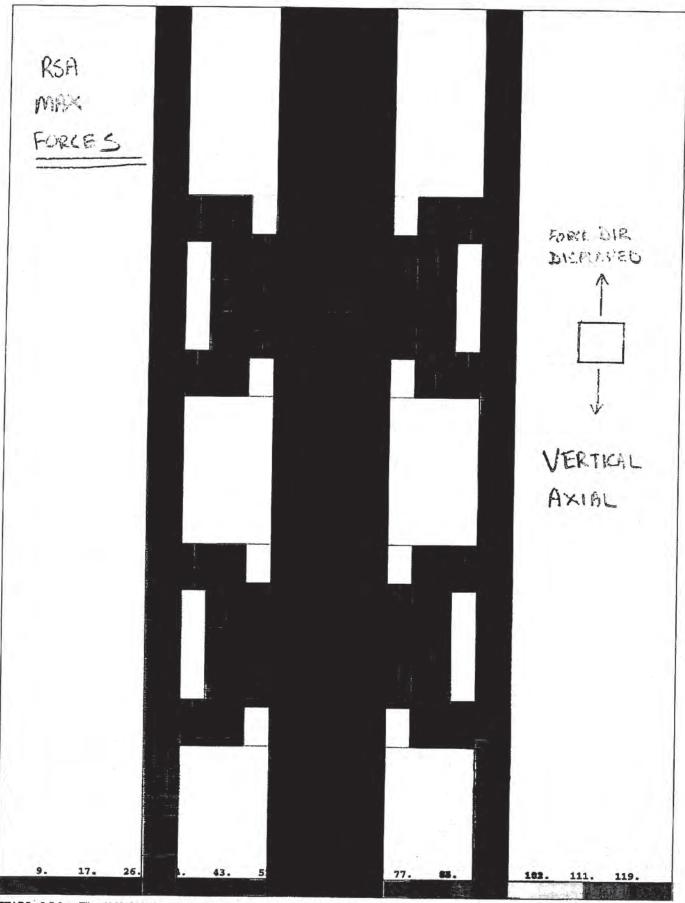
	Story	Spandrel	Load	Loc	Р	V2	V3	T	M2	МЗ	
A?A	12	A1A	E100X530Y MAX	Left	630	5574	13	503	22	10815	
A?B	9	A1B	E100X530Y MAX	Left	2167	3843	10	179	57	12966	- GOVERNS
A?C	13	A3C	E100X530Y MAX	Left	794	1910	21	202	82	8036	FUSE
											Design
C?A	21	C3A	E100X530Y MAX	Left	573	5008	16	563	31	8744	
C?B	18	C3B	E30X100Y5 MAX	Left	1577	2956	10	253	68	9654	
C?C	22	C1C	E100X530Y MAX	Left	971	2051	17	199	72	8736	
D?A	45	D3A	E100X530Y MAX	Left	273	4318	9	443	25	7708	
D?B		D3B		Left	1355	2552		193	61	9134	
D?C		D3C	E100X530Y MAX	Left	811	1803		119	44	8028	

Etabs Model: 2.09-CD



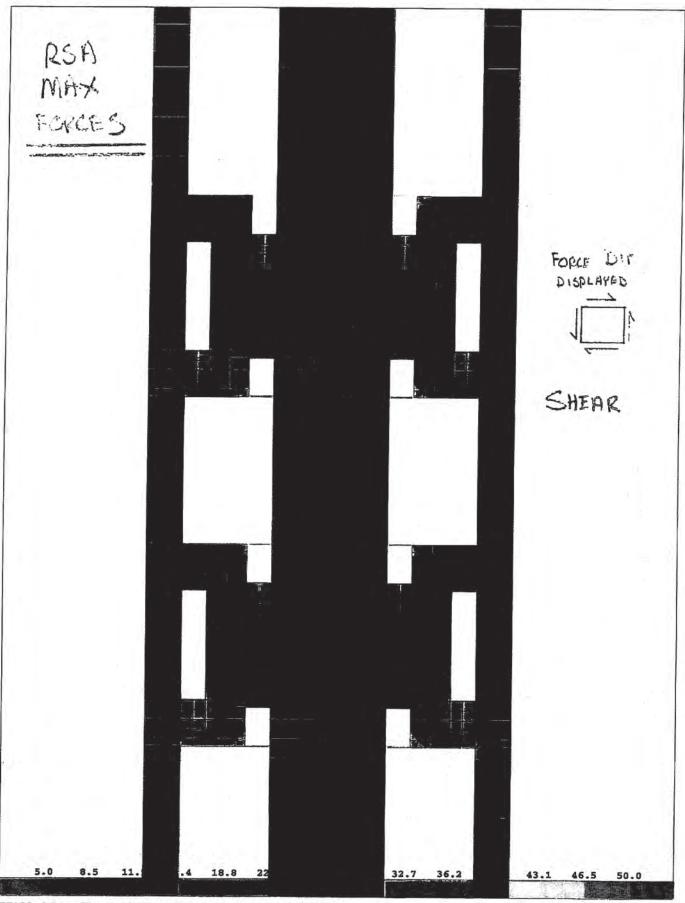
ETABS v8.5.0 - File: 4069-20050504-2.09-CD-TEST2 - May 4,2005 14:54 Elevation View - C Resultant F11 Diagram (E100X530Y) - Kip-in Units

8.1.3-4



ETABS v8.5.0 - File: 4069-20050504-2.09-CD-TEST2 - May 4,2005 14:55 Elevation View - C Resultant F22 Diagram (E100X530Y) - Kip-in Units

8.1.3-5

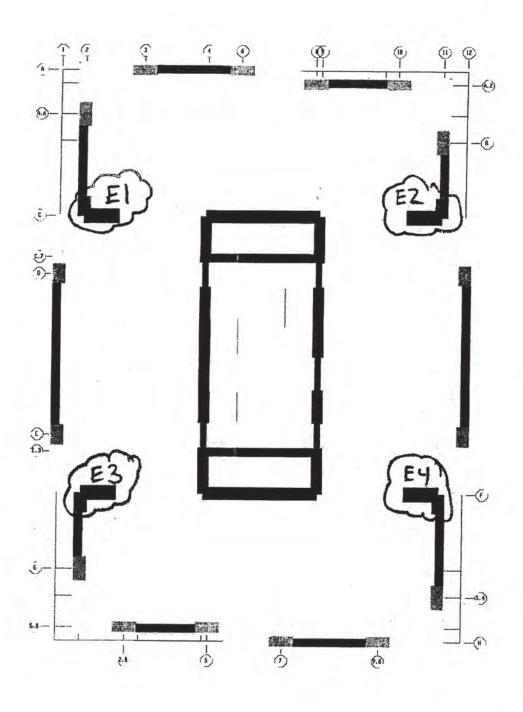


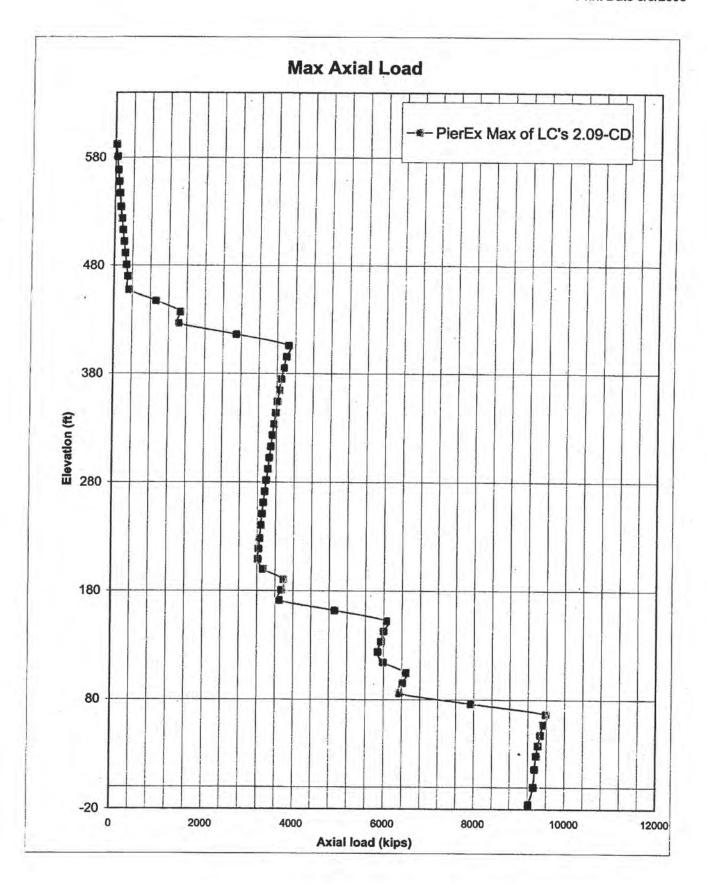
ETABS v8.5.0 - File: 4069-20050504-2.09-CD-TEST2 - May 4,2005 14:51 Elevation View - C Resultant F12 Diagram (E100X530Y) - Kip-in Units

Project	301 Mission
Job#	4069
Date	5/20/2005
Etabs Model	2.09-CD

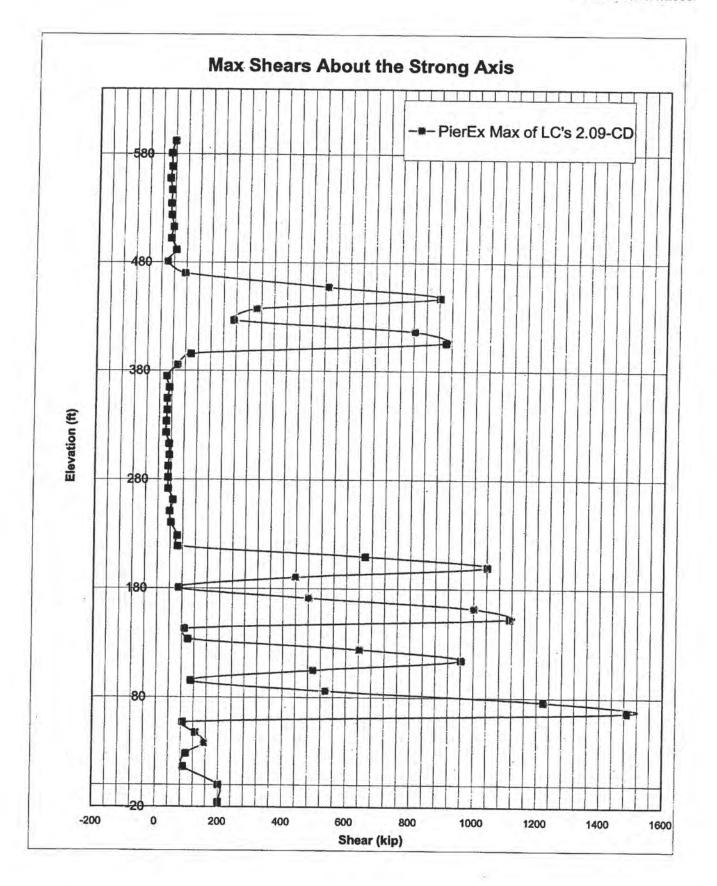
Coupling beam design for outrigger connected to col D at L8 72 in 124 in bw 36 in fc 10000 psi Vu 3843 kips Mu 12966 k-ft 37.44 in^2 Avd Total area of steel in one diagonal fy 75 ksi 24#11 In/d 0.6 4sqrt(fc)bw\*d 1786 kips 1.04 rads 59.9 degs Mn φ-factor 14570 k-ft Avd\*fy\*d\*Cos(a) 0.9 φMn 13113 k-ft 2\*Avd\*fy\*Sin(α) Upper limit (UBC equ. 21-11) 4857 kips 4464 kips 10sqrt(f'c)bw\*d Vn φ-factor 4464 kips (UBC 1921.6.10.2) 0.85 φVn 3794 kips OK 1.01 NG 0.99 OK Check if conventional reinforcment is possible: (ACI-99 21.6.7.3) Requirement Vu<4sqrt(f'c)bw\*d Vu/sqrt(fc)bw\*d<4 if less than 4, conventional CAN be used What is Vp? 1.25fy = 93.75 ksi 2°Avd\*fy\*Sin(a) 6071 kips

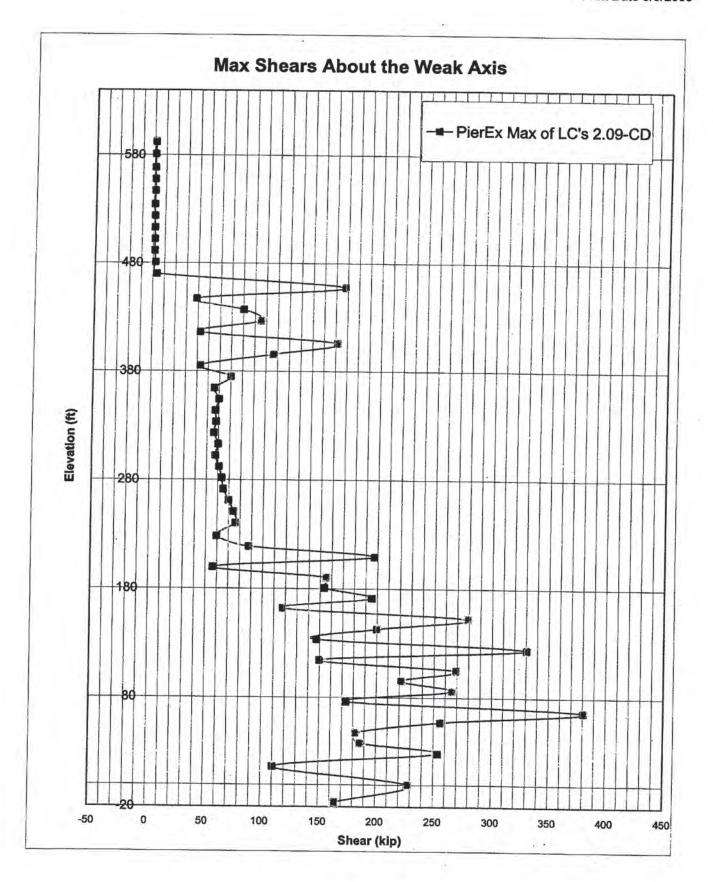
# OUTRIGGER COLUMS

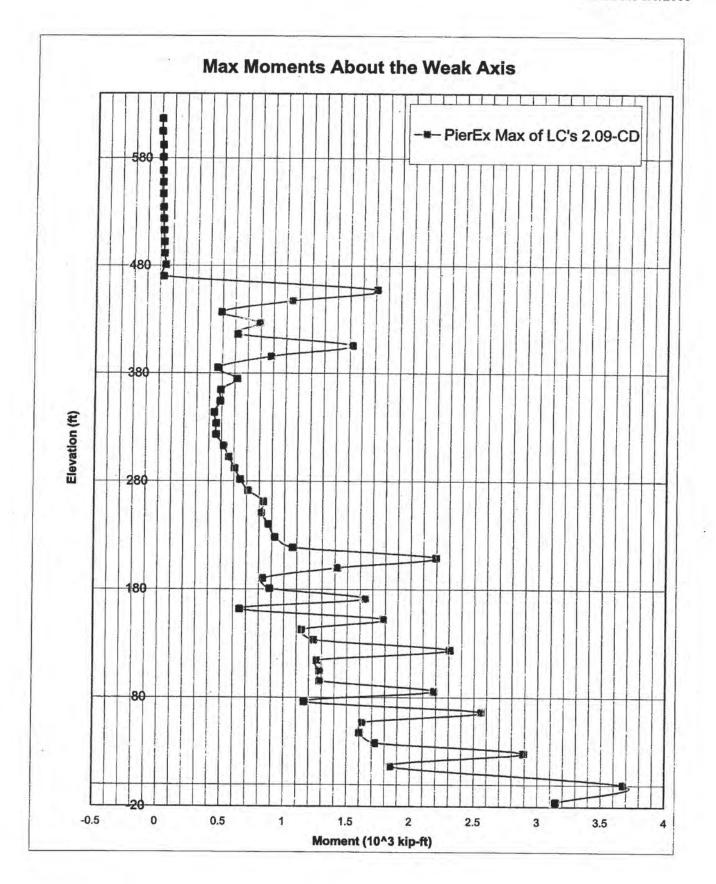




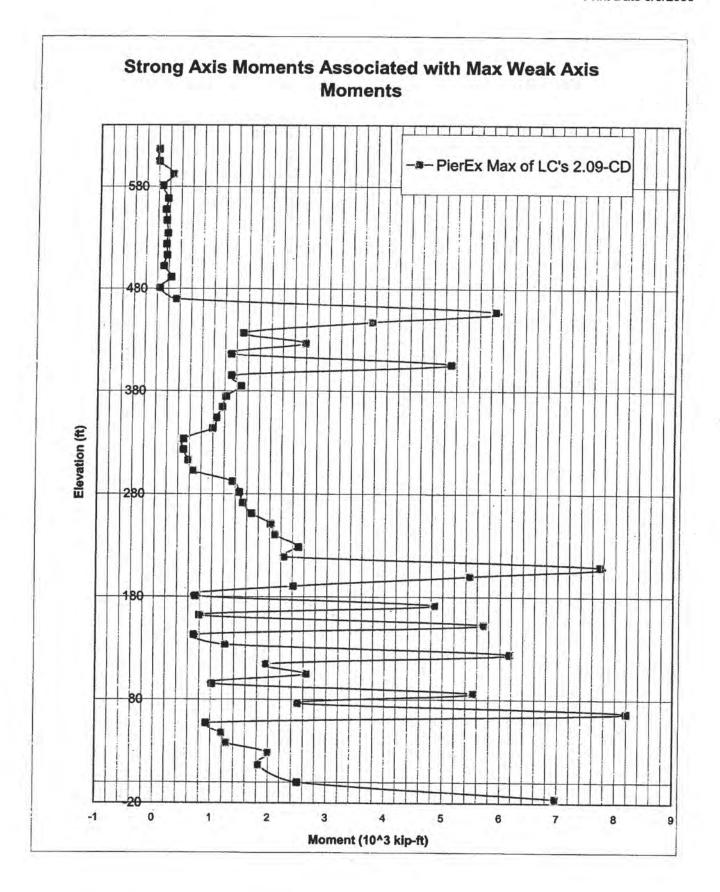
8.1.3-9

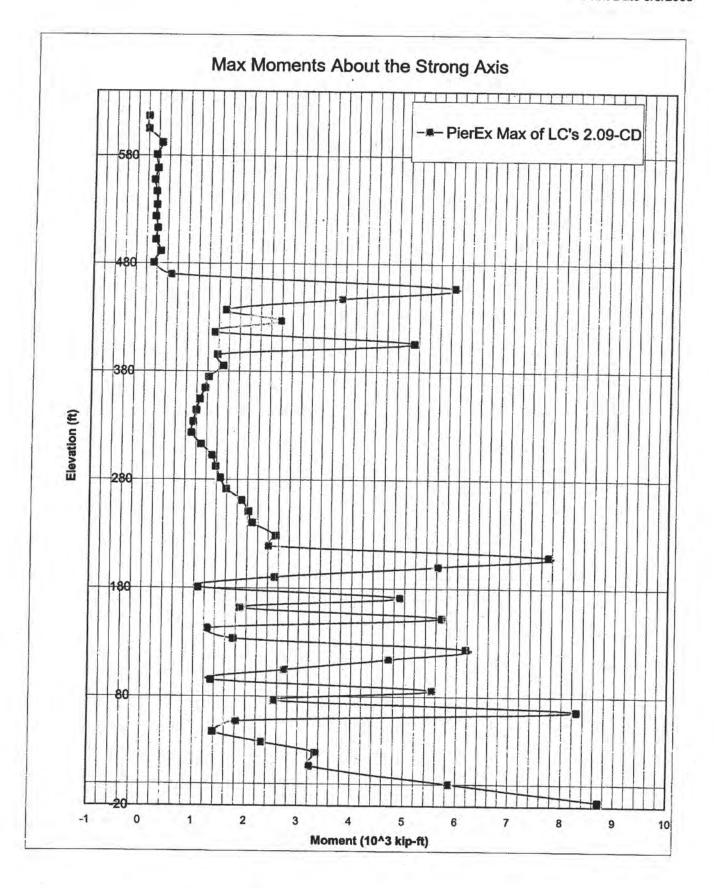


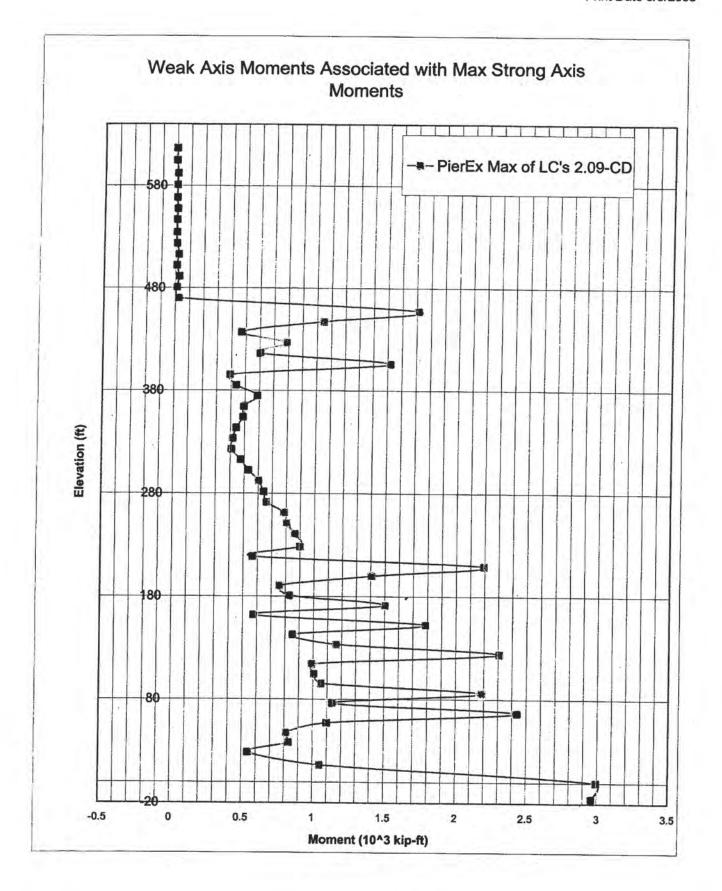




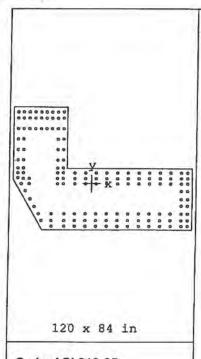
45 45







# C .	I Walmber Tf.	Design	Mps	207	ð	165	5	752	1070	1237	1396	1553	1711	1808	2773	2602	2717	2962	3358	3554	3749	3945	6369	175	4742	4942	5344	37.5	5745	8165	6347	654T	5967	7181	7358	7620	8008	222	8669	2383	9026	9499	9775	1566	10171	10565	23001	65601	11162	11689	11000
				· 00		20	===	w e	2 2		750	59	<b>S1</b> 4	n ×		'n	9	90 H	9 90	-	r.	9.4	2 5	y on	9	00 +	0.0-		9		. 0	*	A 4	100	-		-						, pai	len i			. 164	, se			
1D	П							2 416		Ĭ,			942	99		I		163								2758												4692		5021		ű		5648					634		
¥ 5	142/dinben St	Design	Min Min	200	4	57	E C	22.53	TOBUL	118	151	149	15.	175	217	2397	761	2835	3242	343	3629	30100	401	4400	4897	479	518	337	557	5064	6159	569	6773	1969	717	7389	7824	8036	8412	8600	8788	9226	2445	6996	9878	19000	10453	1064	10842	11348	414.70
100	+-	_	SIF.	146	100	417	536	15	340	372	186	5601	1206	1227	1091	1756	9161	2076	23.70	2508	2547	2786	1900	3208	3350	2492	37.5	3917	4059	4341	4483	4626	4976	2005	\$226	5343	3695	2582	6125	6262	6669	67172	6872	7032	7188	7467	3806	7745	7889	90	94.41
9	١.		5	137	279	388	6	6 8	202	811	916	1018	1121	1	1491	6091	06/1	1947	2220	2351	2482	2612	3878	3011	3145	3278	3546	3686	3814	2048	4216	4350	1000	4771	4916	2016	3965	5511	5768	5897	9009	6323	8478	8299	6776	TUSS.	RIL	1301	7457	71017	
-	Cam Rad.	14	100	9	12	H	31	5 %	1	3	19	2	22	79	1 20	H	116	122	133	139	145	150	Z 19	167	174	2 2	76	200	202	221	227	27	258	364	112	284	182	Ø 3	311	318	335	338	38	352	358	177	378	38	392	513	
	IL O		1 80	1.15	15'0	0.69	050	200	98.0	150	35.0	690	0.48	247	346	24	140	243	290	242	141	141	140	140	1.40	740	140	740	9	100	18	999	0.40	0.40	0,40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	3	240	81	8.8	1 9	95	81	9	
		% multiplier									Ĉ				Ĭ	ĺ			ĺ	Ā	Š	7.0		-		Ÿ	Ź	2			9																			, 6	-
	Out		Sec.	. 49	11	22	8 8	2 5	38	102	130	138	155	101	733	249	266	200	316	333	350	367	900	417	70	451	484	105	518	25.	898	585	1 10	199	150	160	728	7.5	1 E	795	80	345	3	878	968	716	3.	596	626	1038	ALVAN .
[	18	70	ad in	3	289	200	6	8 8	734	818	923	1927	1132	957	1496	1646	1800	1953	2236	2366	2503	9092	2000	3041	3176	3311	3582	3117	3852	4122	1257	4392	8004	4808	4955	2525	5407	5554	5814	1765	8074	6374	6528	1899	05830	2000	7278	1360	1687	7845	200
Mex	JA		100	2	140	901	0 1	8 2	110	104	20	20	2 3	5 2	28	8	5	<b>X</b> 5		133	-	8	2.0	3.50		135				3 2		81	1 20	130	all.	13 13	100	20.3	30	130	2 5	100	153	153	641	3 5	133	133	137	2 6	200
4	å		200	B	268	38	467	200	99	757	853	950	68	1340	1386	1528	1674	0281	2087	2212	2337	2462	2717	75.	1821	2008	382	348	158	3861	3989	4116	438	4507	4645	4788	XOZ	2213	5457	5579	1005	1985	6131	6276	5417	24.5	166	9169	1945	7376	2
	Self Tetal	Wr DL	Mary or	32 67	136	18	3 5	2 2	201 - 34	0.07	76 01	00	6 6	200	146	1 142	146	91 1	13	125	125	133	1 127	1 127	1 127	1 127	127	1111	127	121	123	171	011 9	6 122	6 136	163	9	9 139	6 122	271 9	2 12	5 155	E 150	5 145	141	125	B	5 125	129	184	-
	Mile. S		N Sell				73.5		,	100			60.10			40	0		, 40	-0			B 40		10	40 10	9 49	9		0.0	9	91	4 47	5	44 1	n 4	145	× •	4 10	o,	31 6	* 94	or	37	25.0	5 95	35	45	9 7	2 2	-
	Outrieser N		100	0	0	0	0 0	0 0	0	0	0	0	0 0	0 0	0	11	17	2 5	0	0	0	er c	00	9	0	0 0	0	0	0	0 0	0		0 0	0	11	3 75	H	17	0	0	0	7	7	7	1	5 0	0	ō	0.0	6	
	Outringer O		9	0	0	0	00	00		P4		**	un v	0 1		2,936	3,643	3,043	0	0	0	0 0	0 0	105	0	0 0	0	0	0 0		0	0	0	0	2936	3643	3.643	2,936	0	0	0 0	3643	3,643	3,643	2936	8 6	0	4	00	0	
	Outriener O		a c	0	0	0	9 9	0.0		0	0	0	0 0	9 12	0	5.7	2	25	0	0	0	00	0	0	0	0 0	0	0	0 1	0		0 0	0 0	0	27	22	27	S		0	0	2.2	5.7	5.7	5,7	0	. 0	0	0 0	0	
	Exterior O		E T	2.6	2.9	7	7.	24.	2.8	2.4	2.4	2.4	2.4	7.4	2.8	77	23	75	12	53	573	7:	7 77	23	7	77	12	13	7	12	23	73	77	71	77	17	12	7 7	17	2.1	17	517	2.1	77	22	1 7	17	17	22.	1	
	Bean E	DE W		*	m .		<b>70</b> 0	e) 00	100	*	*		in the		60		NY 1	с и	**	oil	*		9 9	90	0.5	2 9	10	2	9 5	10	10	2 2	9	10	01	2 2	01	2 5	9	30	2 2	2	13	2:	2 :	2 2	n	9	2.5	2 12	
	SMRF Beam	Irib Length WL	=	9 0.900	060 6	9 0,900	9 0.900	9 0 500	9 0.300	9 0.900	9 0.900	9 0 900	9 0 900	0060 6	005'0 6	00570 6	9 0,500	0.900	9 0,500	9 0.500	9 0.900	0 1 175	91.125	9 1.125	9 1.123	9 1.125	9 1.125	9 1.125	91.125	9 1.125	9 1.125	9 1.125	9 1.125	9 1.125	9 1.125	9 1.125	9 1,125	9 1123	9 1,125	9 1.125	9 175	9 1.406	9 1 406	9 1.406	9 1,406	9 1.406	9 1.406	9 1.406	9 1.406	9 1 406	
	Red.	20.42	8	25	2 2	2 5	2 4	2 2	9	- 04	*60	9:	4 4	2	8	40	9 49	2 8	9	9	2:	3 5	\$	9	9 5	2 9	40	04	3 5	8	9	2 5	2 8	0	0 9	9	9	2 9	3	9	2 5	9	9	9 5	9.6			9	0 0		
d	Floor B	10	245	371	508	121	225	7 2	128	128	128	57	128	121	1465	20		977	128	128	27	178	1 27	178	173	128	128	128	99	2	123	2 2		128	128	9 29	128	2 2	12	٥.	178	. 00	128	128	128 A	128	123	128	22.	146.5 100	
	Chim	TribA.	159	318	8	100	180	333	2831	3333	3836	4339	100	5846	6328	6810	E E	8235	8736	9218	8696	10761	4	92911	12108	1001	3552	大学	4000	5479	1965	2009	107		1369	9332	19814	919	85712	1740	2002	23185	1998	24148	66.50	8883				27991 14	
Max			159	139	8	2 5	100	8	5003	203	503	9	2 25	2005	482	91	785	1 2	482	482	2 5	482	40	2	29	1 24	482	482	707	482	0	40		482	\$ \$		-	100	123	77	457 2	89		422					478 72		
	Cum	PAA. I	138	275	716	1691	1897	2038	2479	2519	1360	3800	462	5122	3542	1962	1000	7220	7640	9060	2475	9319	97.38	10158	10801	11417	11836	1256	13005	3515	3934	1474	5193	5613	16453	16872	1281	18131	18550	878	9809	60738	20648	990	907	125	141	99	23532	24414	
Min		MAR I	138	138	7 5	1 3	1	4	77	1	3	3 3	īī	14	420	8 8	400	420	420	900	200				84			8 8	26			30			420	26	84			620				420 20					416 23		
1		Cayth T	31.4	\$13	51.4	51.4	514	51.4	51.4	51.4	51.4	21.4	51.4	51.4	22	75.3	25.5	23	253	753	763	2	73.3	153	75.3	75.3	75.3	73.3	38.3	75.3	75.3	253	153	753	75.3	153	753	32	23	75.3	223	153	22	53	2 22	15.3	53	23	9.87	18.5	
bel .		Width Leagth Trib.A. Trib.A. Trib.A.	51.4	51.4	51.4	51.4	51.4	51.4	51.4	\$1.4	51.4	614	51.4	51.4	75.3	753	75.1	75.3	75.3	75.3	35	333	183	75.3	22	73.3	35.3	75.3	353	75.3	75.3	75.3	15.3	753	22	75.3	753	Ť	Ö	25				255	753	75.3	753	753	78.6	78.6 7	
64 ps				616.7	0000	381.0	685	1.13	547.0	34.5	523.7	502.2	481.5	480.7	170.0	457.5	436.8	428.5	416.2	999	852	74.8	64.5	54.2	13.6	123.2	12.8	3025	100	271.5	51.2	240.5	28.3	8.8	2 50	803	180.8	1.8	523	142.8	123.8	114.3	104.8	85.8	76.3	6.8	57.3	67.5	26.8		
	(	Fir. Ht. Elevation		11.8							10.8					iu.	ń	ŭ		103			10.3 3			Q.						123 20		9.5	9.5		95 18			95				0.00				9 6 8 9 9			
	umns	94		Mech				ĵ.	L)D	n		1	100		9	95		100	ij		200					Par and						I've I			Par Charles		dill de la		i							p.		p.		6	
Live Load (100% units)	Ex (Outrigger Columns)	Floor Uses		8 I		300	8	18	Z.	93	25	9	49	. 33	47 M	8 4	14	43	4	4	200	38	37 1	18 3	8 8		8					N N	100			8 2	0 0	17 17	16 Typ	10 139	13 199	12 Dp	TO THE	9 Tvp	8	7 13	6 6	0 1	3 100	2 Mech	
Live Load	Ex (O.																																																		



Code: ACI 318-95

Units: English

Run axis: Biaxial

Run option: Investigation

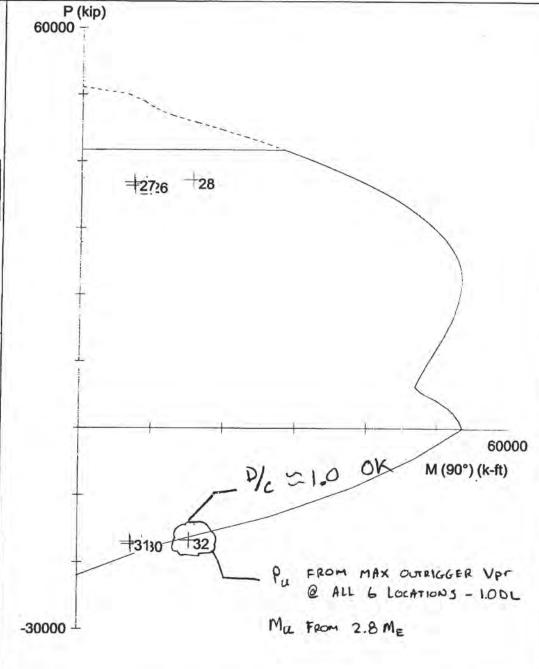
Slenderness: Not considered

Column type: Structural

Bars: ASTM A615

Date: 05/16/05

Time: 15:21:17



#### PCACOL V3.00 (PCA 1999) - Licensed to: Licensee name not yet specified.

File: F:\PROJECTS\4069\PCA\TOWER\CURRENT\3-OUTR~1\LB1.COL

Project:

Column:

Engineer:

fc = 10 ksi

fy = 75 ksi

Ag = 6216.51 in^2

145 #14 bars

Ec = 5000 ksi

Es = 29000 ksi

As = 326.25 in^2

Rho = 5.25%

fc = 8.5 ksi

e\_rup = Infinity

Xo = -0.00 in

lx = 2.86252e+006 in^4

e\_u = 0.003 in/in

Yo = -0.00 in

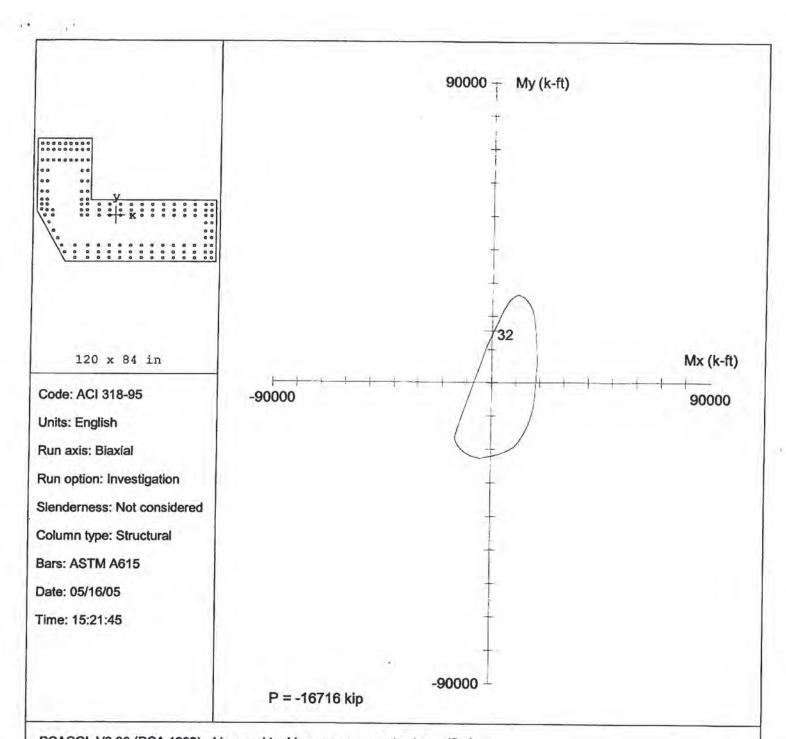
ly = 7.57655e+006 in^4

Beta1 = 0.65

Clear spacing = 1.61 in

Clear cover = N/A

8.1.3-17



#### PCACOL V3.00 (PCA 1999) - Licensed to: Licensee name not yet specified.

File: F:\PROJECTS\4069\PCA\TOWER\CURRENT\3-OUTR~1\LB1.COL

Project:

Column:

Engineer:

fc = 10 ksi

fy = 75 ksi

Ag = 6216.51 in^2

145 #14 bars

Ec = 5000 ksi

Es = 29000 ksi

As = 326.25 in^2

fc = 8.5 ksi

Rho = 5.25%

e\_rup = Infinity

Xo = -0.00 in

Ix = 2.86252e+006 in^4

e\_u = 0.003 in/in

Yo = -0.00 in

ly = 7.57655e+006 in^4

Beta1 = 0.65

Clear spacing = 1.61 in

Clear cover = N/A

#### General Information:

File Name: F:\PROJECTS\4069\PCA\TOWER\CURRENT\3-OUTR~1\LB1.COL

Project:

Column: Code:

ACI 318-95

Engineer: Units: English

Run Option: Investigation

Run Axis: Biaxial

Slenderness: Not considered Column Type: Structural

#### Material Properties:

= 10 ksi = 5000 ksi Ec

= 8.5 ksifc

Ultimate strain = 0.003 in/in

Betal = 0.65

fy = 75 ksi

= 29000 ksi Es

Rupture strain = Infinity

#### Section: =======

Exterior Points

No.	X (in)	Y (in)	No.	X (in)	Y (in)	No.	X (in)	Y (in)
								1 (111)
1	-33.3	-31.7	2	67.3	-31.7	3	67.3	10.3
4	-16.7	10.3	5	-16.7	52.3	6	-52.7	52.3
7	-52 7	2 8		2.5	2-3-5-5		-025.0	02.0

Gross section area, Ag = 6216.51 in^2

 $Ix = 2.86252e+006 in^4$  Xo = -0.000736446 in

 $Iy = 7.57655e + 006 in^4$ Yo = -0.000717949 in

#### Reinforcement: ----------

Rebar Database: ASTM A615

S	ize	Diam (in)	Area (in^2)	S	ize	Diam (in)	Area	(in^2)	S	ize	Diam (in	n) Area	(in^2)
-				-					-				
#	3	0.38	0.11	#	4	0.50		0.20	#	5	0.6	53	0.31
#	6	0.75	0.44	#	7	0.88		0.60	#	8	1.0	00	0.79
#	9	1.13	1.00	#	10	1.27		1.27	#	11	1.4	5.7	1.56
#	14	1.69	2.25	#	18	2.26		4 00	- 4				

Confinement: Tied; #3 ties with #10 bars, #4 with larger bars. phi(a) = 0.8, phi(b) = 0.9, phi(c) = 0.7

Pattern: Irregular

Total steel area, As = 326.25 in^2 at 5.25%

Area in^2	X (in)	Y (in)	Area in^2	X (in)	Y (in)	Area in^2	X (in)	Y (in)
2.25	60.1	-11.0	2.25	60.1	-15.4	2.25	64.4	-15.4
2.25	60.1	-5.3	2.25	64.4	-5.3	2.25	-44.6	-5.1
2.25	-45.9	3.4	2.25	-45.9	7.2	2.25	-41.5	-10.7
2,25	64.4	-11.0	2.25	-33.8	-25.4	2.25	-35.9	-20.8
2.25	-47.3	-0.3	2.25	-49.7	7.2	2.25	-42.0	-0.3
2.25	-23.0	-0.3	2.25	-23.0	3.4	2.25	-27.0	-20.8
2.25	-27.0	-25.4	2.25	-23.0	10.5	2.25	-45.9	10.6
2.25	-19.2	10.6	2.25	-49.7	10.6	2.25	-42.0	3.4
2.25	-38.6	-15.8	2.25	2.9	-20.8	2.25	-4.2	-20.8
2.25	-11.4	-20.8	2.25	-18.5	-20.8	2.25	52.9	-20.8
2.25	45.8	-20.8	2.25	38.6	-20.8	2.25	31.5	-20.8
2.25	24.3	-20.8	2.25	17.2	-20.8	2.25	10.1	-20.8
2.25	-19.2	-0.2	2.25	3.0	-0.3	2.25	-4.1	-0.3
2.25	-11.3	-0.3	2.25	53.0	-0.3	2.25	45.9	-0.3
2.25	38.7	-0.3	2.25	31.6	-0.3	2.25	24.4	-0.3

	17.3		2.25	10.2	-0.3	2.25	-30.6	37.3
2.25	-34.4		2.25	-38.2	37.3	2.25	-42.0	37.3
2.25	-26.8	37.3	2.25	-19.2	3.5	2.25	2.9	-25.4
2.25	-4.2		2.25	-11.4	-25.4	2.25	-18.5	
2.25	3.0	3.4	2.25	-4.1	3.4	2.25	-11.3	
2.25	3.0	-29.2	2.25	-4.1	-29.2	2.25	-11.3	-29.2
2.25	-18.4	-29.2	2.25	3.0	7.2	2.25	-4.1	7.2
2.25	-11.3	7.2	2.25	-23.0	37.3	2.25	-23.0	30.2
2.25	-45.9	37.4	2.25	-45.9		2.25	-19.2	37.4
2.25	-19.2	30.3	2.25	-49.7	37.4	2.25	-49.7	303
2.25	-23.0	23.0	2.25	-45.9	23.1	2.25	-19.2	23.1
2.25	-49.7	23.1	2.25	-23.0	15.9	2.25	-23.0	7.2
2.25	-45.9	16.0	2.25	-30.6	44.6	2.25	-34.4	
2.25	-38.2	44.6	2.25	-45.9	44.6	2.25	-45.9	48.8
2.25	-42.0	44.6	2.25	-42.0	48.8	2.25	-38.2	48.8
2.25	-19.2	16.0	2.25	-19.2	7.3	2.25	-49.7	16.0
2.25	-49.7	3.9	2.25	-23.0	44.6	2.25	-23.0	48.8
2.25	-26.8	44.6	2.25	-26.8	48.8	2.25	-30.6	
2.25	-34.2	48.8	2.25	-19.2	44.7	2.25	-49.7	44.6
2.25	-19.2	48.9	2.25	-49.7	48.8	2.25	60.1	-0.3
2.25	64.4	-0.3	2.25	52.9	-25.4	2.25	45.8	-25.4
2.25	53.0	3.4	2.25	45.9	3.4	2.25	53.0	-29.2
2.25	45.9	-29.2	2.25	53.0	7.2	2.25	45.9	7.2
2.25	38.6	-25.4	2.25	38.7	3.4	2.25	38.7	-29.2
2.25	38.7	7.2	2.25	31.5	-25.4	2.25	24.3	-25.4
2.25	17.2	-25.4	2.25	10.1	-25.4	2.25	31.6	3.4
2.25	24.4	3.4	2.25	17.3	3.4	2.25	10.2	3.4
2.25	60.1	-20.8	2.25	60.1	3.4	2.25	64.4	3.4
2.25	31.6	-29.2	2.25	24.4	-29.2	2.25	17.3	-29.2
2.25	10.2	-29.2	2.25	31.6	7.2	2.25	24.4	7.2
2.25	17.3	7.2	2.25	10.2	7.2	2.25	60.1	-25.4
2.25	64.4	-25.4	2.25	64.4	-20.8	2.25	60.2	-29.2
2.25	60.1	7.2	2.25	64.5	-29.2	2.25	64.4	7.2
	-26.9	-29.2	2122		2312	2.25	04.4	1.2

Factored Loads and Moments with Corresponding Capacities: (see user's manual for notation)

No.	Pu kip	Mux k-ft	Muy k-ft	fMnx k-ft	fMny k-ft	fMn/Mu
1	20226.0	766.0	2669.0	19607.6	68168.1	2E E4E
2	20418.0	1050.0	3229.0	22380.7	68729.0	25.545
3	20675.0	377.0	2621.0	8641.3	60235.7	21.288
4	20871.0	1069.0	5553.0	12090.4	62491.9	22.981
5	17217.0	2894.0	1996.0	46486.3	32006.5	11.256 16.054
5 6	17391.0	1850.0	1827.0	45531.0	45092.0	
7	17633.0	3678.0	2510.0	46209.4	31463.0	24.646 12.555
8	20174.0	3147.0	6975.0	30084.3	66718.3	
9	19593.0	546.0	3345.0	9917.9	61177.7	9.564 18.286
10	20418.0	1050.0	3229.0	22380.7	68729.0	21.288
11	20675.0	2995.0	5857.0	33077.9	64568.6	11.028
12	20871.0	2961.0	8714.0	23229.2	68434.5	7.853
13	-3043.0	766.0	2669.0	22741.4	79307.7	29.712
14	-2879.0	1050.0	3229.0	25336.3	77493.8	24.012
15	-2688.0	377.0	2621.0	8918.7	62771.4	23.944
16	-2406.0	1069.0	5553.0	13159.0	68846.2	12.395
17	-34.0	2894.0	1996.0	49578.6	34144.2	17.123
18	148.0	1850.0	1827.0	45791.5	45124.0	24.726
19	354.0	3678.0	2510.0	49571.8	33895.0	13.486
20	-1708.0	3147.0	6975.0	31859.0	70890.4	10.157
21	-2410.0	546.0	3345.0	10699.3	65584.0	19.606
22	-2879.0	1050.0	3229.0	25336.3	77493.8	24.012
23	-2688.0	2995.0	5857.0	32951.7	64399.3	10.997
24	-2406.0	2961.0	8714.0	26372.0	77803.0	8.926
25	36362.4	0.0	7473.7	-21.2	40456.1	5.413

26	36590.5	0.0	9039.8	16.7	40047.7	4.430
27	36889.2	0.0	7339.0	38.7	39485.2	5.380
28	37207.6	0.0	15549.5	-20.7	38821.2	2.497
29	-17310.8	0.0	7473.7	19.2	12125.9	1.622
30	-17149.3	0.0	9039.8	-9.3	12754.4	1.411
31	-16939.5	0.0	7339.0	2.0	13651.1	1.860
32	-16715.6	0.0	15549.5	2.6	14585.9	0.938

\*\*\* Program completed as requested! \*\*\*

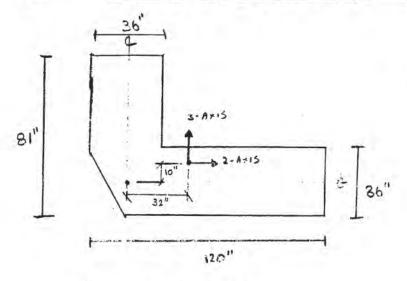
Project No. 4069 B

Page \_\_\_\_ Of \_\_\_ 6

Item

TORSION ON L-SHAPE COL

By NJR Ch'kd



LEVEL 9 ESOXIODYS  $P_{e} = 7048k$  (PIER E3)  $V_{z} = 1356k$ (LEUEL 9 IS)  $V_{3} = 307k$ (PIER E3)  $V_{3} = 307k$ (PIER E3)  $V_{3} = 307k$ (PIER E3)  $V_{3} = 307k$   $V_{3} = 307k$   $V_{4} = 1501k - f + 6$   $V_{3} = 3350k - f + 6$ 

REINFORCING 136 #14 VERT

Acp = 
$$5660 \text{ in}^2$$
 (AREA)  
Pcp =  $389 \text{ in}$  (Perimeter)  
 $\phi = 0.85$  (Act 318-99)  
 $f'_{L} = 10$ 

### ACI 318 11.6.1 (CHECK TO SEE IF TORSION CAN BE NEGLECTED)

If  $T_{u} < \phi \sqrt{F'_{c}} \left( \frac{A_{c}p^{2}}{P_{c}p} \right) \Rightarrow \text{NEGLECT TORSION}$ 1790k#  $< .85\sqrt{10,000} \left( \frac{(5660 i_{n}^{2})^{2}}{389} \right) \left( \frac{1}{1000} \frac{f_{0}}{f_{0}} \right) ?$ 1790k-ft < .583 k-ft

TORSION CAN NOT BE NEGLECTED

 Project
 Page
 2
 Of
 6

 Project No.
 Date
 4/15/05

 Item
 By
 NJR
 Ch'kd

ACI 318-99 SEC 11.6.3 IS SECTION ADEQUATE

$$\sqrt{\frac{v_0}{b_w l}^2 + \left(\frac{T_0 p_h}{1.7 A_{oh}^2}\right)^2} \leq \phi \left(\frac{v_e}{b_w d} + 8Jf'_e\right)$$

$$\sqrt{\frac{369.3^2 + 186.2^2}{413}} \leq \phi \left(0 + 8J_{0000}\right)$$

$$413 \leq 680$$

0 = 0.85

Ach = Soz4 in²

Ph = 372 in

d = 102 in

bw = 36"

Vu = 1356

Tu = 1790 k-ft

= 21,480,000 lb-in

f'c = 10,000 psi

Assume Ve = 0

. SEC IS AdeqUATE

Project \_\_\_\_\_

Page 3 of 6

Project No.

By NJR Ch'kd

Ifem

DETERMINE AMOUNT OF TORSION REW.

### 11.6.3.6 TRANSVERSE

ACI 318-99 SEC 11.6.3

60 kei ⇒ Atmin = 0.197in²
75 ksi ⇒ Atmin = 0.157in²

## 11.6.3.7 LOGITUDINAL

$$A_{\ell} = \frac{A_{t}}{s} P_{h} \left( \frac{f_{yv}}{f_{y} \varrho} \right) \cot^{2}(4s)$$

(60ksi) Ax = 11.7 in2

(75 ksi) Az = 14.6 in2

- ADD 0.157 in to leg of shear STIRUP

. ADD 18.3 12 40 UERT BARS

Project	Page 4 Of 6
Project No.	Date 4 15/05
Item	By NIR Ch'kd

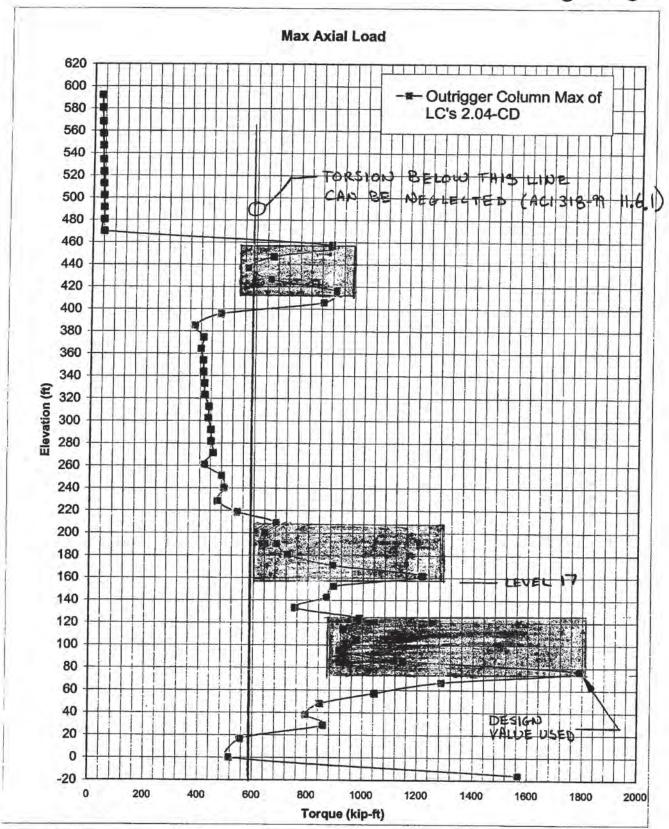
### CHECK MIN TORSION REINFORCMENT (ACI 99 11.6.5)

EQU 11-23 WILL BE CHECKED AND SATISFIED WHEN SHEAR REINFORCEMENT IS CHECKED.

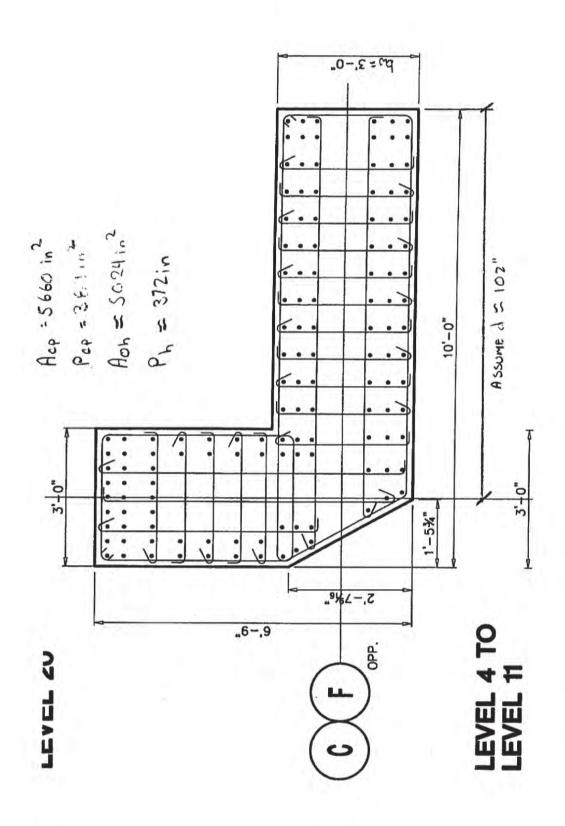
$$Al_{min} = \frac{S\sqrt{f'c} Acp}{f_{yl}} - \left(\frac{A_{t}}{S}\right) P_{n} \frac{f_{yv}}{f_{yl}}$$

$$= 23.1 in^{2} \qquad GOVERNS$$

Add 23.1 in long. Steel



SHADED AREAS REPRESENT OUTRIGGER LOCATIONS



. , . ,

8.1.3-27 DODSONNOC00000487

# 8.2 Secondary Special Moment Frame System

#### 8.2.1 Beams and Columns

The Special Moment Resisting Frames' (SMRF) are designed according to all the requirements of UBC 1921. Beam shear is checked against the probable moment strength of the beams. Since plastic hinge column ties are being used over the full height of the frame columns, strong column weak beam criteria will not be checked per UBC 1921.4.2.3.

# SUMMARY OF EQ LOADS

COUNN A

<b>生态性性性</b>	Story 75	Column	Load & Green	P解性的基	M2	M3
是是 <b>建设于P</b>		C109	SCMFLONG	2170		72
是能震-P		C109	SCMFLONG	-2170		
#事業+M2		C110	SCMFLONG	2170		
Make M2		C110	SCMFLONG	-2170		
ME M3		C97	SCMFLONG	2161		
SAME MS		C97	SCMFLONG	-2161		-
A PARK TO	-	007	COMI LONG	-2101	-41	-285
	Stonesakt	California	Load house	D China	LAD SPECIAL	14.10 ( 186.50
ф	5	C109	SCMFLONG			
NEW PD		C109	SCMFLONG	-2009		
M2		C110	SCMFLONG			
7 N/2		C110	SCMFLONG	2009		
V/2		C110	SCMFLONG	-2009		
170		C110		2009		
300		CITO SE	SCMFLONG	-2009		-161
· i	Story	THE RESERVE OF THE PERSON NAMED IN	The state of the s			
			Load States		M2	
		C109	SCMFLONG	1273	3	
		C109	SCMFLONG	-1273	-3	-45
at M2		C109	SCMFLONG	1188	- 8	54
.M2		C109	SCMFLONG	-1188		-54
SEE+M3		C110	SCMFLONG	1273		75
Figure M3	15	C110	SCMFLONG	-1273	-5	-75
11.7	是一些的概念		を できない		<b>是一个人</b>	
				P.資料整	M2+ ***	M3 💆 📆
#P		C109	SCMFLONG	1033	9	62
(美麗麗)P		C109	SCMFLONG	-1033	-9	-62
<b>海</b> 等M2		C109	SCMFLONG	1033	9	62
是EXE-M2	21	C109	SCMFLONG	-1033	-9	-62
特殊技术M3	21	C96	SCMFLONG	1025	9	65
GIESE M3	21	C96	SCMFLONG	-1025	-9	-65
		<b>Charges</b>	100			
			Load Manage	Paragraphic	M2	M32 30 3
312		C109	SCMFLONG	688	2	46
. [2]		C109	SCMFLONG	-688	-2	-460
1M2		C109	SCMFLONG	620	6	504
1/12 × 1/12		C109	SCMFLONG	-620	-6	-504
11X8		C96	SCMFLONG	612	6	530
SM		C96	SCMFLONG	-612	-6	-530
	2200	STEEL STATE OF	Sales Sales	012	-0	-000
	Story 1	Collandor	Load	Daniel Control	Moderation	Vi2
			SCMFLONG	329	NIZ:SPEZ 573	
					- 0)	43
: i/k			SCMFLONG	-329	-3	-43
42			SCMFLONG	329	4	43
Marina Marina			SCMFLONG	-329	-4	-431
			SCMFLONG	329	4	431
	41		SCMFLONG	-329	-4	-431
:M3						S. Contract
W8	ALC: NO.	Time and				
<u>ः</u> ।\\।इ	Slony	Column	Load 她。如何是		M2:南澳西南	M3-ERE
: XIS         13	Slory 51	Columnia C109	L'oad Marie SCMFLONG	P. 138	M2 1 23 1	
:X8 :13 :43 :43	Story 51 51	Columnia C109 C109	SCMFLONG SCMFLONG			160
3/8 3/8 48 48	51 51 51 53	Columns C109 C109 C109 C109	L'oad Marie A	138	1	160 -160
103 113 113 1102 1102	51 51 51 53	Column 199 C109 C109 C109	SCMFLONG SCMFLONG	138 -138	1 -1	160 -160 161
18 18 48 48	51 51 51 53 53	Columns C109 C109 C109 C109	SCMFLONG SCMFLONG SCMFLONG SCMFLONG	138 -138 103	1 -1 6	160 -160

2.09-CD JPDATE 5/21/05

(GROUP A - LC MESSY25)

MAX PGROUD = 1400kips

MAX M 3 = 1600 k.ft

: GROUP A PREVIOUS DESIGN IS OK

COLUMNIC

4	-	Story		Load Maria	Davis Co	M2	142 PROM
<b>水温</b>	8+P		C158	SCMFLONG	6828		
	WAP		C158	SCMFLONG	-6828		
	+1/12		C107	SCMFLONG	6184		
10.00	M2		C107	SCMFLONG	-6184		
经验录	FM3		C158	SCMFLONG	6828		
7	СМЗ		C158	SCMFLONG	-6828	-100	
	30-1		-		0020	-100	-5040
- 4		Story	Column	Load & 1947	PANTAN	M24 # 4	AA2 WARE TO
	40		C158	SCMFLONG	5962	18	2114
	Rep.		C158	SCMFLONG	-5962	-18	
	FM2		C107	SCMFLONG	5669	20	2098
	M2		C107	SCMFLONG	-5669	-20	
	ЕМ3		C157	SCMFLONG	5870	17	2171
	ZM3		C157	SCMFLONG	-5870	-17	-2171
Anne		12.59		A SECULAR DE LA COLONIA DE LA COLONIA DE LA COLONIA DE LA COLONIA DE LA COLONIA DE LA COLONIA DE LA COLONIA DE	A WASHINGTON	A CONTRACTOR OF THE PARTY OF TH	TO THE STATE OF TH
		Story 3	Column	liosid	P.Weisin	MZ	IM3
-	32		C107	SCMFLONG	3563	10	736
	EP.		C107	SCMFLONG	-3563	-10	-736
	M2		C99	SCMFLONG	3281	20	770
	M2		C99	SCMFLONG	-3281	-20	-770
	МЗ		C107	SCMFLONG	2997	5	857
508	M3		C107	SCMFLONG	-2997	-5	-857
		No.	1000	7	TO STATE OF THE ST	**************************************	<b>国际</b>
		Story	Column to	1000	9	M2-14-14-1	M3 Market
	<b>建</b> 尼	21	C107	SCMFLONG	2894	20	851
	<b>3</b> P		C107	SCMFLONG	-2894	-20	-851
	MZ		C99	SCMFLONG	2859	21	860
	M2		C99	SCMFLONG	-2859	-21	-860
	МЗ		C99	SCMFLONG	2859	21	860
	M3	21	C99	SCMFLONG	-2859	-21	-860
100		经验风险	Y CHANGE				EL SALT
		Story	Columnia	Load Walk	P	Made	M3
	812	31	C107	SCMFLONG	1966	3	- 646
	E-B	31	C107	SCMFLONG	-1966	-3	-646
	M2			SCMFLONG	1759	14	695
	M2			SCMFLONG	-1759	-14	-695
W- '-!	M3	33		SCMFLONG	1759	14	695
	Ma		C99	SCMFLONG	-1759	-14	-695
		Slory	Column	Load Markey	P I	M2	Ms.
		41	C158	SCMFLONG	1045	8	504
	ER.	41	C158	SCMFLONG	-1045	-8	-504
	M2	41		SCMFLONG	1001	10	620
E	M2	41	C107	SCMFLONG	-1001	-10	-620
1	M3	41		SCMFLONG	1001	10	620
77	M3	41		SCMFLONG	-1001	-10	-620
	•	le same	CV 1	THE RESERVE	L v		C . S . 1 . (c)
1 VI		Story and	Column	Koad/	P	MZ	M3 Semily
(i) i		51		SCMFLONG	396	4	270
	EP.			SCMFLONG	-396	-4	-270
	M2			SCMFLONG	286	14	265
	MZ			SCMFLONG	-286	-14	-265
				SCMFLONG	339	2	282
4	M31	521	0101	SCIVIL LONG	UU31		
				SCMFLONG	-339	-2	-282

2.09-CD UPDATE 5/21/05 (GROUP C - LC MFSSX25) MAX P = 8700 kips MAX M3 = 2,800 kip - Pt

GROUP C DESIGN WILL BE UPDATED @ GROUND FOR PE = 8,700 kips COLUMN D

TOOP A	Ctor	Cotton	le seu pro-seu seu seu	Inches	la re-	
HERE REAL	Story 3	Column	Load	Page	M2	
	GROUND		SCMFSHORT	6731		
1 25 E	GROUND		SCMFSHORT	-6731	-12	-167
21 B#M2		C165	E100X100Y5 MAX	1554	558	194
-M2		C165	E100X100Y5 MAX	-1554	-558	-194
SA SET MI		C170	SCMFSHORT	6630	55	697
<b>建</b> 基M3	2	C170	SCMFSHORT	-6630		
學是是						
	Story **	Column	Load Sales	Person	M2	М3.
1 T	4	C161	SCMFSHORT	5956		507
	4	C161	SCMFSHORT	-5956		-507
		C170	E100X100Y5 MAX			
M2		C170	E100X100Y5 MAX			
M3		C161	SCMFSHORT			
žM3	1	C161	SCMFSHORT	5956		
- TOP TWO	4	10101		-5956	-1/	-507
>00	IN THE	Par Property				
575	OLOI Y THE	Column	Coacia			
		C164	SCMFSHORT	2491		
		C164	SCMFSHORT	-2491		
EBEM2		C165	E100X100Y5 MAX			
全要性M2	17	C165	E100X100Y5 MAX	-431	-62	-64
#M8		C170	SCMFSHORT	2207		3447
#M3		C170 ·	SCMFSHORT	-2207	-5	-3447
100	和影響的影響	<b>医</b>		V 21		
	Story	Column	Load To A Company	P層場	M2	M3
62	21	C164	SCMFSHORT	2155		2715
100		C164	SCMFSHORT	-2155		-2715
1 M2		C165	E100X100Y5 MAX	387	66	
<b>1/2</b>		C165	E100X100Y5 MAX	-387		-574
EMB		C171	SCMFSHORT	2155		
#M3		C171	SCMFSHORT	-2155		2727
1		-	SCMFSHORT	-2155		-2727
emple at a second						沙湖市
				P. Min		
V.		C164	SCMFSHORT	1549	3	351
151		C164	SCMFSHORT	-1549	-3	-351
+M2		C165	E100X100Y5 MAX	243	27	203
17.2		C165	E100X100Y5 MAX	-243	-27	-203
(V.S.)		C171	SCMFSHORT	1448	2	1140
:08	33	C171	SCMFSHORT	-1448	-2	-1140
عاطلوا مع				1		
	Story	Column	vead -	Park	M2%	M3 **
STREET, CO.	41	C164	SCMFSHORT	832	1	280
- 18 - 18			SCMFSHORT	-832	-1	-280
34 M2			E100X100Y5 MAX	141	11	63
1M2			E100X100Y5 MAX	-141	-11	-63
BEM3			SCMFSHORT		$\overline{}$	
M3			SCMFSHORT	724	1	761
		C 108	SUMPSHUKI	-724	-1	-761
- A					<b>新</b>	
	OUTY主题		Load See A		_	
884B			SCMFSHORT	330	1	477
			SCMFSHORT	-330	-1	-477
THE PERSON NAMED IN	53	C161	E100X100Y5 MAX	66	6	79
<b>EM2</b>		~	E400V400VE MAY		_	
M2 M2	53	C161	E100X100Y5 MAX	-661	-6	-/9
<b>要</b> EM2			SCMFSHORT	-66 330	<del>-6</del>	-79 477

2.09-CD UPDATE (GROUPD-LC MFSSX25) MAX PGROUPD = 4000 K

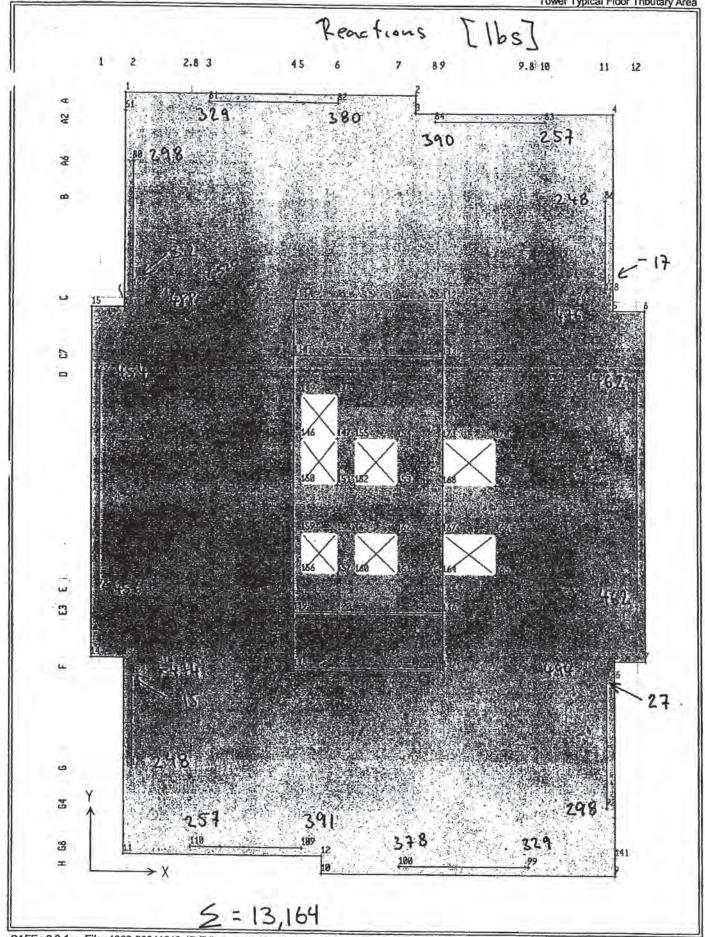
MAX MBGROWD = 6,400 k-ft

.: GROUP D PREVIOUS

DESIGN IS OK

FOR VERTICAL BARS

E GROUD



SAFE v8.0.1 - File: 4069-20041210-JP-Trib-A - December 10,2004 17:11 - Scale: Fit to Page Structural Layer Plan View - Kip-ft Units

	LAGRED-LTL Design	J.	138	200	808	8	100	MIN	5967	170	200	2160	2300	No.	100	2800	New .	長	3156	1906	1997	918	E P	3 6	True	CON	9969	1000			100	3636	SUN	2360	MON	909	Carlo	65	100	200	102	2535	MON	THE .	9000	0010	1004	17.75		9	-
	Defer Defer	Her I	¥ 1	E A	425	980	18	E	316	000	NO.	1286	1356	108	Mak	100	45	180	9081	2005	2156	2367	200	100	XIII	2222	1793	No.	200	1117	100	3389	2	3000	3685	1704			1140		4933	400	4527	4534	4731	4627	4004	3011	200	3368	47.00
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	Com Embeddie LL	odi *	nz		3:	100		3	S E	2	A	ă	360	238	Ā	312	238	5	1	4	200		9	939	4	8	0		100	282	600	8	25	089	200	21	-	S P	312	104	12	100	200	21	2	2	25	2.5	111	888	1000
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3	On Year	おな	医医	651 99	20	35	111 12	100	115	212 98	86 600	H H		20	故 遊	123	160	M 1	200		100	100	201 665	200	201 105								200		100		100	601 100	200	900 000	RIS 100	823 100	100	100	9		2 2	100	13 109	MM 116	
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	n	74 analogiler	100	580	97.6	0.65	0.62	090	0.56	570	0.53	0.52	0.50	6.49	0.48	0.47	0.46	0.46	0.45	0.45	0.45	0.44	200	0.41	0.43	0.43	0.62	200	0.41	0.41	160	0.41	070	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	
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k:	Fir.III	. R.	2	10.75	10.75	10.75	10.75	12.5	10.5	10.5	10.5	10.5	10.5	10.5	10.5	200	10.5	105	10.5	10.5	10.5	10.5	10.5	10.5	10.5	10.5	10.5	505	10.5	11.5	8.5	9.5	9 6	9 9	9.5	3.5	50	20 0	8.8	5.0	100	9 6	o de	20	9.6	9.8	9.5	8.5	12,1687	15.75		
dhoug mou		Usage Wesh	Roof	Moch	Typ	22	Typ	2	Mech	77.	170	de	100	Typ	T)	Meca	Tag.	Typ	The same	The same	占	130	8	The	130	130	ß,	34	Mark	Mech	Mech	g,	172	Type	Typ	Typ	S,	5,5	Typ	d(L	100	25	30	Iva	Typ	170	S.	8	137	Occupative Company		
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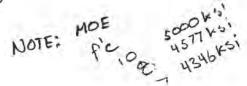
ADD 1000 k-ft for GRAV MOMENT.

M31.42(dIDD+0.5L 0.9\*(dIDD 1.4(dIDD+1.7L Power Plane)

Group A						1					
2,044	Story	Column	Load	i P	M2	Ma	1.42(dlf)D+0.5L	0.9*(dlf)D	1.4(dlf)D+1.7L	Pcome	Ptens
4P	2	C109	SCMFLONG	2170	33	2341	8860			11030	
-P	2	C109	SCMFLONG							11030	
+M2		C110	SCMFLONG					7-11-2		11030	
-M2	2	C110	SCMFLONG				7777			11030	
+M3	2	C97	SCMFLONG							11021	
-M3		C97	SCMFLONG							11021	
	Story	Column	Load	P	M2	Мз	1.42(dlf)D+0.5L	0.9*(416)	1.4/diftD+1.7L	P	P
+P		C109	SCMFLONG							10345	
-P		C109	SCMFLONG						100000	10345	
+M2	1	C110	SCMFLONG						77.075	10345	
-M2		C110	SCMFLONG								
+M3		C110	SCMFLONG							10345	
-M3		C110	SCMFLONG							10345	
	Story	Column	Load	Р	Ma	M3	1.42(dlf)D+0.5L	0.0*(416)	1 4/4/6/044 27	D	D
4P		C109	SCMFLONG						The second secon	100	Ptens
-P			SCMFLONG			41.0X	172.77				2867
+M2		C109			-	100	7.7.7	00.00	7.00		2867
-M2		C109	SCMFLONG			- TEO 6	2727		2,7,14		2952
		C109	SCMFLONG				377.70				2952
+M3		C110	SCMFLONG				7.17.7				2867
-М3	15	C110	SCMFLONG	-12/3	-5	-754	6759	4140	7043	8032	2867
	-		Load	P	M2	МЗ	1.42(dlf)D+0.5L	0.9*(dlf)D	1.4(dlf)D+1.7L	Pcomp	Plens
+P	21	C109	SCMFLONG	1033	9	627	6032	3695	6284	7065	2662
P	21	C109	SCMFLONG	-1033	-9	-627	6032	3695	6284	7065	2662
+M2	21	C109	SCMFLONG	1033	9	627	6032	3695	6284	7065	2662
M2	21	C109	SCMFLONG	-1033	-9	-627	6032	3695	6284	7065	2662
+M3	21	C96	SCMFLONG	1025	9	654	6032	3695	6284		2670
-M3	21	C96	SCMFLONG	-1025	-9	-654	6032	3695	6284	7057	2670
	Story	Column	Load	P	M2	МЗ	1.42(dlf)D+0.5L	0.9*(dlf)D	1.4(dlf)D+1.7L	Pcomo	Ptens
+P	31	C109	SCMFLONG			460					2105
P		C109	SCMFLONG		-2			2793			2105
+M2		C109	SCMFLONG		6		(CDD, 7)	2793			2173
M2		C109	SCMFLONG		-6			2793	200		2173
+M3		C96	SCMFLONG		6		7.070				2181
МЗ		C96	SCMFLONG	77.57		-530		2793	4746		2181
	Story	Column	Load	P	M2	МЗ	1.42(dlf)D+0.5L	O4(4)PD	1.4/dlnD+1.7I	P	P
P		C109	SCMFLONG	329	3	431	3073	1883			
P		C109	SCMFLONG	-329	-3		3073	1883	3201 3201	3402	
M2		C110	SCMFLONG	329	4	431					1554
M2		C110	SCMFLONG	-329	4		3073	1883	3201		1554
-M3		C110	SCMFLONG	329	4		3073	1883	3201		1554
M3			SCMFLONG	-329		431 -431	3073 3073	1883 1883	3201 3201	3402	1554
			John Lotto	020		401	5015	1005	3201	3402	1554
		Column					1.42(dlf)D+0.5L				
P		C109	SCMFLONG	138	1	160	1640	1003	1714	1778	865
P			SCMFLONG	-138	-1	-160	1640	1003	1714	1778	865
M2			SCMFLONG	103	6	161	1640	1003	1714	1743	900
M2			SCMFLONG	-103	-6	-161	1640	1003	1714	1743	900
-M3			SCMFLONG	116	5	218	1640	1003	1714	1756	887
МЗ		C97	SCMFLONG	-116	-5	-218	1640	1003	1714	1756	887

dlf = dead load factor (see gravity load take down)

4069-20041209-NJR-col design



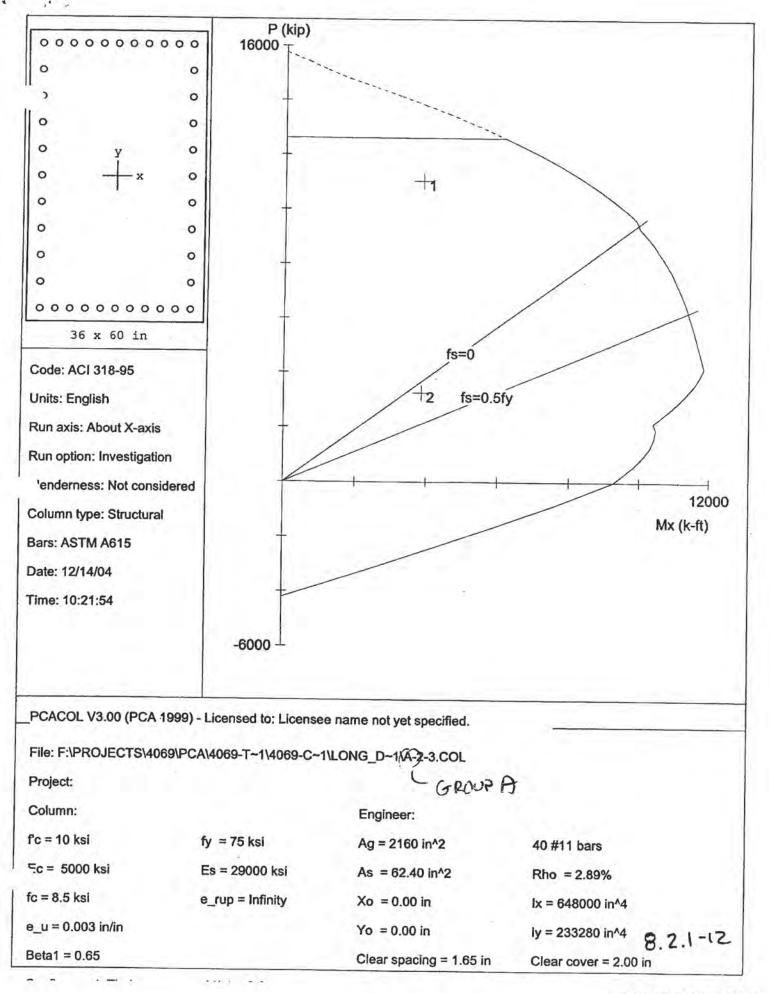
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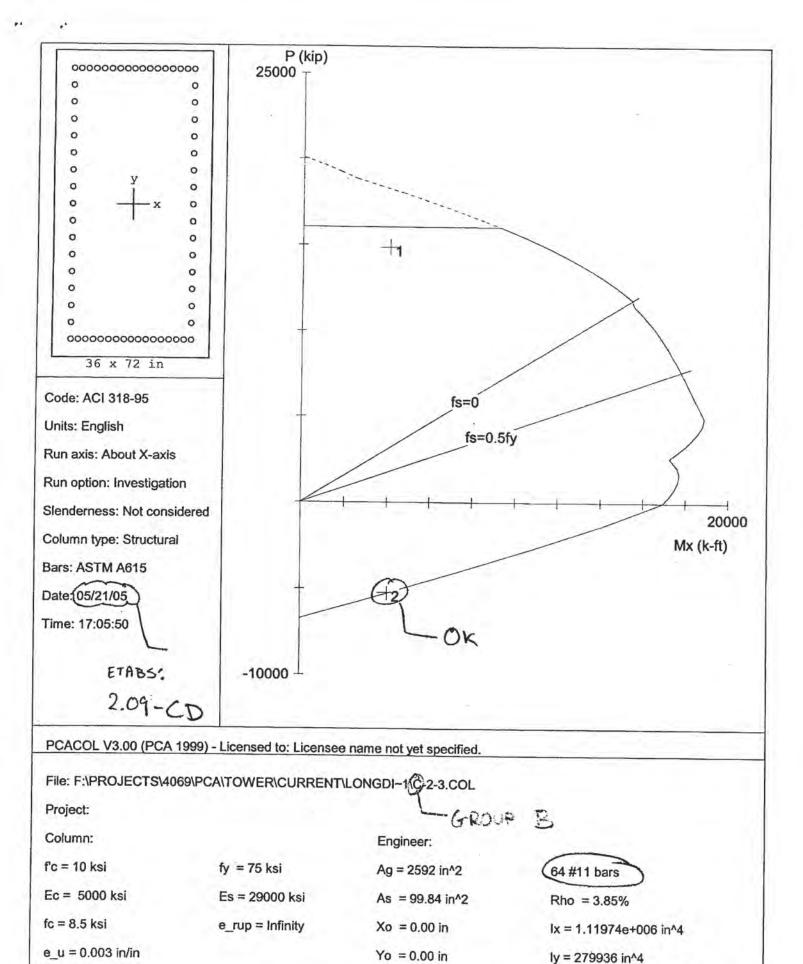
Croup C					1	_		9					12/14/2004	
Story   Column   Load   P								D   PE = 8	100 ki	ps		14 969		
Story   Column   Load   P	Group C				. 30	VU	PDAT	Elasho-			1	17,000	7175	4
P	Group C		Colum	n Load	D /	Ma	1/12	1 42/2100	0.01/1105		1	-/-	5213	Kips
-P	+D					NIZ	MJ				comp	Piens		
+M2 2 C107 SCMFLONG 6184 111 2504 6168 3487 6341 12352 -2897  -M2 2 C107 SCMFLONG 6828 1-10 3040 6168 3487 6341 12352 -2897  +M3 2 C168 SCMFLONG 6828 100 3040 6168 3487 6341 12398 -3341  -M3 2 C168 SCMFLONG 5828 100 3040 6168 3487 6341 12998 -3341  -Story Column Load P M2 M3 1.42(dli)D+0.SL 0.9*(dli)D 1.4(dli)D+1.7L Pomp Plane  +P 5 C168 SCMFLONG 5962 18 2114 5804 3277 5968 11768 -2685  -P 5 C168 SCMFLONG 5962 18 2114 5804 3277 5968 11768 -2685  -M2 5 C107 SCMFLONG 5669 20 2098 5804 3277 5968 11768 -2685  -M2 5 C107 SCMFLONG 5669 20 2098 5804 3277 5968 11767 -2585  -M3 5 C107 SCMFLONG 5669 20 2098 5804 3277 5968 11767 -2593  -M3 5 C157 SCMFLONG 5670 17 2171 5804 3277 5968 11674 -2593  -M3 5 C157 SCMFLONG 5870 17 2171 5804 3277 5968 11674 -2593  -M3 5 C157 SCMFLONG 3563 10 736 4720 2682 5072 8283 901  +M2 17 C39 SCMFLONG 3563 10 736 4720 2682 5072 8283 901  +M2 17 C39 SCMFLONG 3563 10 736 4720 2682 5072 8283 901  +M3 20 C107 SCMFLONG 3281 20 770 4520 2549 4855 7801 732  -M3 20 C107 SCMFLONG 3281 20 770 4520 2549 4855 7801 732  -M3 20 C107 SCMFLONG 3281 20 770 4520 2549 4855 7801 732  -M3 20 C107 SCMFLONG 3281 20 770 4520 2549 4855 7801 732  -M3 20 C107 SCMFLONG 2897 5 857 4220 2380 5184 7217 -817  -M3 20 C107 SCMFLONG 2897 5 857 4220 2380 5184 7217 -817  -M3 20 C107 SCMFLONG 2899 20 851 4121 2324 4240 6880 535  -M4 21 C39 SCMFLONG 2859 21 880 4121 2324 4240 6880 535  -M4 21 C39 SCMFLONG 2859 21 880 4121 2324 4240 6880 535  -M3 3 C39 SCMFLONG 2859 21 880 4121 2324 4240 6880 535  -M4 21 C39 SCMFLONG 2859 21 880 4121 2324 4240 6880 535  -M4 21 C39 SCMFLONG 1866 3 486 3101 1748 3197 5067 -218  -M4 21 C39 SCMFLONG 1866 3 486 3101 1748 3197 5067 -218  -M4 21 C39 SCMFLONG 1896 3 448 3101 1748 3197 5067 -218  -M4 21 C39 SCMFLONG 1896 3 448 3101 1748 3197 5067 -218  -M4 21 C107 SCMFLONG 1896 3 448 3101 1748 3197 5067 -218  -M4 21 C107 SCMFLONG 1896 3 448 3101 1748 3197 5067 -218  -M4 21 C107 SCMFLONG 1896 3 448 3101 1748 3197 5067 -218  -M4 31 C107 SCMFLONG 1896 3 448 3101 1748 3197 5067 -218  -M4 31 C107 SCMFLONG 1896 3				SCMFLONG	6020	3 10	0 304							
May   2 C 1017   SCMFLONG   68184   -111   -2504   e1188   34817   6341   12382   2687				SCMELONG	2 610	4 44	1 250	6168						
Hard   Story Column   Load   P   Mz   M3   1.42(dli)D+0.5L   0.9*(dli)D   1.4(dli)D+1.7L   Pcomp   Plans   Pcomp   Pcomp   Plans   Pcomp   P				SCMFLONG	2 -619	1 11	1 250							
Story   Column   Load   P   M2   M3   1.42(dl)D+0.5L   0.9*(dl)D   1.4(dl)D+1.TL   Pcomp   Plans   P				SCMFLONG	2 6829	100	1 -2504							
## 5 C158 SCMFLONG 5962 18 2114 5804 3277 5968 11768 -2685  P 5 C158 SCMFLONG 5962 18 2114 5804 3277 5968 11768 -2685  ## 2 5 C167 SCMFLONG 5669 20 2098 5804 3277 5968 11773 -2392  ## 3 5 C167 SCMFLONG 5669 20 2098 5804 3277 5968 11673 -2392  ## 3 5 C157 SCMFLONG 5869 17 2171 5804 3277 5968 11674 -2593  ## 3 5 C157 SCMFLONG 5870 17 2171 5804 3277 5968 11674 -2593  ## 3 5 C157 SCMFLONG 5870 17 2171 5804 3277 5968 11674 -2593  ## 2 5 C107 SCMFLONG 5870 17 2171 5804 3277 5968 11674 -2593  ## 2 5 C107 SCMFLONG 3683 10 736 4720 2662 5072 8283 -901  ## 15 C107 SCMFLONG 3563 10 736 4720 2662 5072 8283 -901  ## 15 C107 SCMFLONG 3563 10 736 4720 2662 5072 8283 -901  ## 15 C107 SCMFLONG 3281 20 770 4520 2549 4855 7801 732  ## 3 20 C107 SCMFLONG 3281 -20 -770 4520 2549 4855 7801 732  ## 3 20 C107 SCMFLONG 3281 -20 -770 4520 2549 4855 7801 732  ## 3 20 C107 SCMFLONG 2897 5 857 4220 2380 5184 7217 -817  ## 2 C107 SCMFLONG 2894 20 851 4121 2324 4240 7015 -570  ## 2 C107 SCMFLONG 2894 20 851 4121 2324 4240 7015 -570  ## 2 C109 SCMFLONG 2894 20 851 4121 2324 4240 7015 -570  ## 2 C109 SCMFLONG 2894 20 851 4121 2324 4240 6980 -535  ## 3 C109 SCMFLONG 2899 21 860 4121 2324 4240 6980 -535  ## 3 C109 SCMFLONG 2899 21 860 4121 2324 4240 6980 -535  ## 3 C109 SCMFLONG 2899 21 860 4121 2324 4240 6980 -535  ## 3 C109 SCMFLONG 1966 3 646 3101 1748 3197 5067 -218  ## 2 31 C107 SCMFLONG 1966 3 -646 3101 1748 3197 5067 -218  ## 2 31 C107 SCMFLONG 1966 3 -646 3101 1748 3197 5067 -218  ## 3 C109 SCMFLONG 1966 3 -646 3101 1748 3197 5067 -218  ## 4 C108 SCMFLONG 1966 3 -646 3101 1748 3197 4860 -11  ## 4 C108 SCMFLONG 1966 3 -646 3101 1748 3197 4860 -11  ## 4 C108 SCMFLONG 1966 3 -646 3101 1748 3197 4860 -11  ## 4 C108 SCMFLONG 1966 3 -646 3101 1748 3197 4860 -11  ## 4 C108 SCMFLONG 1966 3 -646 3101 1748 3197 4860 -11  ## 4 C108 SCMFLONG 1966 3 -646 3101 1748 3197 4860 -11  ## 5 C107 SCMFLONG 1966 3 -646 3101 1748 3197 4860 -11  ## 5 SCMFLONG 1966 3 -646 3101 1748 3197 4860 -11  ## 5 SCMFLONG 1966 3 -646 3101 1748 3197 4860 -11  ## 5 SCMFLONG 19					3 -6828	3 -100	0 -304							
## 5 C158 SCMFLONG 5962 18 2114 5804 3277 5968 11768 -2685  P 5 C158 SCMFLONG 5962 18 2114 5804 3277 5968 11768 -2685  ## 2 5 C167 SCMFLONG 5669 20 2098 5804 3277 5968 11773 -2392  ## 3 5 C167 SCMFLONG 5669 20 2098 5804 3277 5968 11673 -2392  ## 3 5 C157 SCMFLONG 5869 17 2171 5804 3277 5968 11674 -2593  ## 3 5 C157 SCMFLONG 5870 17 2171 5804 3277 5968 11674 -2593  ## 3 5 C157 SCMFLONG 5870 17 2171 5804 3277 5968 11674 -2593  ## 2 5 C107 SCMFLONG 5870 17 2171 5804 3277 5968 11674 -2593  ## 2 5 C107 SCMFLONG 3683 10 736 4720 2662 5072 8283 -901  ## 15 C107 SCMFLONG 3563 10 736 4720 2662 5072 8283 -901  ## 15 C107 SCMFLONG 3563 10 736 4720 2662 5072 8283 -901  ## 15 C107 SCMFLONG 3281 20 770 4520 2549 4855 7801 732  ## 3 20 C107 SCMFLONG 3281 -20 -770 4520 2549 4855 7801 732  ## 3 20 C107 SCMFLONG 3281 -20 -770 4520 2549 4855 7801 732  ## 3 20 C107 SCMFLONG 2897 5 857 4220 2380 5184 7217 -817  ## 2 C107 SCMFLONG 2894 20 851 4121 2324 4240 7015 -570  ## 2 C107 SCMFLONG 2894 20 851 4121 2324 4240 7015 -570  ## 2 C109 SCMFLONG 2894 20 851 4121 2324 4240 7015 -570  ## 2 C109 SCMFLONG 2894 20 851 4121 2324 4240 6980 -535  ## 3 C109 SCMFLONG 2899 21 860 4121 2324 4240 6980 -535  ## 3 C109 SCMFLONG 2899 21 860 4121 2324 4240 6980 -535  ## 3 C109 SCMFLONG 2899 21 860 4121 2324 4240 6980 -535  ## 3 C109 SCMFLONG 1966 3 646 3101 1748 3197 5067 -218  ## 2 31 C107 SCMFLONG 1966 3 -646 3101 1748 3197 5067 -218  ## 2 31 C107 SCMFLONG 1966 3 -646 3101 1748 3197 5067 -218  ## 3 C109 SCMFLONG 1966 3 -646 3101 1748 3197 5067 -218  ## 4 C108 SCMFLONG 1966 3 -646 3101 1748 3197 4860 -11  ## 4 C108 SCMFLONG 1966 3 -646 3101 1748 3197 4860 -11  ## 4 C108 SCMFLONG 1966 3 -646 3101 1748 3197 4860 -11  ## 4 C108 SCMFLONG 1966 3 -646 3101 1748 3197 4860 -11  ## 4 C108 SCMFLONG 1966 3 -646 3101 1748 3197 4860 -11  ## 4 C108 SCMFLONG 1966 3 -646 3101 1748 3197 4860 -11  ## 5 C107 SCMFLONG 1966 3 -646 3101 1748 3197 4860 -11  ## 5 SCMFLONG 1966 3 -646 3101 1748 3197 4860 -11  ## 5 SCMFLONG 1966 3 -646 3101 1748 3197 4860 -11  ## 5 SCMFLONG 19		Story	Colum	n Load	P	M2	МЗ	1.42(dlf)D+0.51.	0.0*(4)00	1.4/(1)(1)(1)(1)	P	P.		
-P 5 C168 SCMFLONG -5962 -18 -2114	+P													
+M2 5 C107 SCMFLONG 5669 20 2098 5804 3277 5968 11473 -2392								77.7						
-M2 5 C107 SCMFLONG 5669 20 2098 5804 3277 5968 11473 2392 -M3 5 C157 SCMFLONG 5870 17 2171 5904 3277 5968 11473 2392 -M3 5 C157 SCMFLONG 5870 17 2171 5904 3277 5968 11674 2593 -M3 5 C157 SCMFLONG 5870 17 2171 5904 3277 5968 11674 2593 -M3 5 C157 SCMFLONG 5870 17 2171 5904 3277 5968 11674 2593 -M3 5 C157 SCMFLONG 5807 17 2171 5904 3277 5968 11674 2593 -M5 15 C107 SCMFLONG 3563 10 736 4720 2662 5072 8283 901 -P 15 C107 SCMFLONG 3563 10 736 4720 2662 5072 8283 901 -M2 17 C99 SCMFLONG 3281 20 770 4520 2549 4855 7801 732 -M2 17 C99 SCMFLONG 3281 20 770 4520 2549 4855 7801 732 -M3 20 C107 SCMFLONG 2997 5 857 4220 2380 5184 7217 -617 -M3 20 C107 SCMFLONG 2997 5 857 4220 2380 5184 7217 -617 -M3 20 C107 SCMFLONG 2997 5 857 4220 2380 5184 7217 -617 -M3 20 C107 SCMFLONG 2894 20 851 4121 2324 4240 7015 570 -P 21 C107 SCMFLONG 2894 20 851 4121 2324 4240 7015 570 -P 21 C107 SCMFLONG 2894 20 851 4121 2324 4240 7015 570 -P 21 C107 SCMFLONG 2899 21 860 4121 2324 4240 6980 -535 -M3 21 C99 SCMFLONG 2859 21 860 4121 2324 4240 6980 -535 -M3 21 C99 SCMFLONG 2859 21 860 4121 2324 4240 6980 -535 -M3 21 C99 SCMFLONG 2859 21 860 4121 2324 4240 6980 -535 -M3 21 C99 SCMFLONG 2859 21 860 4121 2324 4240 6980 -535 -M3 21 C99 SCMFLONG 1759 14 895 3101 1748 3197 5067 -218 -P 31 C107 SCMFLONG 1966 3 646 3101 1748 3197 5067 -218 -M2 33 C99 SCMFLONG 1759 14 895 3101 1748 3197 4860 -11 -M3 33 C99 SCMFLONG 1759 14 895 3101 1748 3197 4860 -11 -M3 33 C99 SCMFLONG 1759 14 895 3101 1748 3197 4860 -11 -M3 33 C99 SCMFLONG 1759 14 895 3101 1748 3197 4860 -11 -M3 33 C99 SCMFLONG 1045 8 504 2077 1169 2447 3078 168 -M2 41 C107 SCMFLONG 1045 8 504 2077 1169 2447 3078 168 -M2 41 C107 SCMFLONG 1045 8 504 2077 1169 2447 3078 168 -M3 41 C107 SCMFLONG 1045 8 504 2077 1169 2447 3078 168 -M4 41 C107 SCMFLONG 1045 8 504 2077 1169 2447 3078 168 -M4 41 C107 SCMFLONG 1045 8 504 2077 1169 2447 3078 168 -M3 41 C107 SCMFLONG 1045 8 504 2077 1169 2447 3078 168 -M4 55 C107 SCMFLONG 286 -4 270 1073 604 1114 1499 208 -M4 55 C107 SCMFLONG 286 -4 270 1073 604 1114 1499 208 -4 5	+M2													
+M3	-M2						-2098	5804						
Story Column   Load   P   M2   M3   L42(dli)D+0.5L   0.9*(dli)D   1.4(dli)D+1.7L   P   P   P   15   C107   SCMFLONG   3563   10   736   4720   2662   5072   8283   901	+M3			SCMFLONG	5870	17	2171	5804						
## 15 C107   SCMFLONG   3563   10   736   4720   2662   5072   8283   901   ## 15 C107   SCMFLONG   3281   20   770   4520   2549   4855   7801   732   ## 17 C99   SCMFLONG   3281   20   770   4520   2549   4855   7801   732   ## 20 C107   SCMFLONG   2997   5   857   4220   2380   5184   7217   617   ## 20 C107   SCMFLONG   2997   5   857   4220   2380   5184   7217   617   ## 21 C107   SCMFLONG   2894   20   851   4121   2324   4240   7015   570   ## 21 C107   SCMFLONG   2894   20   851   4121   2324   4240   7015   570   ## 21 C107   SCMFLONG   2894   20   851   4121   2324   4240   7015   570   ## 21 C107   SCMFLONG   2894   20   851   4121   2324   4240   7015   570   ## 21 C107   SCMFLONG   2894   20   851   4121   2324   4240   6980   535   ## 21 C99   SCMFLONG   2859   21   860   4121   2324   4240   6980   535   ## 21 C99   SCMFLONG   2859   21   860   4121   2324   4240   6980   535   ## 32 1 C99   SCMFLONG   2859   21   860   4121   2324   4240   6980   535   ## 32 1 C99   SCMFLONG   2859   21   860   4121   2324   4240   6980   535   ## 32 1 C99   SCMFLONG   2859   21   860   4121   2324   4240   6980   535   ## 32 1 C99   SCMFLONG   2859   21   860   4121   2324   4240   6980   535   ## 33 C99   SCMFLONG   2859   21   860   4121   2324   4240   6980   535   ## 31 C107   SCMFLONG   1966   3   646   3101   1748   3197   5667   218   ## 4 2 33 C99   SCMFLONG   1759   14   695   3101   1748   3197   4860   -11   ## 4 33 C99   SCMFLONG   1759   14   695   3101   1748   3197   4860   -11   ## 4 1 C158   SCMFLONG   1045   8   504   2077   1169   2147   3078   168   ## 4 1 C158   SCMFLONG   1045   8   504   2077   1169   2147   3078   168   ## 4 1 C167   SCMFLONG   1001   10   620   2077   1169   2147   3078   168   ## 4 1 C107   SCMFLONG   1001   10   620   2077   1169   2147   3078   168   ## 5 1 C107   SCMFLONG   396   4   270   1073   604   1114   1469   208   ## 5 1 C107   SCMFLONG   286   14   265   1073   604   1114   1459   318   ## 5 2 C107   SCMFLONG   286   14   265   1073   604   1114   1459   3	-M3	5	C157	SCMFLONG	-5870	-17	-2171	5804						
+P 15 C107 SCMFLONG 3563 10 736 4720 2662 5072 8283 -901   +M2 17 C99 SCMFLONG 3563 -10 -736 4720 2662 5072 8283 -901   +M2 17 C99 SCMFLONG 3281 20 770 4520 2549 4855 7801 -732   -M2 17 C99 SCMFLONG 3281 -20 -770 4520 2549 4855 7801 -732   -M3 20 C107 SCMFLONG 2997 5 857 4220 2380 5184 7217 -617   -M3 20 C107 SCMFLONG 2997 -5 -857 4220 2380 5184 7217 -617   -M3 20 C107 SCMFLONG 2894 20 851 4121 2324 4240 7015 -570   -F 21 C107 SCMFLONG 2894 20 851 4121 2324 4240 7015 -570   -F 21 C107 SCMFLONG 2894 20 851 4121 2324 4240 7015 -570   -M2 21 C99 SCMFLONG 2895 21 860 4121 2324 4240 6980 -535   -M3 21 C99 SCMFLONG 2859 21 860 4121 2324 4240 6980 -535   -M3 21 C99 SCMFLONG 2859 21 860 4121 2324 4240 6980 -535   -M3 21 C99 SCMFLONG 2859 21 860 4121 2324 4240 6980 -535   -M3 21 C99 SCMFLONG 2859 21 860 4121 2324 4240 6980 -535   -M3 21 C99 SCMFLONG 2859 21 860 4121 2324 4240 6980 -535   -M3 21 C99 SCMFLONG 2859 21 860 4121 2324 4240 6980 -535   -M3 21 C99 SCMFLONG 2859 21 860 4121 2324 4240 6980 -535   -M3 21 C99 SCMFLONG 1966 3 646 3101 1748 3197 5067 -218   -P 31 C107 SCMFLONG 1966 3 646 3101 1748 3197 5067 -218   -P 31 C107 SCMFLONG 1966 3 646 3101 1748 3197 5067 -218   -P 31 C107 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11   -M3 33 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11   -M3 33 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11   -M4 33 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11   -M4 31 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168   -M4 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168   -M4 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168   -M4 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168   -M4 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168   -M3 41 C107 SCMFLONG -286 14 265 1073 604 1114 1469 208   -M4 53 C107 SCMFLONG -286 14 265 1073 604 1114 1469 208   -M4 53 C107 SCMFLONG -286 14 265 1073 604 1114 1469 208   -M4 53 C107 SCMFLONG -286 14 265 1073 604 1114 1412 285		Story	Column	Load	P	M2	МЗ	1.42(dlf)D+0.5L	0.9*(dlf)D	1.4(dlf)D+1.7L	Pcomo	Ptens		
-P 15 C107 SCMFLONG 3281 20 770 4520 2549 4855 7801 -732   -M2 17 C99 SCMFLONG 3281 20 770 4520 2549 4855 7801 -732   -M3 20 C107 SCMFLONG 2997 5 857 4220 2380 5184 7217 -617   -M3 20 C107 SCMFLONG 2997 5 -857 4220 2380 5184 7217 -617   -M3 20 C107 SCMFLONG 2997 5 -857 4220 2380 5184 7217 -617   -M3 20 C107 SCMFLONG 2997 5 -857 4220 2380 5184 7217 -617   -M4 20 C107 SCMFLONG 2899 4 20 851 4121 2324 4240 7015 -570   -P 21 C107 SCMFLONG 2894 20 851 4121 2324 4240 7015 -570   -P 21 C107 SCMFLONG 2899 4 20 851 4121 2324 4240 7015 -570   -P 21 C107 SCMFLONG 2859 21 860 4121 2324 4240 6980 -535   -M2 21 C99 SCMFLONG 2859 21 -860 4121 2324 4240 6980 -535   -M3 21 C99 SCMFLONG 2859 21 -860 4121 2324 4240 6980 -535   -M3 21 C99 SCMFLONG 2859 21 -860 4121 2324 4240 6980 -535   -M3 21 C99 SCMFLONG 2859 21 -860 4121 2324 4240 6980 -535   -M3 21 C99 SCMFLONG -859 -21 -860 4121 2324 4240 6980 -535   -M3 21 C99 SCMFLONG -859 -21 -860 4121 2324 4240 6980 -535   -M3 21 C99 SCMFLONG -859 -21 -860 4121 2324 4240 6980 -535   -M3 21 C99 SCMFLONG -1966 3 846 3101 1748 3197 5067 -218   -P 31 C107 SCMFLONG 1966 3 646 3101 1748 3197 4860 -11   -M2 33 C99 SCMFLONG 1759 14 -695 3101 1748 3197 4860 -11   -M3 33 C99 SCMFLONG 1759 14 -695 3101 1748 3197 4860 -11   -M3 33 C99 SCMFLONG 1759 14 -695 3101 1748 3197 4860 -11   -M3 33 C99 SCMFLONG 1045 8 504 2077 1169 2147 3078 168   -P 41 C158 SCMFLONG 1045 8 504 2077 1169 2147 3078 168   -P 41 C158 SCMFLONG 1045 8 504 2077 1169 2147 3078 168   -P 41 C158 SCMFLONG 1045 8 504 2077 1169 2147 3078 168   -M2 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168   -M3 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168   -M43 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168   -M44 51 C107 SCMFLONG -286 14 265 1073 604 1114 1469 208   -M45 53 C107 SCMFLONG -286 14 265 1073 604 1114 1499 308   -M48 52 C107 SCMFLONG -286 14 265 1073 604 1114 1499 308   -M48 52 C107 SCMFLONG -286 14 265 1073 604 1114 1412 285		15	C107	SCMFLONG	3563	10								
-M2 17 C99 SCMFLONG 3281 20 770 4520 2549 4855 7801 -732 17 C99 SCMFLONG 3281 20 -770 4520 2549 4855 7801 -732 17 C99 SCMFLONG 3281 20 -770 4520 2380 5184 7217 -617 17 SCMFLONG 2997 -5 857 4220 2380 5184 7217 -617 17 SCMFLONG 2997 -5 -857 4220 2380 5184 7217 -617 17 SCMFLONG 2894 20 851 4121 2324 4240 7015 -570 18 SCMFLONG 2894 20 851 4121 2324 4240 7015 -570 18 SCMFLONG 2894 20 851 4121 2324 4240 7015 -570 18 SCMFLONG 2894 20 851 4121 2324 4240 6890 -535 18 SCMFLONG 2899 21 860 4121 2324 4240 6890 -535 18 SCMFLONG 2859 21 860 4121 2324 4240 6890 -535 18 SCMFLONG 2859 21 860 4121 2324 4240 6890 -535 18 SCMFLONG 2859 21 860 4121 2324 4240 6890 -535 18 SCMFLONG 2859 21 860 4121 2324 4240 6890 -535 18 SCMFLONG 2859 21 860 4121 2324 4240 6890 -535 18 SCMFLONG 2859 21 860 4121 2324 4240 6890 -535 18 SCMFLONG 2859 21 860 4121 2324 4240 6890 -535 18 SCMFLONG 2859 21 860 4121 2324 4240 6890 -535 18 SCMFLONG 2859 21 860 4121 2324 4240 6890 -535 18 SCMFLONG 2859 -21 -860 4121 2324 4240 6890 -535 18 SCMFLONG 2859 -21 -860 4121 2324 4240 6890 -535 18 SCMFLONG 2859 -21 -860 4121 2324 4240 6890 -535 18 SCMFLONG 2859 -21 -860 4121 2324 4240 6890 -535 18 SCMFLONG 2859 -21 -860 4121 2324 4240 6890 -535 18 SCMFLONG 2859 -21 -860 4121 2324 4240 6890 -535 18 SCMFLONG 2859 -21 -860 4121 2324 4240 6890 -535 18 SCMFLONG 2859 -21 -860 4121 2324 4240 6890 -535 18 SCMFLONG 2859 -21 -860 4121 2324 4240 6890 -535 18 SCMFLONG 2850 -3 -646 3101 1748 3197 4860 -11							-736							
-MZ 17 (99) SCMFLONG 3281 -20 -770								4520						
+M3				SCMFLONG	-3281	-20	-770	4520		27227.7				
Story   Column   Load   P   M2   M3   1.42(dlf)D+0.5L   0.9*(dlf)D   1.4(dlf)D+1.7L   P <sub>comp</sub>   P <sub>lans</sub>							857	4220						
+P 21 C107 SCMFLONG 2894 20 851 4121 2324 4240 7015 -570 -P 21 C107 SCMFLONG 2894 -20 -851 4121 2324 4240 7015 -570 -P 21 C107 SCMFLONG 2895 21 860 4121 2324 4240 6980 -535 -M2 21 C99 SCMFLONG 2859 -21 -860 4121 2324 4240 6980 -535 -M3 21 C99 SCMFLONG 2859 -21 860 4121 2324 4240 6980 -535 -M3 21 C99 SCMFLONG 2859 -21 860 4121 2324 4240 6980 -535 -M3 21 C99 SCMFLONG 2859 -21 860 4121 2324 4240 6980 -535 -M3 21 C09 SCMFLONG 2859 -21 860 4121 2324 4240 6980 -535 -M3 21 C09 SCMFLONG -8559 -21 860 4121 2324 4240 6980 -535 -M3 21 C09 SCMFLONG -8559 -21 860 4121 2324 4240 6980 -535 -M3 21 C09 SCMFLONG 1966 3 646 3101 1748 3197 5067 -218 -P 31 C107 SCMFLONG 1966 3 646 3101 1748 3197 5067 -218 -P 31 C107 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 -M2 33 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 -M3 33 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 -M3 33 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 -M3 33 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 -M3 33 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 -M3 34 C99 SCMFLONG 1045 8 504 2077 1169 2147 3122 124	-M3	20	C107	SCMFLONG	-2997	-5	-857	4220	2380					
+P						M2	МЗ	1.42(dlf)D+0.5L	0.9*(dlf)D	1.4(dlf)D+1.7L	Pcomp	Ptens		
-P 21 C107 SCMFLONG 2859 21 860 4121 2324 4240 6980 -535	10.00						851	4121	2324					
+M2 21 C99 SCMFLONG 2859 21 860 4121 2324 4240 6980 -535 + M3 21 C99 SCMFLONG 2859 21 860 4121 2324 4240 6980 -535 + M3 21 C99 SCMFLONG 2859 21 860 4121 2324 4240 6980 -535 + M3 21 C99 SCMFLONG 2859 -21 -860 4121 2324 4240 6980 -535 + M3 21 C99 SCMFLONG 2859 -21 -860 4121 2324 4240 6980 -535 + M3 21 C99 SCMFLONG 1966 3 646 3101 1748 3197 5067 -218 + M3 1 C107 SCMFLONG 1966 3 646 3101 1748 3197 5067 -218 + M3 23 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 + M3 33 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 + M3 33 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 + M3 33 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 + M3 33 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 + M3 33 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 + M3 33 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 + M3 33 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 + M3 32 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 + M3 34 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 + M3 34 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 + M3 34 C99 SCMFLONG 1045 8 504 2077 1169 2147 3122 124 + M2 41 C107 SCMFLONG 1045 8 504 2077 1169 2147 3122 124 + M2 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168 + M3 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168 + M3 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168 + M3 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168 + M3 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168 + M3 41 C107 SCMFLONG 396 4 270 1073 604 1114 1469 208 + M3 41 C107 SCMFLONG 396 4 270 1073 604 1114 1469 208 + M3 50 C107 SCMFLONG 286 14 265 1073 604 1114 1359 318 + M3 50 C107 SCMFLONG 286 14 265 1073 604 1114 1359 318 + M3 50 C107 SCMFLONG 286 14 265 1073 604 1114 1359 318 + M3 50 C107 SCMFLONG 286 14 265 1073 604 1114 1359 318 + M3 50 C107 SCMFLONG 286 14 265 1073 604 1114 1359 318 + M3 50 C107 SCMFLONG 339 2 282 1073 604 1114 1359 318 + M3 50 C107 SCMFLONG 339 2 282 1073 604 1114 1412 265							-851	4121						
-M2 21 C99 SCMFLONG -2859 -21 -860 4121 2324 4240 6980 -535   +M3 21 C99 SCMFLONG 2859 21 860 4121 2324 4240 6980 -535   -M3 21 C99 SCMFLONG -2859 -21 -860 4121 2324 4240 6980 -535    Story Column Load P M2 M3 1.42(dlf)D+0.5L 0.9*(dlf)D 1.4(dlf)D+1.7L Pomp Plens Plans P							860	4121	4 - 546	4240				
+M3							-860	4121	2324	4240	6980			
Story   Column   Load   P   M2   M3   1.42(dlf)D+0.5L   0.9*(dlf)D   1.4(dlf)D+1.7L   P <sub>comp</sub>   P <sub>tens</sub>								4121		4240				
+P 31 C107 SCMFLONG 1966 3 646 3101 1748 3197 5067 -218 -P 31 C107 SCMFLONG -1966 -3 -646 3101 1748 3197 5067 -218 -P 31 C107 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 -M2 33 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 -M3 33 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 -M3 33 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11 -M3 33 C99 SCMFLONG 1759 -14 -695 3101 1748 3197 4860 -11 -M3 33 C99 SCMFLONG 1759 -14 -695 3101 1748 3197 4860 -11 -M3 33 C99 SCMFLONG 1045 8 504 2077 1169 2147 3122 124 -P 41 C158 SCMFLONG 1045 8 504 2077 1169 2147 3122 124 -P 41 C158 SCMFLONG 1001 10 620 2077 1169 2147 3078 168 -M2 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168 -M3 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168 -M3 41 C107 SCMFLONG -1001 10 620 2077 1169 2147 3078 168 -M3 41 C107 SCMFLONG -1001 10 620 2077 1169 2147 3078 168 -M3 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168 -M3 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168 -M3 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168 -M3 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168 -M3 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168 -M3 52 C107 SCMFLONG 396 4 270 1073 604 1114 1469 208 -P 51 C107 SCMFLONG -286 -4 -270 1073 604 1114 1469 208 -M4 53 C107 SCMFLONG -286 -14 -265 1073 604 1114 1359 318 -M3 52 C107 SCMFLONG -286 -14 -265 1073 604 1114 1359 318 -M3 52 C107 SCMFLONG -286 -14 -265 1073 604 1114 1412 265	-M3	21	C99	SCMFLONG	-2859	-21	-860	4121	2324	4240	6980	-535		
+P 31 C107 SCMFLONG 1966 3 646 3101 1748 3197 5067 -218   +D 31 C107 SCMFLONG -1966 -3 -646 3101 1748 3197 5067 -218   +M2 33 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11   -M2 33 C99 SCMFLONG 1759 -14 -695 3101 1748 3197 4860 -11   +M3 33 C99 SCMFLONG 1759 14 695 3101 1748 3197 4860 -11   -M3 33 C99 SCMFLONG 1759 -14 -695 3101 1748 3197 4860 -11   -M3 33 C99 SCMFLONG 1759 -14 -695 3101 1748 3197 4860 -11   -M3 33 C99 SCMFLONG 1759 -14 -695 3101 1748 3197 4860 -11   -M3 33 C99 SCMFLONG 1045 8 504 2077 1169 2147 3122 124   -P 41 C158 SCMFLONG 1045 8 504 2077 1169 2147 3122 124   -P 41 C158 SCMFLONG 1001 10 620 2077 1169 2147 3078 168   -M2 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168   -M3 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168   -M3 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168   -M3 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168   -M3 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168   -M3 41 C107 SCMFLONG 396 4 270 1073 604 1114 1469 208   -P 51 C107 SCMFLONG 396 4 270 1073 604 1114 1469 208   -P 51 C107 SCMFLONG 286 14 265 1073 604 1114 1469 208   -M2 53 C107 SCMFLONG -286 -14 -265 1073 604 1114 1359 318   -M3 52 C107 SCMFLONG 339 2 282 1073 604 1114 1412 265						M2	МЗ	1.42(dlf)D+0.5L	0.9*(dlf)D	1.4(dlf)D+1.7L	Pcomp	Ptens		
+M2						3	646	3101						
-M2	1000					-3	-646	3101	1748	3197	5067	-218		
+M3							0.57	3101	1748	3197	4860	-11		
-M3 33 C99 SCMFLONG -1759 -14 -695 3101 1748 3197 4860 -11  Story Column Load P M2 M3 1.42(dlf)D+0.5L 0.9*(dlf)D 1.4(dlf)D+1.7L P <sub>comp</sub> P <sub>tens</sub> +P 41 C158 SCMFLONG 1045 8 504 2077 1169 2147 3122 124 -P 41 C158 SCMFLONG -1045 -8 -504 2077 1169 2147 3122 124 +M2 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168 -M2 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168 +M3 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168 -M3 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168 -M3 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168 -M5 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168 -M6 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168 -M7 51 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168 -M8 51 C107 SCMFLONG 396 4 270 1073 604 1114 1469 208 -P 51 C107 SCMFLONG 396 -4 -270 1073 604 1114 1469 208 -M8 53 C107 SCMFLONG 286 14 265 1073 604 1114 1459 318 -M8 52 C107 SCMFLONG -286 -14 -265 1073 604 1114 1359 318 -M8 52 C107 SCMFLONG 339 2 282 1073 604 1114 1412 265									1748	3197	4860	-11		
Story Column Load P M2 M3 1.42(dlf)D+0.5L 0.9*(dlf)D 1.4(dlf)D+1.7L P <sub>comp</sub> P <sub>tens</sub> +P 41 C158 SCMFLONG 1045 8 504 2077 1169 2147 3122 124 -P 41 C158 SCMFLONG -1045 -8 -504 2077 1169 2147 3122 124 +M2 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168 -M2 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168 +M3 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168 -M3 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168 -M3 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168 -M5 Story Column Load P M2 M3 1.42(dlf)D+0.5L 0.9*(dlf)D 1.4(dlf)D+1.7L P <sub>comp</sub> P <sub>tens</sub> +P 51 C107 SCMFLONG 396 4 270 1073 604 1114 1469 208 -P 51 C107 SCMFLONG -396 -4 -270 1073 604 1114 1469 208 +M2 53 C107 SCMFLONG 286 14 265 1073 604 1114 1359 318 -M2 53 C107 SCMFLONG -286 -14 -265 1073 604 1114 1359 318 -M3 52 C107 SCMFLONG 339 2 282 1073 604 1114 1412 265				SCMFLONG	1759	14	695		1748	3197	4860	-11		
+P 41 C158 SCMFLONG 1045 8 504 2077 1169 2147 3122 124 -P 41 C158 SCMFLONG -1045 -8 -504 2077 1169 2147 3122 124 +M2 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168 -M2 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168 +M3 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168 -M3 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168 -M3 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168 -M5 Story Column Load P M2 M3 1.42(dlf)D+0.5L 0.9*(dlf)D 1.4(dlf)D+1.7L P <sub>comp</sub> P <sub>lens</sub> +P 51 C107 SCMFLONG 396 4 270 1073 604 1114 1469 208 -P 51 C107 SCMFLONG -396 -4 -270 1073 604 1114 1469 208 +M2 53 C107 SCMFLONG 286 14 265 1073 604 1114 1359 318 -M2 53 C107 SCMFLONG -286 -14 -265 1073 604 1114 1359 318 -M3 52 C107 SCMFLONG 339 2 282 1073 604 1114 1412 265	-IVI3	33	C99	SCMFLONG	-1759	-14	-695	3101	1748	3197	4860	-11		
-P 41 C158 SCMFLONG -1045 -8 -504 2077 1169 2147 3122 124 +M2 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168 -M2 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168 +M3 41 C107 SCMFLONG 1001 10 620 2077 1169 2147 3078 168 -M3 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168 -M3 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168  Story Column Load P M2 M3 1.42(dlf)D+0.5L 0.9*(dlf)D 1.4(dlf)D+1.7L P <sub>comp</sub> P <sub>lens</sub> +P 51 C107 SCMFLONG 396 4 270 1073 604 1114 1469 208 -P 51 C107 SCMFLONG -396 -4 -270 1073 604 1114 1469 208 +M2 53 C107 SCMFLONG 286 14 265 1073 604 1114 1359 318 -M2 53 C107 SCMFLONG -286 -14 -265 1073 604 1114 1359 318 +M3 52 C107 SCMFLONG 339 2 282 1073 604 1114 1412 265						M2			0.9*(dlf)D 1	.4(dlf)D+1.7L	Роопр	Ptens		
+M2												124		
-M2						7.1						124		
+M3												168		
-M3 41 C107 SCMFLONG -1001 -10 -620 2077 1169 2147 3078 168  Story Column Load P M2 M3 1.42(dlf)D+0.5L 0.9*(dlf)D 1.4(dlf)D+1.7L P <sub>comp</sub> P <sub>lens</sub> +P 51 C107 SCMFLONG 396 4 270 1073 604 1114 1469 208 -P 51 C107 SCMFLONG -396 -4 -270 1073 604 1114 1469 208 +M2 53 C107 SCMFLONG 286 14 265 1073 604 1114 1359 318 -M2 53 C107 SCMFLONG -286 -14 -265 1073 604 1114 1359 318 +M3 52 C107 SCMFLONG 339 2 282 1073 604 1114 1412 265												168		
Story Column Load P M2 M3 1.42(dlf)D+0.5L 0.9*(dlf)D 1.4(dlf)D+1.7L P <sub>comp</sub> P <sub>lens</sub> +P 51 C107 SCMFLONG 396 4 270 1073 604 1114 1469 208 -P 51 C107 SCMFLONG -396 -4 -270 1073 604 1114 1469 208 +M2 53 C107 SCMFLONG 286 14 265 1073 604 1114 1359 318 -M2 53 C107 SCMFLONG -286 -14 -265 1073 604 1114 1359 318 +M3 52 C107 SCMFLONG 339 2 282 1073 604 1114 1412 265							620							
+P 51 C107 SCMFLONG 396 4 270 1073 604 1114 1469 208 -P 51 C107 SCMFLONG -396 -4 -270 1073 604 1114 1469 208 +M2 53 C107 SCMFLONG 286 14 265 1073 604 1114 1359 318 -M2 53 C107 SCMFLONG -286 -14 -265 1073 604 1114 1359 318 +M3 52 C107 SCMFLONG 339 2 282 1073 604 1114 1412 265	-IVI3	41	C107	SCMFLONG	-1001	-10	-620	2077	1169	2147	3078	168		
-P 51 C107 SCMFLONG -396 -4 -270 1073 604 1114 1469 208 +M2 53 C107 SCMFLONG 286 14 265 1073 604 1114 1359 318 -M2 53 C107 SCMFLONG -286 -14 -265 1073 604 1114 1359 318 +M3 52 C107 SCMFLONG 339 2 282 1073 604 1114 1412 265							МЗ		0.9*(dlf)D 1	.4(dlf)D+1.7L	P <sub>comp</sub>	Plens		
+M2 53 C107 SCMFLONG 286 14 265 1073 604 1114 1359 318 -M2 53 C107 SCMFLONG -286 -14 -265 1073 604 1114 1359 318 +M3 52 C107 SCMFLONG 339 2 282 1073 604 1114 1412 265									604	1114	1469	208		
-M2 53 C107 SCMFLONG -286 -14 -265 1073 604 1114 1359 318 +M3 52 C107 SCMFLONG 339 2 282 1073 604 1114 1412 265									604	1114	1469	208		
-M2 53 C107 SCMFLONG -286 -14 -265 1073 604 1114 1359 318 +M3 52 C107 SCMFLONG 339 2 282 1073 604 1114 1412 265									604	1114	1359	318		
+M3 52 C107 SCMFLONG 339 2 282 1073 604 1114 1412 265									604	1114		318		
-M3 52 CTU/ SCMFLONG -339 -2 -282 1073 604 1114 1412 265									604	1114	1412	265		
	-W3	52	L10/	SCMFLONG	-339	-2	-282	1073	604	1114	1412	265		

dlf = dead load factor (see load take down)

Group #	D												
Gloup p	Story		Column	Load	P	M2	МЗ	1.42(dlf)D+0.5L	0.0*(3100)	ACAIGNAL TI		D	
+P			C161	SCMFSHORT	673		1674						
-P		40.0	C161	SCMFSHORT	-6731		-1674			077.70	13270		
+M2	0,,0		C165	E100X100Y5 MAX							13270 7912		
-M2			C165	E100X100Y5 MAX						6599			
+M3			C170	SCMFSHORT	6630		6975			6599		1425	
-M3			C170	SCMFSHORT			-6975				12988		
INIO		-	0170	SOMI SHOKI	-0030	-55	-09/3	6358	2979	0099	12988	-3051	
	Story		Column	Load	P	M2	МЗ	1.42(dlf)D+0.5L	0.9*(dlf)D 1	.4(dlf)D+1.7L	Pcomp	Ptens	
+P		4	C161	SCMFSHORT	5956	17	5074				12031		
-P		4	C161	SCMFSHORT	-5956	-17	-5074		4 - 5 - 4 - 4		12031		
+M2		4	C170	E100X100Y5 MAX	1395	127	1051		2838	6304			
-M2		4	C170	E100X100Y5 MAX					2838		7470		
+M3		4	C161	SCMFSHORT	5956		5074		2838		12031		
-M3		4	C161	SCMFSHORT			-5074		2838		12031		
	Story		Column	Lond	D	110	***	1 40/1100 10 51	0.04/11070.1	4/110m : 1 m			
40	Sibiy		Column		P	M2		1.42(dlf)D+0.5L				Ptens	
+P			C164	SCMFSHORT	2491		733.2		2204	4973	7267	-287	
-P			C164	SCMFSHORT	-2491		212.2	27100	2204	4973	7267	-287	
+M2			C165	E100X100Y5 MAX					2104	4753	4994	1673	
-M2			C165	E100X100Y5 MAX					2104	4753	4994	1673	
+M3			C170	SCMFSHORT	2207		3447		2104	4753	6770	-103	
-M3		31	C170	SCMFSHORT	-2207	-5	-3447	4563	2104	4753	6770	-103	
	Story		Column	Load	P	M2	МЗ	1.42(dlf)D+0.5L	0.9*(dlf)D 1.	4(dlf)D+1.7L	Pcomo	Ptens	
+P		21	C164	SCMFSHORT	2155		2715		1902	4314	6291	-253	
-P		21	C164	SCMFSHORT	-2155		-2715	4136	1902	4314	6291	-253	
+M2		21	C165	E100X100Y5 MAX	387			4136	1902	4314	4523	1515	
-M2			C165	E100X100Y5 MAX			-574	4136	1902	4314	4523		
+M3			C171	SCMFSHORT	2155		2727	4136	1902	4314	6291	-253	
-M3			C171	SCMFSHORT	-2155		-2727	4136	1902	4314		-253	
	01		A 1	rest.	2				earlialetif A	Salare A ac		EI	
CE TO	Story		Column		P			1.42(dlf)D+0.5L	0.9*(dlf)D 1.	4(dlf)D+1.7L	Pcomp	Ptens	
+P			C164	SCMFSHORT	1549			3042	1389	3176	4591	-160	
-P				SCMFSHORT	-1549	-3	-351	3042	1389	3176	4591	-160	
+M2			C165	E100X100Y5 MAX	243	27	203	2836	1293	2966	3079	1050	
-M2			C165	E100X100Y5 MAX	-243	-27	-203	2836	1293	2966	3079	1050	
+M3				SCMFSHORT	1448	2	1140	2836	1293	2966	4284	-155	
-M3		33	C171	SCMFSHORT	-1448	-2	-1140	2836	1293	2966	4284	-155	
	Story		Column	Load	P	M2	Ма	1.42(dlf)D+0.5L	1 CONTACT	MAIND+1 7I	D	D	
+P				SCMFSHORT	832	1	280					Ptens	
-P				SCMFSHORT	-832	-1		2077	1169	2147	2909	337	
+M2				E100X100Y5 MAX			-280	2077	1169	2147	2909	337	
-M2					141	11	63	2077	1169	2147	2218		
+M3				E100X100Y5 MAX	-141	-11	-63	2077	1169	2147	2218	1028	
				SCMFSHORT	724	1	761	2077	1169	2147	2801	445	
-M3		44	C159	SCMFSHORT	-724	-1	-761	2077	1169	2147	2801	445	
	Story		Column	Load	P	M2	МЗ	1.42(dlf)D+0.5L	0.9*(dif)D 1.	4(dlf)D+1.7L	Pcomp	Ptens	
+P		53	C164	SCMFSHORT	330	1	477	817	364	876	1147	34	
-P				SCMFSHORT	-330	-1	-477	817	364	876	1147	34	
+M2				E100X100Y5 MAX	66	6	79	817	364	876	883	298	
-M2				E100X100Y5 MAX	-66	-6	-79	817	364	876	883	298	
+M3				SCMFSHORT	330	1	477	817	364	876	1147	34	
-M3				SCMFSHORT	-330	-1	-477	817	364	876	1147	34	
												-5.0	

dif = dead load factor (see load take down)

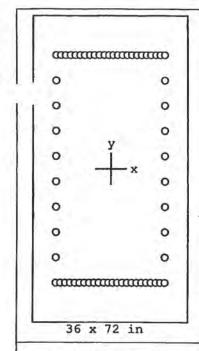




Clear spacing = 0.31 in

Beta1 = 0.65

Clear cover = 3.50 in 8.2.1-13



Code: ACI 318-95

Units: English

Run axis: About X-axis

Run option: Investigation

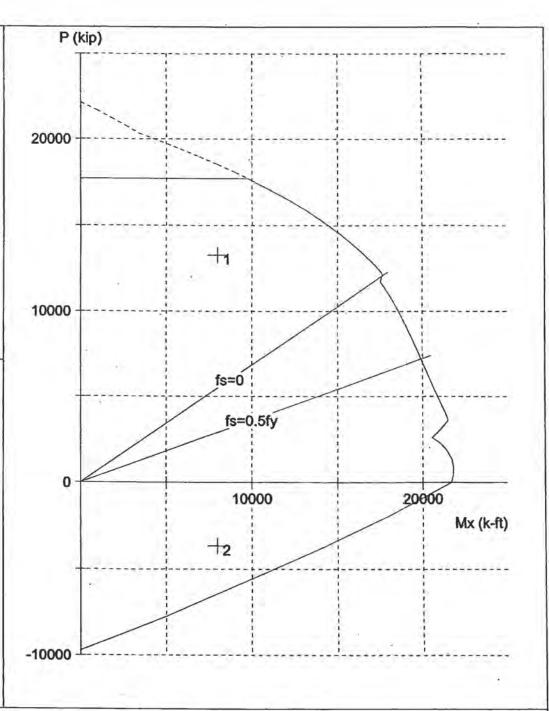
'enderness: Not considered

Column type: Structural

Bars: ASTM A615

Date: 01/03/05

Time: 11:35:51



PCACOL V3.00 (PCA 1999) - Licensed to: Licensee name not yet specified.

Group D
File: F:\PROJECTS\4069\PCA\TOWER\CURRENT\SHORT\_~2\E-B-4.COL

Project:

Engineer:

Column: fc = 10 ksi

fy = 75 ksl

Ag = 2592 in^2

64 #14 bars

LEVEL B -> LOY

Ec = 5000 ksi

Es = 29000 ksi

As = 144.00 in^2

Rho = 5.56%

fc = 8.5 ksi

e\_rup = Infinity

Xo = 0.00 in

Ix = 1.11974e+006 in^4

e\_u = 0.003 in/in

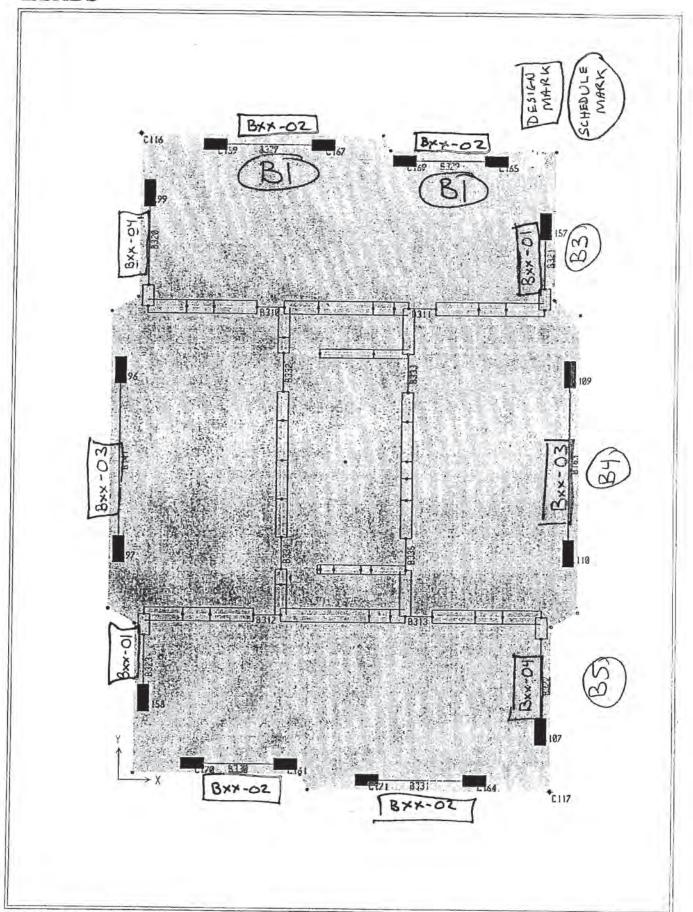
 $Y_0 = 0.00 in$ 

ly = 279936 in^4

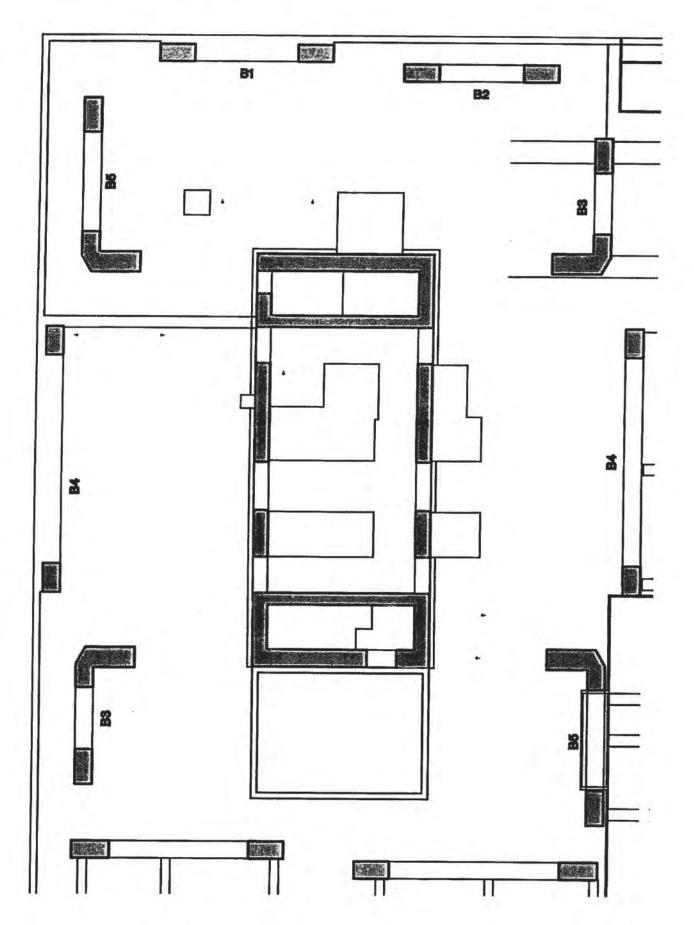
Beta1 = 0.65

Clear spacing = -0.59 in

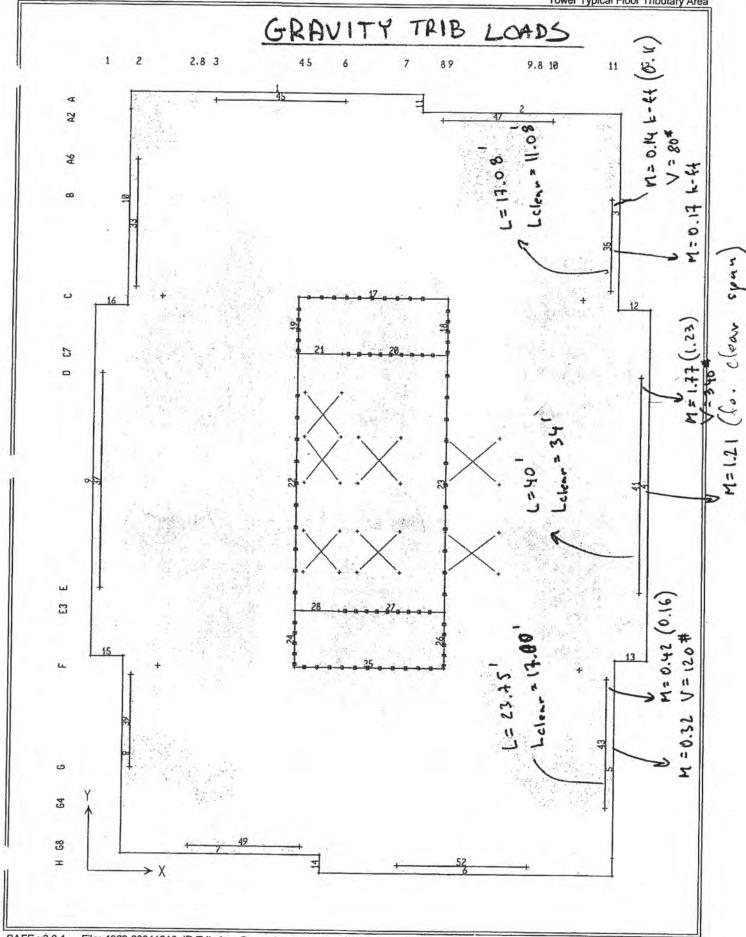
Clear cover = 4.50 in 8.2.1-14



ETABS v8.4.7 - File: 4069-20041206-PeriMF 2.03 - 25MF - December 10,2004 9:21 Plan View - 20 - Elevation 190.25 - Kip-ft Units



8.2.1-16



SAFE v8.0.1 - File: 4069-20041210-JP-Trib-A - December 10,2004 16:53 - Scale: Fit to Page Structural Layer. Plan View - Kip-ft Units

8.2.1-17

2. Val. Nat. Val. Co. P. P. L. Labellander C. M. State M.	Mused	Design Design	tip-ft	157	121	-120	-120	-120	2 4	*	16	F 1	100	16	-93	-93	66.	G 6	101	101	-101	101-	-101	101-	101-	191-	-101	72	ti i	4 6	1	2.	77	7	7 5		F.	F	1	-	38	S,	2	7	38	28	-58	200	90 0	38
Correction   Cor		Delgn Delgn	No-it	4	5 19	19	9	Ģ	9 9	7	4	9	7 4	4	-48	÷	4	4 9	7 5	7	.53	-53	-53	Ş	2 0	3 13	9	27.	7	2 2	348	-38	-38	38	36	7.47	38	Ħ	25. E	7 7	7	31	31	77	7	31	31	15	6	16
Section   Corp.   Part   Marketon   Market		Am-05t.	u-de	148	907	901-	-100	選	g s	1 12	şj	EJ S	4 4	42	Ŋ	st i	2 5	2 9	8	8	8.	-30	96-	R	8	8	8	\$	3	* 4	4	4	*	Ģ	ą	9 49	4	q	<b>\$</b> \$	3 5	9	S.	zą c	70	S	i c	Ç	75	r, r	9
1997   1997		_	ll-skx	or 9	92	91	91	91	9 5	n	13	m :	2 (5)	13	12	2:	2.0	2 #	22	11	13	13	2	2 1	1 11	2 22	13		0. 0	0		6	6	O1 (0	. 0	- en	9	6	o. o		4	7	-	-	7	1	7			,
Curr   Took   Miska   Light		Monacat Cent	The state of	- 2	11	14	ž:	2 :	12	11	n	= =	==	H	11	1	4.5	7 17		11	311	11	11		: :		и				,	. 2	7	7			7	7	2 6			40	10 V	0 40	. 0	9			9 4	100
Curr   Took   Miska   Light	1	Money Ead	n-der	2 1	2	69	9	S 6	8 34	×	x	* 2	35	x	x	X :	t	t z	37	LS.	25	23	25	20	15	25	.15	9		4	19	7	7	7 1		19	41	Q.		7 9	N	14	4 5	2 23	X	22	Ħ	R	24 22	31
Core   Dec	3		and and	71	P	20	R	RF	2 3	z	×	3, 3	1 25	25	3	* 3	, 3		85	83	20	B	8	8 8	8	81	ħ	Q I	2 5	q	42	4	G	2 0	9	9	0	<b>4</b> 5	7 0	9	*	*	<b>x</b> 3		×	A	75	X:	# 2	15
Core   Dec	50	Memoral Ead N					-	-	n m	m	2	4 0	2	24	es i	40			14	4		2				24	2		40	. 14		2	*	4 6	e ru	7	. 2		4 14	4	2	5				1	0			T
Char Doe Doe   Action   Acti	L'exterior dado	second Confer		g 40		n	5			+	•		,	*						*	4	*			4		+	-		m	m	141	m s	40	. 69	1	2	r4 F	4 14	14	24	7	4 6	100	2	13		e 1	* **	74
No. 1, 1997   Char   Floor Photo   Martiness   Marti	Ш	Special East Ma		. 1	11	*	1 2	2	u	.0	4		u	-	0 :				14	14	14	Y :				H	3	9 9	10	01	10	0	0 9	2 0	00	10	01	9.5	2 2	01	0.	Gh G	n o	· o	0	OA.	<b>6</b> 1	D 6		
Ches   Dec	Digital	Outh	ł	1 12	21	1	7.5	3 2	9	91	36	2 12	16	16	10	16	16	16	22	77	21	22	2 2	a	R	27	n:	9 9	9	91	16	16	16	16	16	16	91	4 4	18	91	32	51 6	2 2	2	15	13	18	15	13	15
Apple   Char   Prop.   Part   Abstract   A	DL S.	Mer Ead Mo	a	i ca	8	2 :	2 5	2 50	¥	41	7 7		7	= 1	2 5	41	41	4	7	14:	<b>=</b> :	2 :	7 4	41	41	41	4 2	o p	R	R	R	8	2 2	20	29	83	21	9 2	A	20	111	a F	12	12	22	22	1111	2 2	n	ដ
Char   Dept.   Large   Dept.	SDL	red Couter Ma	8	. 0	40		40	2	34	7.2	1,2	. 25	35	* * *	5.7	7	x	25	Z.	*	* 3	77		3	25	X:				2	*			. 20	*	*	* >		*	*	DE 6					m i		e0 pe		10
Char   Dept.   Large   Dept.	Marine	net East Monne																																															7	
Mark   Dept.   Logd Month   L	PSF Mexim	Center Moun	0.4																			35	0.3	0.3	031	63	0.3	0.20	0.23	0.20	0.22	200	0.20	0.22	0.22	0.20	27.0	0.22	0.22	023	0.17	0.17	0.17	0.17	0.17	0.17	210	0.17	71.0	0.17
4 4 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	出 量	9 7	0.33		0.338	0.138	0.338	0.338	0.763	0.367	0.767	0.263	0263	0.263	0.263	6263	6320	0.263	0.263	0.263	0.263	0.263	0.263	0.263	0.253	570	0.186	0.186	0.586		20.00	0110	0.186	0.136	0.136	0.185	0.186	0.186	0.146	0.186	0.140	0.140	0.140	0.145	0.360	0.140	0.10	0.140	0,140	0.140
# ** ** ** ** ** ** ** ** ** ** ** ** **	r Phot	of the		8 40	2 2		9 00	9			13	9 8	9 4	9 9	9	8 40	\$	9 5	9	7 5	40	90	8						9	2 :	3 5	9	8	Ž,	3		4	4	9	9 5	2 2	\$	9	9 4	8 8	7 9	1 13	3	8	9.8
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**	ð	l Leg		200	9 9	91 9	9 16	91 9	9 143	9 14	9 141	6 14.1	9 14 14	9	6 14.3	6 143	3	9		7							12.5	12.5	123	123	3.5	12.5	12.5	12.5	2	200	12.5	12.5	128	12.5	10.83	10.83	10.83	10.83	10.83	10.83	10.83	10.83	10.83	10.83
**		th Dept	7	* *			*				*	4										-	*		4 4		*	*	7	2 4	7 %	19	*	\$	3 :	5.4	2	45	9 :	9 4	1 50	45	\$ :	9 %	2 52	8	2	45	2 :	7
		Ster Wild	605	785	2	17.5	1.75 2	538	dk	12	75 3	25	25 3	20	25	21	200	2 4	1 15	75 2	75 24	20 20	75 24	Q H	2 %	3 22	25 22	12 34	2	2 8	200	52	75 24	2 2	2 2	7 7 2	22 22	15 24	**		30	30	21	S S	200	2 2	S 30	5 30		2 2
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	Maximum			\$	# B	2 5	t g	2 26	*	89	E 1	2 0	2	88	38	88	2 1	3 5	2 2	3 33	88	28 :	2 2	2 23	88	22 2	2 12	12	P	K	E F	P.	N.	12 1	2 12	200	78	27 27	*	78	-	7	11	-	2 12	112	11	7.1	5.1	11
,	Maximum	Center Mament End Mouseau No-ft Ke-ft		457	314	310	310	310	310	281	7 5	7.00	279	279	279	279	E !	278	270	303	303	2	101	303	303	96	200	192	267	267	190	267	267	363	265	265	592	286	265	265	9 9	266	260	580	280	250	260	250	007	107
Total DL				089	468	797	462	462	9	619	419	407	417	487	417	407	417	417	417	455	453	63	455	433	639	69	9	399	389	386	100	400	350	197	200	397	397	162	397	397	386	38	389	360	388	388	388	389	260	252
dding	wf734 3	Center Moment End Moment kip-ft kip-ft		15 2	ri r	182	: 25	81	20	100	ec 10	91	92	20	16	20	0 4	9	9	16	90	36	9	91	91	9 4	16	14	14	2 2	2 2	15	7	20 5	2 53	2)	នា	2 2	13	13	2 2	12	71	2 2	1 22	17	12	27	11	13
DL exterior cladding	8/Ja	Montest Center		a :	2 2	i Pa	R	24	27	27	N X	3 5	n	22	n	2	a x	3 20	i A	22	23	X 7	1 12	n	X :	q x	n	77	n	2 2	12	n	12	R	20	20	20	2 2	20	2	==	25	10 1	10	100	803	18	80 c	21	**
(dais se	wf/34	Moment 2.84		= =	1	1	.12		11	21	2 2	2 8	R	2	8	RI	2 6	R	2	63	63	E 8	2 22	93	2 :	2 2	8	2	22 22	2 2	1 21	82	82	2 0	1 23	a	2 1	2	Z.	2 :	16	66	56	10	93	15	83	2 2	2 5	2.7
DL. Self wt (excludes slab)	Maximum with with with with	Mp-ft		130	2 2	E E	120	120	120	100	108	101	101	104	101	20 20	100	108	106	1	4	3 3	191	141	3 :	1	141	127	127	127	121	177	127	122	121	121	127	127	127	127	145	145	200	16	145	145	16	140	145	1
D)	Marie Sed	Hp-ft.		214	214	214	234	214	214	193	193	193	193	183	193	183	193	193	193	193	193	161	193	193	191	193	193	23	170	2 2	170	170	2,5	13.5	170	21	2.5	170	021	22	2 15	155	155	155	155	155	155	200	155	
SOL	thrum A	Ep-8	434	314	314	314	314	314	314	70	38	283	283	283	283	282	283	283	283	283	283	283	283	283	202	1 22	225	269	246	249	249	249	249	249	249	249	160	269	249	202	m	227	E E	122	227	227	227	202	227	
H	Marinosa Marinosa Center Moment End Moment	Mp-ft	1696	1,675	1.675	1.675	1.675	1,675	1,675	1511	1,511	1511	1311	1511	1311	1211	1311	1511	1,511	1211	150	1311	1.511.	1.511	151	1211	1511	1332	312	1332	1,332	1332	1332	1332	1332	1332	1312	1,332	1.332	1,210	1210	1210	1210	1210	1210	1210	1210	210	.210	
1	g F	4	430	450	450	980	450	057	111	1123	112	216	111	111	111	211	111	1211	111		-	10	101	211	11	31	=======================================	2 25	48	48	報	8 5	2 15	3	22	84	2 22	1.948	25	2 8	04	P	9	9	2	2		. 0	9	
	Meni End Mo	Prof.	2 2 2	40 2	40 2	8 8	9	94	40	2 2	5	40	9 9	2 6	200	40	60 2:	40 2:	2	2 5	2 2 2	17	22	2 0	12			91		61 0	0 0	900	100	0 1.9	61 0	91	139	61 0	61 0	17	1.7	17	1,778	1.7	17	1.7	1	1.7		
	Ploter Fi	W.	2 2	128	128	128	27	22	18	178	178	128	178	178	2	128	128	128	128	126	171	178	178	128	128	171	11	1 22	123	128	821	178	173	128	22	22	128	128	22	7	178	9 2	178	128. 4	128	87	77	178 4	128 40	i
	1		40.00	40.00	40.00	40.00	40.00	40.00	38.00	38,00	38.00	38.00	38,00	38.00	38.00	38.00	38.00	38.00	38.00	18.00	38.00	38.00	38,00	38.00	38.00	38.00	15.00	35.67	35.67	35.67	1958	15.67	15,67	15,67	15.67	18.81	15.67	19'91	19.07						87.8					
	Depth	g	28	35	* 3	2 2	2 3	2 2	K	R	M.	2 3	g 15	25. 50	38	36	20	22 2	2 5	1 50				2 49		9:	7 4	. 2	45	9	9 1	*	9	4	9 4	2 2	2	2 :	3 2	45	9	3 5	45	45	2 :	7 4	2 59	45 3	45	-
J	Wilden Depth	4	25	75	2 2	77.	3 7	* 2	H	28	*	3 3	2.0	×	7	z	74	2 2	5.2	5 25	*	×	* *	7 7	8	4 :	4.2	22	34	24	2 2	7	Z	77	7 7	75	74	* 7	3 2	30	90	2 00	8	2	RS	2,9	2	9	25	
	Bevados	R. F. F. F.	808	285	579	557.K	546.75	536	525.25	512.75	502.25	491.75	470.75	480.25	449.75	439.25	428.75	418.25	307.26	386.75	376.25	366.75	37.4476	334.25	\$23.75	313.25	282.75	281.75	271.25	260.75	270.75	228.25	218,75	208.26	18.70	180.75	171.25	167.73	142.75	133.25	123.75	104.75	85.25	88.75	55.75 55.75	57.75	67.75	38.25	28.75	40 507.22
ı	Fir. Hr.	22.5	13	2	10.75	10.75	10.75	10.75	12.5	10,5	10.5	10.0	10.5	10,5	10.5	10.5	10.5	10.5	10.5	10.5	10.5	50	10.5	10.5	10.5	10.5	10.5	10.5	10.5	10.5	115	9.5	9.5	9 9	9 69	9.5	(g)	0 0	8.5	9.0	0 0	9.5	65	200	2 00	22	9.00	60.5	/00/	
		Roof	Roof	A.	45	Typ	Typ	17.0	Mech	T)p	25	100	Type I	Type	Typ	Die .	D.	1		60	D'A	Pro-	d a	de	45	100	1,5	46	6	8.5	1	30	30	23	2.5	· A	2	di di		1	20	B	0	2		. 0			12,155/	
			8	28 0	3 8	8 8	2	3	23	15 S	3 3	2 5	4	48	45	3	3 5	1 17	9	39	88	100	1 19	×	2	N F	8							3 2			18	19	15 7	7	2 5	11 3	P		7 7	6 T	10	4	2 197	A Bress

DX_Self wt (eurologes self)   DX_enterior classifing   DX_enterior classifing   DX_enterior classifing   DX_enterior classifing   DX_enterior classifing   DX_enterior classifing   DX_enterior classification   DX_enter	March   Dit Soul for (Note to bedden)   Dit contravy collection   March   Ma
75, estactor clindeling   75, estactor clind	December obsolving
	Tooj Cl.   Tooj Cl.

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Bxx-0		Laconomic	A Green and the second	G days		Vinne.	
<u> </u>		Bean			V2		M3
+M3	_	B323		(		_	
+V2		B323			36	_	225
被斗下	3	B321	E100X5100Y MAX	14.0834	233	3 27	146
J. 11	Story	Bean	Load Sale	Loc	V2.	T	M3.
<b>4M3</b>	6	B321	SCMFLONG	14.0834	41	_	255
+V2	6	B321	SCMFLONG	14.0834	_	7	255
包括丁	11	B321	E100X5100Y MAX		-	_	
	Sforv	Ream	Load 11 × 5 (4)	Loc wat	h/n	F	hio
+M3		B323	SCMFLONG			_	
+V2		B323	SCMFLONG	0	-	_	105
¥T		B321		0		_	105
(金元)	20	10021	E100X5100Y MAX	14.0834	127	20	78
	Story	Beam	Load S	Loc A	V2.5	TM	M3
<b>3</b> M3		B323	SCMFLONG	0		-	878
+V2	21	B323	SCMFLONG	0	142	-	878
經红		B321	E100X5100Y MAX		100	20	623
1	Ptoni	Doom	LI LANGE BY LANGE BY CO.	H. A. L. Sales	is once	-	1 14 17
			Load A COME	1	-	_	M3.
+M3	_	B323	SCMFLONG	0	84	_	524
+V2	$\overline{}$	B323 B323	SCMFLONG E100X5100Y MAX	0	84 52	12	319
Bxx-03		Beam	Note that the second second	In assert to a part	100	学示	1.10
+M3		B141	Load	Loc	V26		М3
1+V2			SCMFLONG	3	70	6	1189
		B141	SCMFLONG	3	70		1189
總紀下	3	B141	E100X5100Y MAX	3	46	27	775
15	tory	Beam	Load M	Loc -	V24	一	M3,4
+M3	9	B163	SCMFLONG	3	101		1721
+V2	9	B163	SCMFLONG	3	101	_	1721
和主工	10	B163	E100X5100Y MAX	3	64	_	1090
Is	tory l	Ream!	Load States	h economic	1794	Test	No A
<b>3</b> M3	16	B141	SCMFLONG			$\overline{}$	
+V2		B141	SCMFLONG	3	59	5	996
ATT.	$\overline{}$	B141	E100X5100Y MAX	3	59 36	19	996
					50	10]	013
	tory I	Beam'	Load 透過時代後世	Loc -	V2.	Tall	M3 =
+M3	21	3141	SCMFLONG	3	50	5	849
FV2	21	3141	SCMFLONG	3	50	5	849
解开	22	3141	E100X5100Y MAX	3	$\overline{}$	21	539
- 12	rte sole	3eam	Load All a Dev	Loc	10:1	ra la	10.00
152				THE PERSON NAMED IN	17.2 Will.	1.06 48	A Chief
					V2		
+M3	44 E	3141	SCMFLONG	3	21	3	360
	44 E						

2.09 UPDATE: 5/21/05
ALL M3 ARE

I 5% OF PREVIOUS

DESIGN.

Bxx-	04						
		Beam	Load	Loc	V2	T	M3 2
+M:		B320	SCMFLONG	0	224	-	_
+V2	3	B320	SCMFLONG	0	-	_	
#3-1			E100X5100Y MAX		_	_	-
	Story	Beam	Load Washington	Loc 1	V2 %	Te	M3 ₹
3+M3	8	B322	SCMFLONG	20.7467			
FIV2	8	B322	SCMFLONG	20.7467	298	-	
器扩	9	B320	E100X5100Y MAX	0	_		_
1	Story	Beam	Load	Loc	V25	Ta	M3-
+M3	8	B322	SCMFLONG	20.7467	298	8	
#V2	8	B322	SCMFLONG	20.7467	298	8	
不到了	. 9	B320	E100X5100Y MAX	0	_	39	
	Störy!	Beam	Load	Loc I	V2	TE	МЗ
<b>НМ3</b>	16	B320	SCMFLONG	0	164	7	1564
\$+V2	16	B320	SCMFLONG	0	164	7	1564
\$2+T	20	B320	E100X5100Y MAX	0	107	20	1023
	Story	Beam	Load 新作业的	Locy	V2	Te	M3 ***
+M3	21	B322	SCMFLONG	20.7467	140	5	1331
<b>4V2</b>	21	B322	SCMFLONG	20.7467	140	5	1331
級打	27	B292*	E100XM30YM	11	131	33	684
	Story	Beam,	Load Programme	Loc	V2 :	Tø.	M3 :
+M3		B291*	E30X100Y	6	71	-5	684
HV2	43	B291*	E100X30Y	10	116	-3	684
MAT.		B320	E100X5100Y MAX	0	52	11	504

#### Bxx-02

	Story	Beam	Load	Loc	V2	T	M3
+M3	2	B22	MFSSX25	3	497	39	4388
+V2	2	B21	MFSSX25	17.3333	606	39	4344
+Ť	2	B21	MFSSX25	17.3333	606	39	4344

1	Story	Beam	Load	Loc	V2	T	M3
+M3	7	B1	MFSSX25	3	155	3	1369
+V2	7	B2	MFSSX25	17.3333	186	3	1333
+T	14	B1	E30X100Y5 MAX				327

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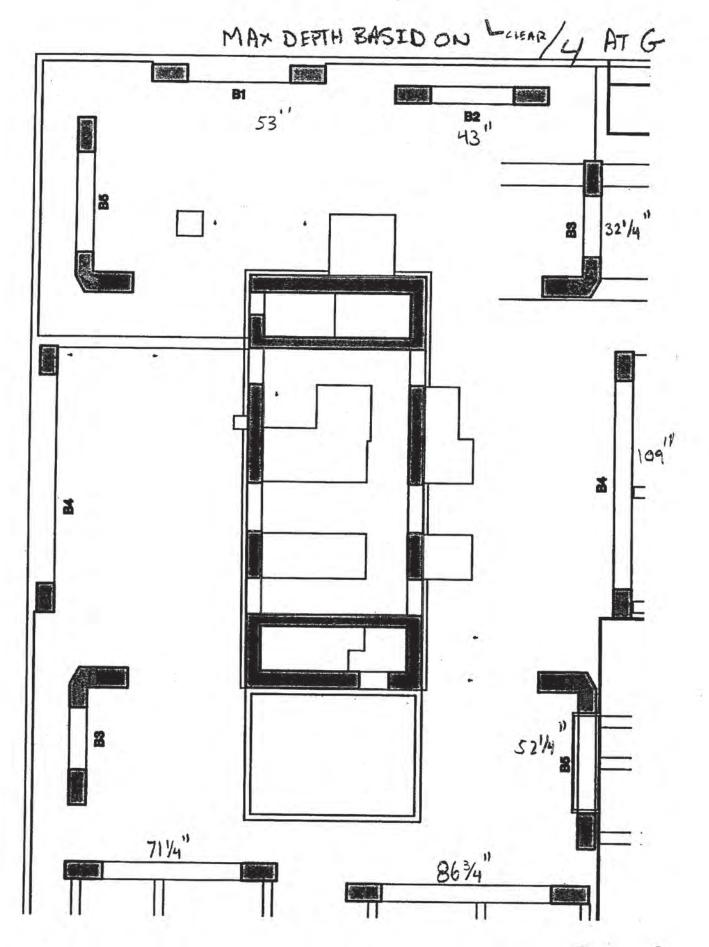
102-01		10-1608		10-518	B21-01	841.01	
							NOW-REVERSILE PH
9997 9997 9997 9997 9997	1600 1600 1600 2600 2600	9000 2000 1000	0002	88	0001 0001 0001 0001	009 009 000 000 000 000 000 000	NOW-REV
End Mu-	-2607	-1114	-941	-608			
Center Nut S7	16	22	22	78			
Mut Mut	Mu+ 2524	Mu+ 1013	Mu+ 840	Mu+ 475			
COMBINED DESIGN! LOND CASES   COMBINED DESIGN! LOND CASES   Municipal Market   Muthor Multipal Muthor Mut	0.9*D 1,4D+1,7L 1,4D+1,7L -31 -58 57	0.9*D 1.4D+1.7L 1.4D+1.7L -38 -71	0.9*D 1.4D+1.7L 1.4D+1.7L	0.9*D 1.4D+1.7L 1.4D+1.7L			
Market Ma	1.42D+6.5L -62	1.42D+0.5L -63	1.42D+0.5L -63	1,42D+0,5L -82			
M3 6 2257 8 2257 27 1460	M3 7 2555 7 2555 39 1454	M3 9 1051 9 1051 20 784	M3 5 878 5 878 20 623	M3 3 524 3 524 12 319			
V2 T 361 361 233	V2 T 411 411 235	169 169 169 127	142 142 142 100	72 T 84 T 52 T			
Loc 14,083	Loc 14.0834 14.0834 14.0834	Loc 14.083	Loc 14.083	20	ŵ		
Load SCMFLONG SCMFLONG E100X5100Y MAX	Load SCMFLONG SCMFLONG E100X5100Y MAX	Losd Loc SCMFLONG SCMFLONG E100X5100Y MAX 14,0834	Load Loc SCAFLONG SCAFLONG E100X5100Y MAX 14.0834	LOBIG SCMFLONG SCMFLONG E100X5100Y MAX			
Story Beam 3 B323 3 B323 3 B321	Story Beam 6 B321 6 B321 11 B321	Story Beam 16 B323 16 B323 20 B321	Story Beam 21 B323 21 B323 25 B321	Slory Beam 45 B323 45 B323 52 B323			
Bxx-01 +W3 +V2 +T	+W3 +T	+W3 +V2 +T	+M3 +V2 +T	+443 +7			

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End Mu- 4440	-1420			 7						
Center Mu+ 57	16									
End C Mu+ 4357	Mu+ 1338									
Mucana 1.4D+1.7L 57	4D+1,7L 57			4						
Mysered no. MUseus 0.9*D 1.4D+1.7L 1.4D+1.7L -31 -58 57	0.9*D 1.4D+1.7L 1.4D+1.7L -31 -58 57									
Myery and mus. N 1.42D+0.5L -52	1.42D+0.5L -52									
Loc V2 T M3 3 496.7 38.76 4388 17.3333 606 38.76 4344 17.3333 606 38.76 4344	Loc V2 T M3 3 154.9 2.52 1369 17.3333 185.9 2.64 1333 3 37.05 10.68 327									
Load MFSSX25 MFSSX25 MFSSX25	Load MFSSX25 MFSSX25 E30X100Y5 MAX									
Story Beam Load 2 B22 MFSS 2 B21 MFSS 2 B21 MFSS	Story Beam Load 7 B1 MFSS 7 B2 MFSS 14 B1 E30X									
8xx-02 +M3 +V2 +T	+M3 +V2 +T									

000) 000)	9000	0001	3000	009 009 009 009 009 009 009 009 009 009
End Mu-	-2309	-1598	-1451	₹ · · · · · · · · · · · · · · · · · · ·
Center Mu+ 448	446	462	794	# Jan X
End C Mu+ 836	Mu+ 1371	Mu+ 639	Mu+ 492	ADD ADD
	0.9*D 1,4D+1,7L 1,4D+1,7L,	0.9*D I.4D+I.7L I.4D+I.7L -357 -688 462	0.9*D I.4D+1.7L 1.4D+1.7L -357 -688 462	NON-R SHENG
Myorveries Myorveries Miles 1,420+0,51 0,5*D 1,40+1,71 -592 -353 689	0.9*D 1.4 -350	0.9°D L4	99°D 1.4	0,9°D 1,4
Myrav end max 1.42D+0.5L -592	1.42D+0.5L -588	1.42D+0.5L -602	1.42D+0.5L -602	1.420+0.5L -636
M3 1189 175	M3 1721 1721 1090	M3 996 996 619	M3 849 539	M3 360 380 208
T 6	994	F 8 8 8	7 5 21	F ∞ ∞ <del>E</del>
3 V2 3 70 3 70 3 46	3 701 3 101 3 64	3 V2 3 59 3 36	3 42 32 32 32 32	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
3	3	9 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	8	. e e e g
SCMFLONG SCMFLONG SCMFLONG E100X5100Y MAX	Load SCMFLONG SCMFLONG E100X5100Y MAX	Load SCMFLONG SCMFLONG E100X5100Y MAX	Load SCMFLONG SCMFLONG E100X5100Y MAX	LONG FLONG (\$100Y MAX
Story Beam 3 B141 3 B141 3 B141	Slory Beam 9 B163 9 B163 10 B163	Story Beam 16 B141 16 B141	Story Beam 21 8141 22 8141 1	Story Beam Load 44 B141 SCMF 44 B141 E100)
+M3 +V2 +T	+W3	+W3	+M3	-W2 -W2

2006	2000	9000	0000	1000 1000
End Nu- -2318	-3011	-4731	-1498	- Mac - 683
Mu+	132	55	133	151
Mu+ 2046	Mu+ 2740	Mu+ 1464	Mu+ 1231	- 584 884
Newwestern Miland Milander 0.9*D 1,4D+1.7L 1,4D+1.7L -102 -188 133	0.9*D 1.4D+1.7L 1.4D+1.7L -102 -167 132	0,9*D 1,4D+1,7L 1,4D+1,7L -100 -186 133	0.9*D 1.4D+1.7L 1.4D+1.7L -100 -186 133	0.9*D 1.4D+1.7L 1.4D+1.7L
1,42D+0,5L	1.42D+0.5L -169	1.42D+0.5L -167	1.42D+0.51, -167	1.42D+0.5L -169
M3 6 2148 6 2148 27 1390	M3 8 2842 8 2842 39 1808	M3 7 1564 7 1564 20 1023	M3 5 1331 5 1331 33 684	684 684 504 504
Loc V2 T 224 224 145	Loc V2 T 20.7467 298 20.7467 298 189	Loc V2 T 164 164 107	Loc V2 T 20,7467 140 20,7467 140 11 131	7 L 1 1 1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
LONG LONG (5100Y MAX	Load SCMFLONG SCMFLONG E100X5100Y MAX	Load SCMFLONG SCMFLONG E100X5100Y MAX	Load SCMFLONG SCMFLONG E100XM30YM	DY MAX
Story Beem Load 3 B320 SCMF 3 B320 SCMF 3 B320 E1000	Story Beam 8 B322 8 B322 9 B320	Story Beam 16 B320 16 B320 20 B320	Story Beam 21 B322 21 B322 27 B292*	Story Besm Load 50 B291* E30X100Y 43 B291* E100X30Y 45 B320 E100X510
+W3 +V2 +T	+ 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	+W3	+ 4 4 + 42 + 42	++++++++++++++++++++++++++++++++++++++



8.2.1-29

Seismic (Y/N) Mu =	y 2,309	k-ft	Mu=	27,708	k-in
Width, b =	24	in			
Total depth, h =	38	in			
Clear Cover =	1.5	in			
Stirrup size # =	5		Stirrup $\phi =$	0.625	in
$f_c =$	10	ksi	$\beta_1 =$	0.65	
$f_y =$	75	ksi	$\rho_{max} =$	2.5%	§1921.3.2
φ=	0.90		$\rho_{min} =$	0.4%	§1921.3.2

	§1921.5.1		Required for 1 layer steel					Required for 2 layer steel				
	MIN COL h							Clear Spacing for				
Bar	(in)	No.	As	for 1 row	p limits check	No.	As	2 rows	p limits check			
3	9.4	111.6	12.28	> smin NG	ОК	114.15	12.56	> smin NG	OK			
4	12.5	61.5	12.30	> smin NG	OK	63.04	12.61	> smin NG	OK			
5	15.6	39.8	12.33	> smin NG	OK	40.84	12.66	> smin NG	ок			
6	18.8	28.1	12.35	> smin NG	ОК	28.89	12.71	> smin NG	ОК			
7	21.9	20.6	12.38	> smin NG	ок	21.28	12.77	1.08	OK			
8	25.0	15.7	12.40	> smin NG	ок	16.23	12.82	1.64	OK			
9	28.2	12.4	12.43	> smin NG	ок	12.88	12.88	2.30	ок			
10	31.8	9,8	12.46	> smin NG	ок	10.19	12.94	3.24	ОК			
11	35.3	8.0	12.49	> smin NG	ок	8.33	13.00	4.38	ОК			
14	42.3	5.6	12.54	2.25	ок	5.83	13.13	7.73	OK			
18	56.4	3.2	12.66	5.82	OK	3.35	13.39	23.71	OK			

Bar size #=	10	d =	34.11	
No. of bars =	12.00	a =	5.60	
No. of rows =	2	φ Mn =	32202	k-in
Diameter =	1.270		2683	k-ft
As total =	15.24			
ρ	1.86%	$Mu/\phi Mn =$	0.86	should be less than 1.0 to be OK
		Clear Spacing	2.43	in
		p limits check	OK	

$$\begin{array}{ccc} \text{fy for overstrength} = & 1.25 \\ & a = & 7.00 \\ & \text{Mpr=} & 43724 & \text{k-in} \\ & & 3644 & \text{k-ft} \end{array}$$

Seismic (Y/N)	У				
Mu =	4,400	k-ft	Mu =	52,800	k-in
Width, $b =$	36	in			
Total depth, h =	42	in			
Clear Cover =	1.5	in			
Stirrup size #=	5		Stirrup $\phi =$	0.625	in
f'c=	10	ksi	$\beta_1 =$	0.65	
$f_y =$	75	ksi	$\rho_{\text{max}} =$	2.5%	§1921.3.2
φ=	0.90		$\rho_{\min} =$	0.4%	§1921.3.2

	§1921.5.1		Required for 1 layer steel					Required for 2 layer steel					
	MIN COL h			ar at the		3.75	Clear Spacing for						
Bar	(in)	No.	As	for 1 row	ρ limits check	No.	As	2 rows	p limits check				
3	9.4	191.7	21.08	> smin NG	OK	195.55	21.51	> smin NG	OK				
4	12.5	105.6	21.12	> smin NG	ок	107.96	21.59	> smin NG	OK				
5	15.6	68.3	21.16	> smin NG	ок	69.91	21.67	> smin NG	OK				
6	18.8	48.2	21.20	> smin NG	ОК	49.44	21.75	> smin NG	OK				
7	21.9	35.4	21.24	> smin NG	ок	36.39	21.84	> smin NG	OK				
8	25.0	26.9	21.27	> smin NG	ок	27.74	21.92	1.39	OK				
9	28.2	21.3	21.31	> smin NG	ок	22.00	22.00	1.93	OK				
10	31.8	16.8	21.36	> smin NG	ок	17.40	22.10	2.69	OK				
11	35.3	13.7	21.40	> smin NG	ок	14.23	22.19	3.55	OK				
14	42.3	9.6	21.49	1.82	ок	9.95	22,39	5.87	OK				
18	56.4	5.4	21.67	4.42	ок	5.70	22.79	13.70	OK				

Bar size #=	11	d=	37.97	
No. of bars =	16.00	a =	6.12	
No. of rows =	2	$\phi Mn =$	58810	k-in
Diameter =	1.410	3.72	4901	k-ft
As total =	24.96			
ρ	1.83%	$Mu/\phi Mn =$	0.90	should be less than 1.0 to be OK.
		Clear Spacing	2.92	in
		p limits check	OK	

Seismic (Y/N) Mu =	y 1,781	k-ft	Mu=	21,372	k-in
Width, b =	24	in		21,0,12	х ш
Total depth, h =	38	in			
Clear Cover =	1.5	in			
Stirrup size # =	5		Stirrup $\phi =$	0.625	in
f'c =	10	ksi	$\beta_1 =$	0.65	
$f_y =$	75	ksi	$\rho_{\text{max}} =$	2.5%	§1921.3.2
φ=	0.90		$\rho_{min} =$	0.4%	§1921.3.2

	§1921.5.1	1.5.1 Required for 1 layer steel					Required for 2 layer steel				
Bar	MIN COL h (in)	and the same of				Clear Spacing for					
		No.	As	for 1 row	p limits check	No.	As	2 rows	p limits check		
3	9.4	84.7	9.32	> smin NG	ок	86.57	9.52	> smin NG	OK		
4	12.5	46.7	9.34	> smin NG	OK	47.80	9.56	> smin NG	OK		
5	15.6	30.2	9.36	> smin NG	OK	30.96	9.60	> smin NG	OK		
6	18.8	21.3	9.37	> smin NG	ок	21.90	9.64	1.16	OK		
7	21.9	15.7	9.39	> smin NG	ок	16.13	9.68	1.80	ok		
8	25.0	11.9	9.41	> smin NG	ок	12.30	9.72	2.64	OK		
9	28.2	9.4	9.43	> smin NG	ок	9.76	9.76	3.67	OK		
10	31.8	7.4	9.45	1.60	OK	7.72	9.80	5.19	ok		
11	35.3	6.1	9.47	2.21	ок	6.31	9.85	7.10	OK		
14	42,3	4.2	9.51	3.90	OK	4.42	9.94	13.25			
18	56.4	2.4	9.60	10.24	.ok	2.53	10.13	63.44	OK OK		

Bar size #=	10	d=	34.11	
No. of bars =	12.00	a =	5.60	
No. of rows =	2	♦ Mn =	32202	k-in
Diameter =	1.270	17.	2683	k-ft
As total =	15.24		0.13-6	
P	1.86%	$Mu/\phi Mn =$	0.66	should be less than 1.0 to be OK
		Clear Spacing	2.43	in
		p limits check	OK	
fy for overstrength =	1.25			

У				
2,318	k-ft	Mu =	27,816	k-in
24	în			
38	in			
1.5	in			
5		Stirrup $\phi =$	0.625	in
10	ksi	$\beta_1 =$	0.65	
75	ksi	$\rho_{\text{max}} =$	2.5%	§1921.3.2
0.90		$\rho_{\min} =$	0.4%	§1921.3.2
	2,318 24 38 1.5 5 10 75	2,318 k-ft 24 in 38 in 1.5 in 5 10 ksi 75 ksi		2,318 k-ft Mu = 27,816 24 in 38 in 1.5 in 5 Stirrup $\phi$ = 0.625 10 ksi $\beta_1$ = 0.65 75 ksi $\rho_{max}$ = 2.5%

	§1921.5.1		Requir	ed for 1 layer ste	eel		Requir	ed for 2 layer	steel
	MIN COL h	100		Clear Spacing	THE PARTY			Clear Spacing for	
Bar	(in)	No.	As	for 1 row	ρ limits check	No.	As	2 rows	p limits check
3	9.4	112.1	12.33	> smin NG	ок	114.63	12.61	> smin NG	OK
4	12.5	61.8	12.36	> smin NG	OK	63.31	12.66	> smin NG	OK
5	15.6	39.9	12.38	> smin NG	ок	41.01	12.71	> smin NG	OK
6	18.8	28.2	12.40	> smin NG	ок	29.02	12.77	> smin NG	OK
7	21.9	20.7	12.43	> smin NG	ок	21.37	12.82	1.07	OK
8	25.0	15.8	12.46	> smin NG	ок	16,30	12.87	1.62	OK
9	28.2	12.5	12.48	> smin NG	ок	12.93	12.93	2.28	OK
10	31.8	9.9	12.51	> smin NG	ок	10.23	12.99	3.22	OK
11	35.3	8.0	12.54	> smin NG	ок	8.37	13.06	4.35	OK
14	42.3	5.6	12.60	2.23	ok	5.86	13.18	7.66	OK
18	56.4	3.2	12.72	5.77	ок	3.36	13.45	23.44	OK

Bar size #=	10	d=	34.11	
No. of bars =	16.00	a =	7.47	
No. of rows =	2	φ Mn =	41655	k-in
Diameter =	1.270		3471	k-ft
As total =	20.32		36.00	2
ρ	2.48%	$Mu/\phi Mn =$	0.67	should be less than 1.0 to be OK.
		Clear Spacing	1.37	in
		p limits check	OK	
fy for overstrength =	1.25			

Shear Rein

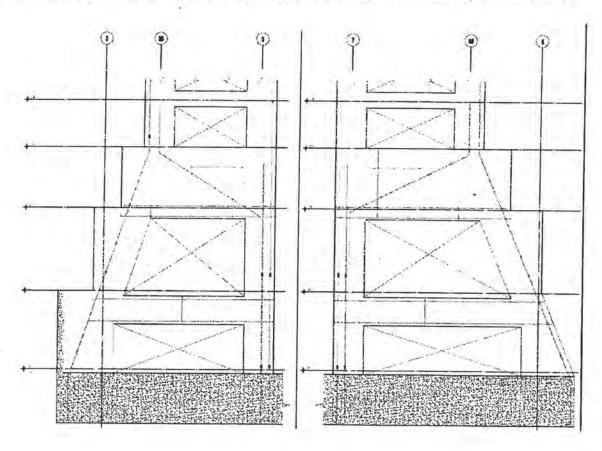
	_	100		1			Min L in	9	Ve	Worse Case		Max spacing		Bar				inside
	revei	DIMO	ď	0	o -	Mpr	group	(ZMpr/L)	(analysis)	1.2*VpL+0.5*VLL	?	in PH	Sactual	Size	As req	legs	u/\	PH IN
	5	K-ff	.⊑	'n	ksi	k-ft	#	kips	kips	kips	Ī	ij	Ë	in^2	in <sup>2</sup>		kips	
Bxx-01	12-13		24	34.11	10	3644	10.83	673	361	15	688	8.5	4	0.31	1,315	9	809	0.8
	L4-L14		24	34.11	10	3644	10.83	673	411	15	688		4	_	1315		808	0.85
	L15-L20		24	30.38	10	1696	12.5	271	169	15	286	7.6	9	-	0.893		320	0.89
	L21-L40		24	26.38	8	1434	12.5	229	142	15	244			_	0.870	4	278	0.88
	L41-Roof		24	25.38	7	937	14.83	126	84	15	141	6.3	9		0.498	4	173	0.82
00			1	1	-													
DXX-03	12-13	1	24	34.11	9	3644	34	214	70	62	276	8.5	9	0.31	0.628	4	360	0.77
	L4-L14		24	34.11	9	3644	34	214	101	62	276	8.5	9	0.31	0.628	4	360	0.77
	L15-L20		24	30.25	10	2577	35.67	144	69	62	206	7.6	9	9	0.478	4	319	0.65
	L21-L40		24	26.25	80	2138	35.67	120	20	62	182	9.9	9		0.457	4	277	0.66
	L41-Roof		24	24.38	7	1484	38	78	21	62	140	6.1	9	1	0.320	4	166	0.85
3			1										I					-
BXX-04	12-13		24	34.11	10	4673	20.5	456	224	22	478	8.5	9	0.31	1.337	9	539	0.89
	L4-L14		24	34.11	10	4673	20.5	456	298	22	478	8.5	9	0.31	1.337	9	539	0.89
	L15-L20		24	30.25	10	2577	21.3	242	164	22	264	7.6	9	0.31	0.800	4	319	0.83
	121-140		24	26.25	80	2138	21.3	201	140	22	223	6.6	9	0.31	0,765	4	277	0.81
	L41-Roof		24	24.38	7	1297	22.5	115	116	22	138	6.1	9	0.20	0.473	4	186	0.83

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As red long AVA DH INT	1080	7"	SON		1.461 61 1029 0 941	
Bar Size A		-	7.111	200 20	3.5 0.31	25 0 34
Max spacing in PH	D			130	0.6	75
3	I			088	3	996
Worse Case		KIDS		15	2	15
Ve (analysis)	-	KIDS		909		186
Ve (2Mpr/L)	100	Sdix		951	120	108
Min L in group	4	ш		14	1	14
Mpr	41	N-II	0200	8000	0000	0000
f <sub>c</sub>	Loi	VS	10,	2	40	101
Ф			0.40	18.70	2000	29.9
ď	.5	100	100	200	VC	4.7
φMn	44	MAIL			-	
Level			1010	F4-L3	14-14	21.7
			Rvv-03	סעע-חק		

#### 8.2.2 Slope Column Design

There are two SMRF columns on the south side of the tower that slope starting at level 3 as shown below. Each frame is in different physical planes (see plan.) This slope was required in order to provide for the code required Porte Cochere vehicle clearances. The columns make angles of 18 and 22 degrees with the vertical. A direct load path to the ground is provided for the axial force by uninterrupted vertical rebar. A strut and tie load path is provided for the horizontal load that will be induced at level 3 where the column starts to slope. Each frame is designed to resolve their own forces in their respective planes without any force transfer to the diaphragms. The columns starting at level 2 have been designed to resist the additional moments that will be induced. Design loads are based on the capacities of the columns directly above level 3.



# **DESIMONE**

Project		Page Of	
Project No.		Date 5/21/05	
Item	MF SLOPING COLUMN	BY NIR Child	

THE CAPACITY OF COLD = 
$$\begin{cases} 0 & P_E = 2500 \text{k(c)} 1.25 \text{mn} = 30,500 \text{ k} \cdot \text{P} + \frac{1}{2} \text{mn} = \frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2} \text{mn} = \frac{1}{2}$$

USING THE BEHAVIOR PORCES, I CAN BETERMINE THE DESIGN FORCES.

$$M_{U} = (1.2 \text{ k-ft})(2000 \text{ sites}) + (1.2 \text{ k-ft})(30,500 \text{ sites})$$

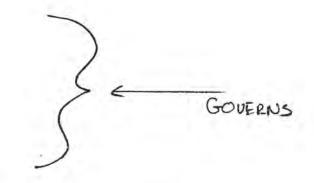
$$= 2000 \text{ k ft} + 3050$$

$$= 6050 \text{ k -ft}$$

$$P_{E} = (0.6 \text{ k})(2500) + (0.6 \text{ k})(\frac{30,500}{12})$$

$$= 3025 \text{ kips}$$

② 
$$M_0 = (1.2)(10,000)$$
  
= 12,000 k-ft



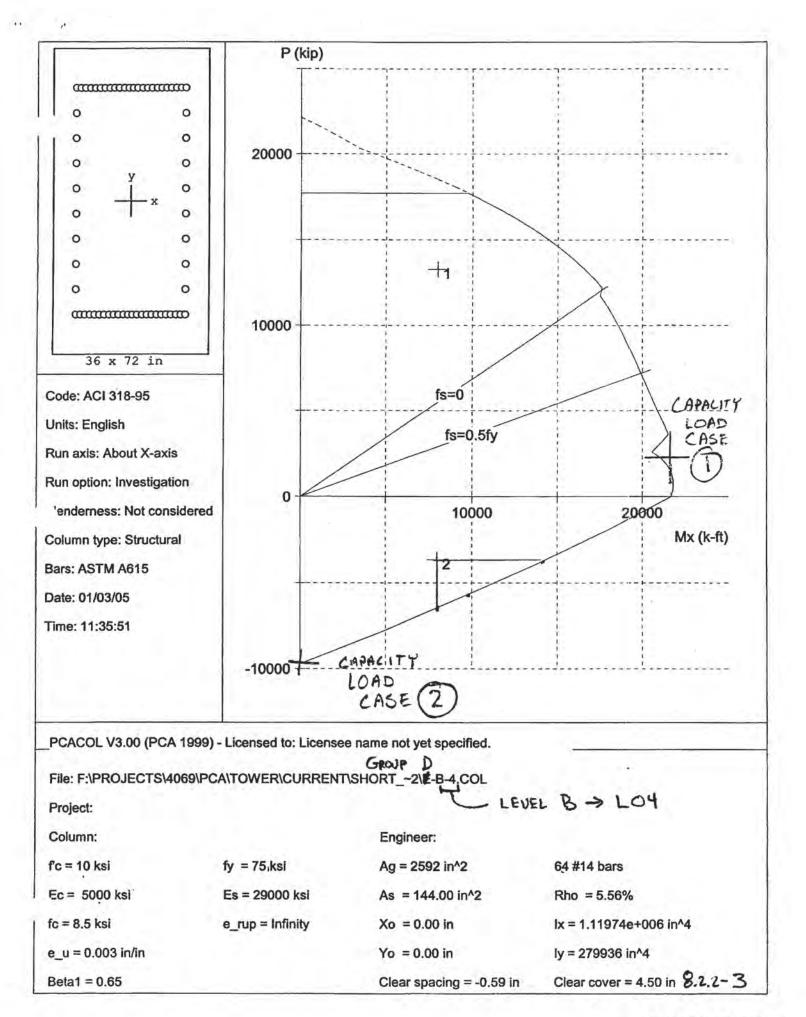
GRAV @ LEVEL ?

1.42 01 + 0.51 = 6203 k

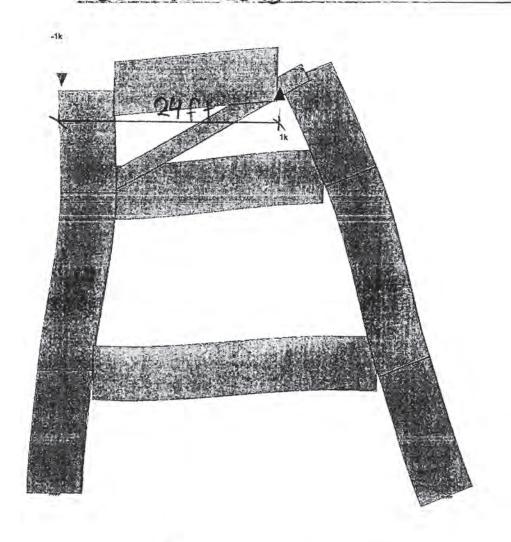
0.9 01 = 2902 k

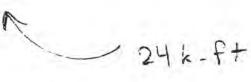
# MAX LOADS FROM ETASS ANALYSIS

V	Story	Brace	Load	Loc	P	V2	M3
Max P	GROUND	D102	MFSSX25	0	4244	195	1715
Max V2	3	D106	MFSSX25	13	3520	479	5931
Max M3	3	D106	MFSSX25	13	3520	479	5931



# BEHAVIOR STUDY FOR UNIT

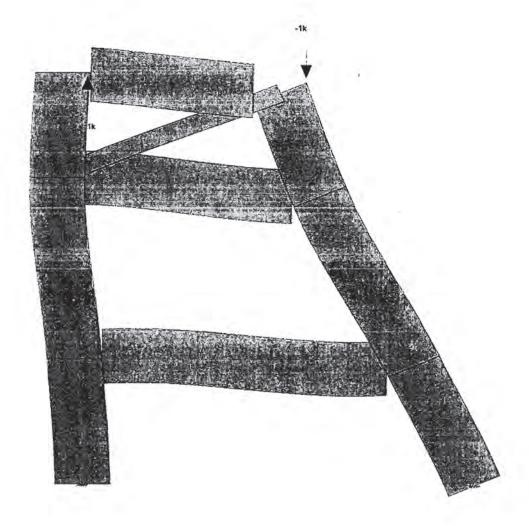




Loads: BLC 1, test Results for LC 1, test

DeSimone Consulting Eng...

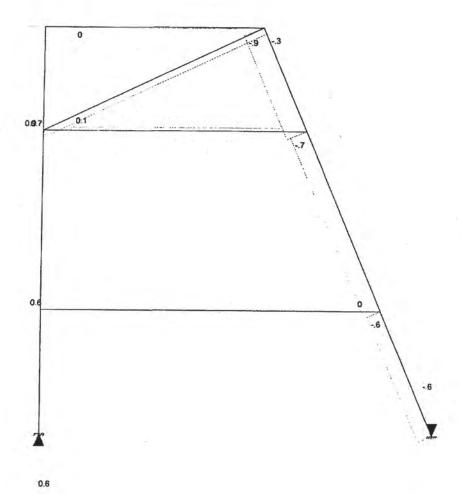
May 21, 2005 at 8:27 PM 20050504.R3D



Loads: LC 1, test Results for LC 1, test

DeSimone Consulting Eng...

May 21, 2005 at 8:28 PM 20050504.R3D



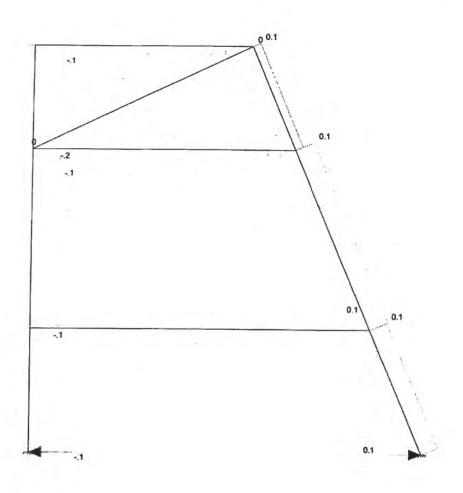
Results for LC 1, test Member Axial Forces (k) Reaction units are k and k-ft

DeSimone Consulting Eng...

May 21, 2005 at 8:27 PM

20050504.R3D

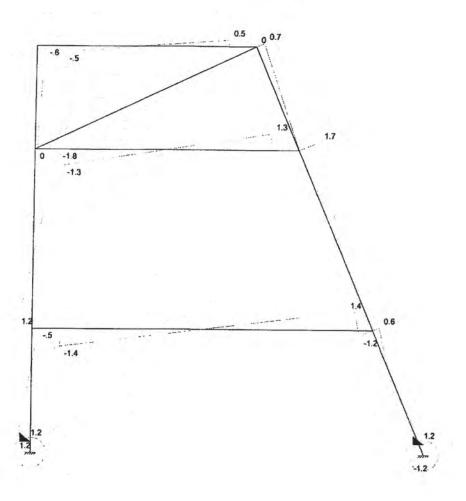
.Y



Results for LC 1, test Member y Shear Forces (k) Reaction units are k and k-ft

DeSimone Consulting Eng...

May 21, 2005 at 8:26 PM 20050504.R3D

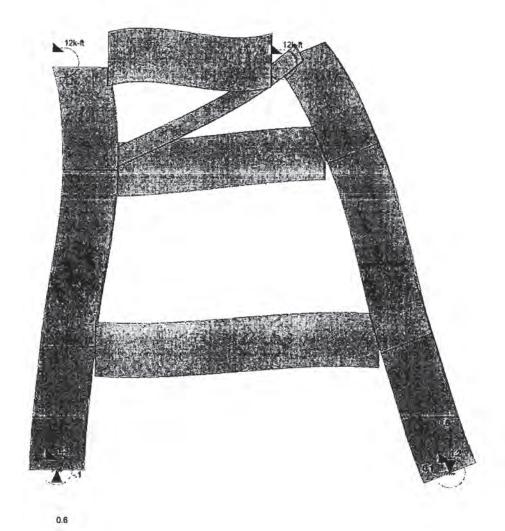


Results for LC 1, test Member z Bending Moments (k-ft) Reaction units are k and k-ft

DeSimone Consulting Eng...

May 21, 2005 at 8:26 PM 20050504.R3D



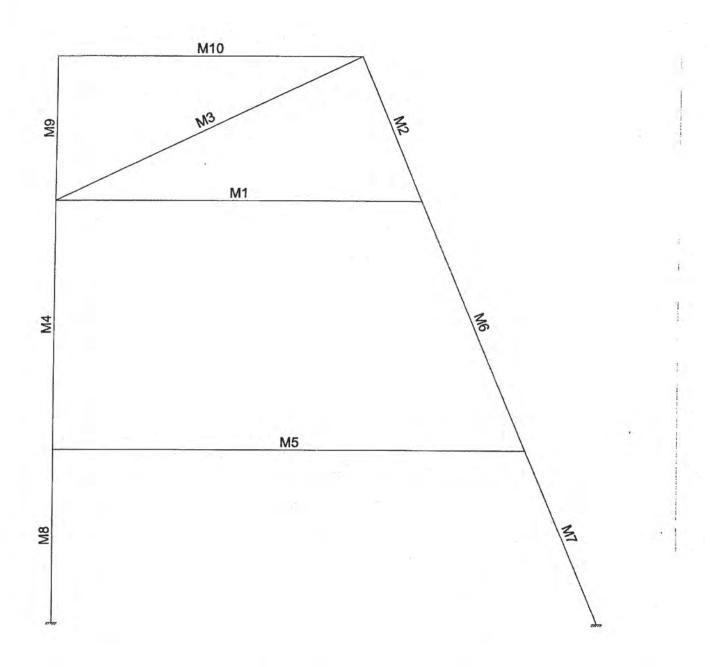


Loads: LC 2, Unit Moment Results for LC 2, Unit Moment Reaction units are k and k-ft

DeSimone Consulting Eng...

May 21, 2005 at 8:46 PM 20050504.R3D





Results for LC 4, 0.9DL-LC2

DeSimone Consulting Eng...

May 22, 2005 at 9:40 AM
20050504.R3D

**DeSimone Consulting Engineers** 

Company : Designer : Job Number :

May 22, 2005 9:38 AM Checked By:\_

Member Section Forces (By Combination)

	LC	Member Label	Sec	Axial[k]	y Shear[k]	z Shear[k]	Torque[k-ft]	y-y Moment	z-z Moment
1	1	M2	11 -	1363.335	511.937	0	0	0	8375.353
2			2	1363.335	511.937	0	0	0	6774.07
3			3	1363.335	511.937	0	0	0	5172.79
4	1300-321	MACANTHIAN STAN	4	1363.335	511.937	0	ŏ	Ö	3571.51
5			5	1363.335	511.937	0	0	0	1970.229
6	1	M4	4.7	11558.161	195.781	ŏ	0	Control Control	1494.762
7		The second second	2	11558.161	195.781	0			
8	-	85 388 E80 C	3	11558.161		0	0	0	522.219
9			4	11558.161	195.781	0		0	-450,325
10		T 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	5	11558.161			0	0	-1422.86
11	4	M6	1	857.854			0	0	-2395.41
12	T	IVIO	2		-145.769	0	0	0	-1094.44
				857.854	-145.769	0	0	0	-309.838
13	-	and the state of	3	857.854	-145.769	0	0	0	474.766
14	- A		4	857.854	-145.769	0	0	0	1259.369
15	4		5	857.854	-145.769	0	0	0	2043.973
16	1 :	M7	1	743.52	-155.507	0	0	0	181.65
17		· · · · · · · · · · · · · · · · · · ·	2	743.52	-155.507	0 1	0	0	761.632
8	200		3	743.52	-155.507	0	0	0	1341.615
9			4	743.52	-155.507	0	0	0	1921.597
20	3		5	743.52	-155.507	0	0	0	2501.58
21	1	M8	1	11659.886	142.539	0	0	0	-514.254
2			2	11659.886	142.539	0	0	0	-1004.946
3			3	11659.886	142.539	0	0	0	-1495.637
4	(300)		4	11659.886	142.539	0	0	0	-1986.329
5	5 5 5		5 0	11659.886	142.539	0	0	0	-2477.02
6	1	M9	13	10523.647	620.813	0	0	0	-1834.488
7				10523,647	620.813	Ö	0	0	-3627.084
8			3	10523.647	620.813	Ŏ	0	0	-5419.681
9				10523.647	620.813	0	0	0	
0		10 To 10 To	5	10523.647	620.813	0	0	0	-7212.277
1	2	M2	1	-3178.789	276.904	0	0	0	-9004 873
2	-	IVIZ	2	-3178.789	276.904	0	Ö		4646.271
3		-	3	-3178.789	276.904			0	3780.145
4	y	- 1 t	4			0	0	0	2914.02
5				-3178.789	276.904	0	0	0	2047.895
	2	MA	5	-3178.789	276.904	0	0	0	1181.769
6	2	M4		15027.661	-571.363	0	0	0	-8010.231
7	-		2	15027.661	-571.363	0	0	0	-5171.985
8	-	- L	3	15027.661	-571.363	0	U	0	-2333.738
9	-		4	15027.661	-571.363	0	0 i	0	504.508
0	-		5	15027.661	-571.363	0 1	0	0	3342.754
1	2	M6	1	-2639.526	482.145	0	0	0	7235.118
2	_1_		2	-2639.526	482.145	0	0	0	4639.972
3	Ð		3	-2639.526	482.145	0	0	0	2044.826
4				-2639.526		0 1	0	0	-550.32
5	. 1		5	-2639.526	482.145	0	0	0	-3145.465
6	2	M7	1	-2345.038	463.514	0	0	0	2002.392
7		1	2	-2345.038	463.514	0	0	0	273.665
8				-2345.038	463.514	0	0	Ō	-1455.062
9		-	4	-2345.038	463,514	0	0	0	-3183.789
)	13	and the second second		-2345.038		Ŏ	0	0	-4912.517
1	2	M8		14748.851		0	0	0	
2	-	1110		14748.851		0 1	0		-1769.576
3	-1-							0	-136.312
4	1	- 1		14748.851		0	0	0	1496.953
5		36.4			-474.441 -474.441	0	0	0	3130.218 4763.482

RISA-3D Version 5.0b

[F:\Projects\4069\Risa\transfer girder\20050504.R3D]

Company : DeSimone Designer : Job Number :

**DeSimone Consulting Engineers** 

May 22, 2005 9:38 AM Checked By:\_

Member Section Forces (By Combination) (Continued)

57	LC	Member Label	Sec 2	Axial[k]: 15825.129	y Shear[k] 209.942	z Shear[k]	Torque[k-ft]	y-y Moment[.	z-z Moment[
58			3	15825.129	209.942	0.00	Ö	Ö	-3165.325
59			4	15825.129	209.942	0	0	0	-3771.533
60	100	Service Line had	5	15825.129		ŏ	Ŏ	Ö	-4377.741
61	3	M2	1		-1259.706	0	0	0	
62	77. 79.		2	FEET 1	-1259.706	Ö	0	0	-19753.372
63	1		3		-1259.706	0	0		-15813.156
64	100	N. C. A. C. C. C. C. C. C. C. C. C. C. C. C. C.	4	9572.159	and the same of th	0		0	-11872.94
65	-	7.77			-1259.706		0	0	-7932.724
66	3	AM.	5	9572.159	-1259.706	0	0	0	-3992.508
	3	M4		4206.331	1778.995	0	0	0	22266.506
67	1	Carried SA T of	2	4206.331	1778.995	0	0	0	13429.346
68	2		3	4206.331	1778.995	0	0	0	4592.186
69			4	4206,331	1778.995	0	0	0	-4244.974
70			5	4206.331		0	0	0	-13082.134
71	3	M6	11	8252.452	-1515.402	0	0	0	-20222.019
72	100	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	2		-1515.402	0	0	0	-12065.364
73			3	8252.452	-1515.402	0	0	0	-3908.71
74			4	8252,452	-1515.402	0	0	0	4247.945
75			5		-1515.402	0	0	0	12404.6
76	3	M7	1		-1444.222	0	0	0	-3701.259
77			2		-1444.222	0	0	0	1685.135
8			3		-1444.222	0	Ö	Ö	7071.528
9			4		-1444.222	0	0	0	
10			5		-1444.222	0			12457.922
1	3	M8	1				0	0	17844.315
2	3	WO		5078.067	1489.961	0	0	0	2891.907
			2	5078.067	1489.961	0	0	0	-2237.283
3			3		1489.961	0	0	0	-7366.473
4			4		1489.961	0	0	0	-12495.664
5			5		1489.961	0	0	0	-17624.854
6	. 3	M9	State of the state of		-1128.006	0	0	0	6239.946
7			2	2399.074	-1128.006	0	0	. 0	9497.064
8	-	L VALUE S	3	2399.074	-1128.006	0	0	0	12754.181
9			4	2399.074	-1128.006	0	0	0	16011.298
0			5		1128.006	0	0	0	19268.415
1	4	M2	1	13842.671	-923.576	0	0		-14419.243
2			. 2	13842.671	-923,576	0	Ŏ		-11530.403
3	1		-63	13842.671	-923.576	0	0	0	-8641.562
4		M4 170	4/1/40		-923.576	0	0		
5		1	5		-923,576	0		0	-5752,721
6	4	M4 800	1		2489.949		0	0	-2863.88
7	4	1V14 1262	1 2 1			0	0		31014.708
	14- 1		1 3 4	1001.002	2489.949	0	0	0	18645.886
3			1 0	1001.002		0	0		6277.063
9			4	1001.002		0	0	0	-6091.76
0	1		5	1001.002		0	0	0	-18460.582
1	4	M6	1	11484.392 -		0	0	0	-27839.903
2	131		2	11484.392	2093.457	0	0	0	-16571.868
3			3	11484.392 -	2093.457	0	0		-5303.833
4	1		4	11484.392	2093,457	0	0	0	5964.203
5			5	11484.392		0	0		17232.238
6	4	M7	ho i au	10187.909 -		0	0		-5382.069
7			2	10187.909 -		0	0	0	2141.741
8	(1,11)	1,7 1,1	3	10187.909 -	2017 315	0	0	0	
9			4	10187.909 -		0	0		9665.55
0	1		5	10187.909 -					17189.359
1	4	M8				0 1	0		24713.169
	4	IVIO	1	2226,204		0	0		4012.529
2			2	2226.204		0	0		-3071.669
3			3	2226.204	2057.864	0	0	_ 0 -	10155.867

RISA-3D Version 5.0b

[F:\Projects\4069\Risa\transfer girder\20050504.R3D]

: DeSimone Consulting Engineers

May 22, 2005 9:38 AM Checked By:\_

Company : Designer : Job Number :

Member Section Forces (By Combination) (Continued)

	LC	Member Label	Sec	Axial[k]	y Shear[k]	z Shear[k]	Torque[k-ft]	v-v Momentf.	.z-z Momentí
114			4	2226.204	2057.864	0	0	0	-17240.066
115			5	2226.204	2057.864	0	0	0	-24324.264
116	4	M9	1. * * *	-2661.753	-611.089	0	0	0	5939.187
117			2	-2661.753	-611.089	0	. 0	0	7703.708
118			3	-2661.753	-611.089	0	0	0	9468,228
119			4	-2661.753	-611.089	0	0	0	11232.749
120	Be A	and a street H	5	-2661.753	-611.089	0	0	0	12997.269

Member Section Forces (By Combination)

1	LC 1	Member Label M1	Sec 1	Axial[k]	y Shear[k] 213.908	z Shear[k]	Torque[k-ft]	y-y Moment[	z-z Moment[k-ft] 2392.13
2	100	340 M SAUCE	2			0	0	0	
3			3	-800.94	213.908	0			1100,001
4	100	12 1 10 - 2 V m 1 -	4	-800.94		Ö	0	0	-15.408
5	1		5	-800.94	213.908	0		0	-1219.177
6	1	M5	- ř	-53.242		The second secon	0	0 1	-2422.946
7	-	IVIO	2		-101,725	0		1 0	-1557.148
8	$\vdash$	1	3	-53.242	-101.725	0	0	0	-773.865
9	1			-53.242	-101.725	0	0	0	9.417
	1 1		4	-53.242	-101.725	0	0	0	792.7
10	-	1440	5	-53.242	-101.725	- 0	0	0	1575.982
11	1	M10	1	-620.813	1820.647	0	0	0	16033.184
12			2	-620,813	1820.647	0		0	7976.819
13	1		3	-620.813	1820.647	0	0	0	-79.546
14			4	-620.813	1820.647	0	0	0	-8135.911
15	Ī		5	-620.813	1820.647	0 !	0	0	-16192.276
16	2	M1	1	395.083	-419.286	0	0	0	-4642.646
17			2	395.083	-419.286	0	0	0	-2283,112
18			3	395.083	-419.286	0 1	0	0	76.422
19			4	395.083	-419.286	0 ;	0	0	2435.956
20	1	The state of the s	5	395.083	-419.286		0	0	4795.49
21	2	M5	1	96.922	278.809	0	0	0	4311.429
22	- 0	- 1 - VA	2	96.922	278.809	0	Ö	0	2164.597
23			3	96.922	278.809	0	Ö	0	17.764
24	14		4	96.922	278.809	0	Ö	o l	
25			5	96.922	278.809	0	0	-	-2129.069
26	2	M10	1	-209.942	-377.871	0		0	-4275.902
7	-	IVIIO	2	-209.942			0	0	-3244.128
8	-		3	-209.942	-377.871	0	0	0	-1572.048
29	-			-209.942	-377.871	0	0	. 0	100.032
30	-		5	-209.942	-377.871	0	0	0	1772.112
	3	144	- 5		-377.871	0	0	0	3444.192
31	3	M1	1	-738.079	1121.136	0	0	0	12370.671
32			2	-738.079	1121.136	0	0	0	6061,478
33	- 1		3	-738.079	1121.136	0	0	0	-247.716
14	-		4	-738.079	1121.136	0	0	0	-6556.909
5	-		5	-738.079	1121.136	0	0	0 1	-12866.103
	3	M5	1	-289.035	-871.736	0	. 0	0	-13490.65
7	- 1		2	-289.035	-871.736	0	0	0	-6778.279
8			3	-289.035	-871.736	0	0	0	-65.909
9	1		4	-289.035	-871.736	0	0	0	6646.462
0	90 3		5	i-289.035	-871.736	0	0	0 0 0	13358.832
1	3	M10	1		-1303.926	0	0	0	-11819.807
2		1000	2		-1303.926	0	o !	Ö	-6049.935
3		3	3		-1303.926	0	0	0	-280.062
4			4	10 40 00	-1303.926	0	o i	o I	5489.81
5			5	-	-1303.926	0	0	0	
	4	M1	1		1728.898	0	0	0	11259.683
7	-		2		1728.898	0			19128.176
В	-1	1-1-1	3		1728.898	0	0	0	9398.8
9		· · · · · · · · · · · · · · · · · · ·					0 1	0	-330.576
	1		4		1728.898	0	0	0	-10059.952
0 !	4	ME	5		1728.898	0 !	0	0	-19789.328
	4	M5	1		-1225,202	0	0	0	-18938.702
2		- L	2		1225.202	0	0	0	-9504.65
3			3		1225.202	0	0	0 .	-70.598
4	i		4	-432.085	1225.202	0	0	0	9363.454
5			5		1225.202	0	0	0	18797.506
	4	M10	1	611.089		0	0	0	9591.529

**DeSimone Consulting Engineers** 

Company : Designer : Job Number :

May 22, 2005 9:39 AM Checked By:\_

Member Section Forces (By Combination) (Continued)

	LC	Member Label	Sec	Axial[k]	y Shear[k]	z Shear[k]	Torque[k-ft]	y-y Moment[	z-z Moment[k-ft]
57			2	611.089	1135.247	0	0	0	4568.063
58			3	611.089	1135.247	0	. 0	0	-455,404
59			4	611.089	1135.247	0	0	0	-5478.871
60	150		5	611.089	1135.247	0	0	0	-10502.337

Company : Designer : Job Number :

: DeSimone Consulting Engineers

May 22, 2005 9:39 AM Checked By:\_

Member Section Forces (By Combination)

	LC	Member Label	Sec	Axial[k]	y Shear[k]	z Shear[k]	Torque[k-ft]	y-y Moment[	z-z Moment[k-ft]
1	11	M3	1	1813.55	-29.047	0	0	0	-470.429
2			2	1813.55	-29.047	0	0	0	-278.973
3			3	1813.55	-29.047	0	0	0	-87.517
4	art d		4	1813.55	-29.047	0	0	0	103.939
5	1		5	1813.55	-29.047	0	0	0	295.395
6	2	M3	1.	-845.723	8.545	0	0	0	68.465
7	1	1	2	-845.723	8.545	0	0	0	12.145
8	100		3	-845.723	8.545	0	0	0	-44.175
9		7	4	-845.723	8.545	0	0	0	-100.496
10		7.7	5	-845.723	8.545	0	0	0	-156.816
11	3	M3	_ 1	1549.259	-8.243	0	0	0	75.167
12			2	1549.259	-8.243	0	0	0	129.496
13			3	1549.259	-8.243	0	0	0	183.824
14			4	1549.259	-8.243	0	0	0	238.152
15		- 1	5	1549.259	-8.243	0	0	0	292.48
16	4	M3	_ 1	4315.334	-48.239	0	0	0	-511.166
17			2	4315.334	-48.239	0	0	0	-193,212
18	70	201	3	4315.334	-48.239	0	0	0	124,742
19			4	4315.334	-48.239	0	0	0	442.696
20			5	4315.334	-48.239	0	0	0	760.65

Company	
Company	
Decignor	
Designer	
Job Number	
JOD LAGITIDE	

**DeSimone Consulting Engineers** 

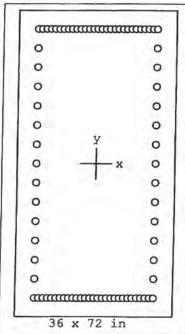
May 22, 2005 10:00 AM Checked By:\_

**Envelope Joint Reactions** 

	Joint		X [k]	lc	Y [k]	lc	Z[k]	lc	MX [k-ft]	lc	MY [k-ft]	Ic	MZ [k-ft]	lc
1	N6	max	2058	4	14749	2	0	1	0	1	0	1	4763	12
2		min	-474	2	2226	4	0	1	0	1	0	1	-24324	4
3	N4	max	474	2	10180	4	0	1	0	1	0	1	4913	2
4	and the state of	min	(-2058)	4	-2343	2	0	1	0	1	0	1	-24713	4
5	Totals:	max	-0	4	12406	3	0	1 1						1
6		min	10	2	12406	1	0	1	0.	0.15	190	deg .	- S. F. C.	

$$As_{REQ} = \frac{2058 \text{ kips}}{(0.75) 75 \text{ ksi}}$$

PROVIDE 20-#14



Code: ACI 318-95

Units: English

Run axis: About X-axis

Run option: Investigation

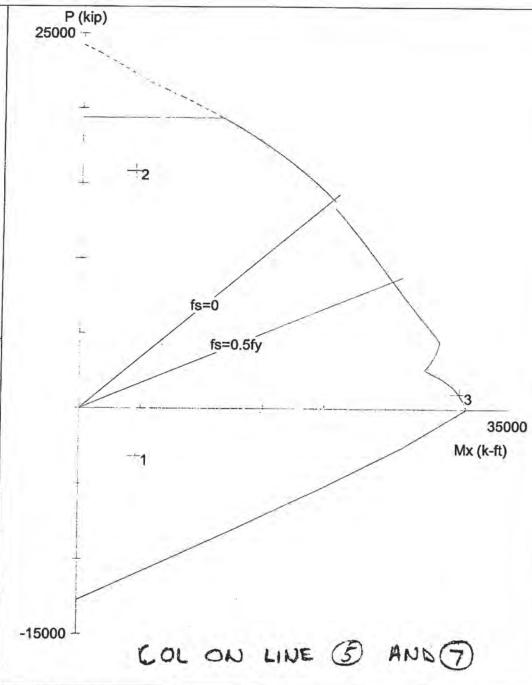
Slenderness: Not considered

Column type: Structural

Bars: ASTM A615

Date: 05/22/05

Time: 09:57:27



## PCACOL V3.00 (PCA 1999) - Licensed to: Licensee name not yet specified.

fv = 75 ksi

Es = 29000 ksi

e rup = Infinity

File: F:\PROJECTS\4069\PCA\TOWER\CURRENT\SHORTD~1\SLOPEE.COL

Project:

Column:

fc = 10 ksi

Ec = 5000 ksi

fc = 8.5 ksi

Beta1 = 0.65

e\_u = 0.003 in/in

Confinement Tied

Engineer:

 $Ag = 2592 \text{ in}^2$ 

As = 189.00 in^2

Xo = 0.00 in

Yo = 0.00 in

Clear spacing = -0.72 in

64 # 14 TYP COLE

20 #14 BAPS ADD

84 #14 bars

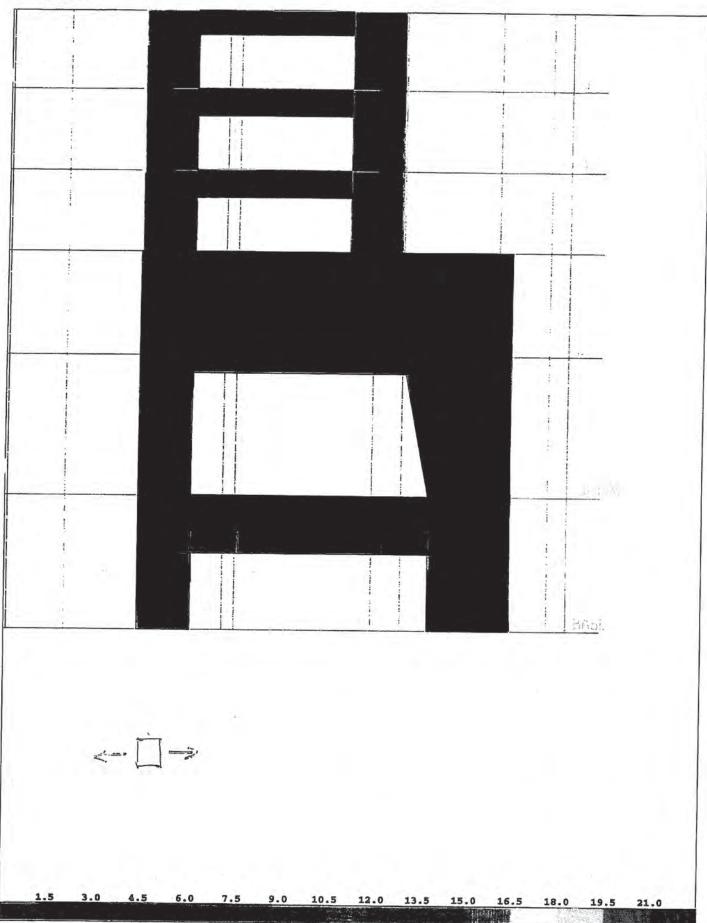
Rho = 7.29%

lx = 1.11974e+006 in^4

ly = 279936 in^4

Clear cover = 3.50 in 8.2.2-18

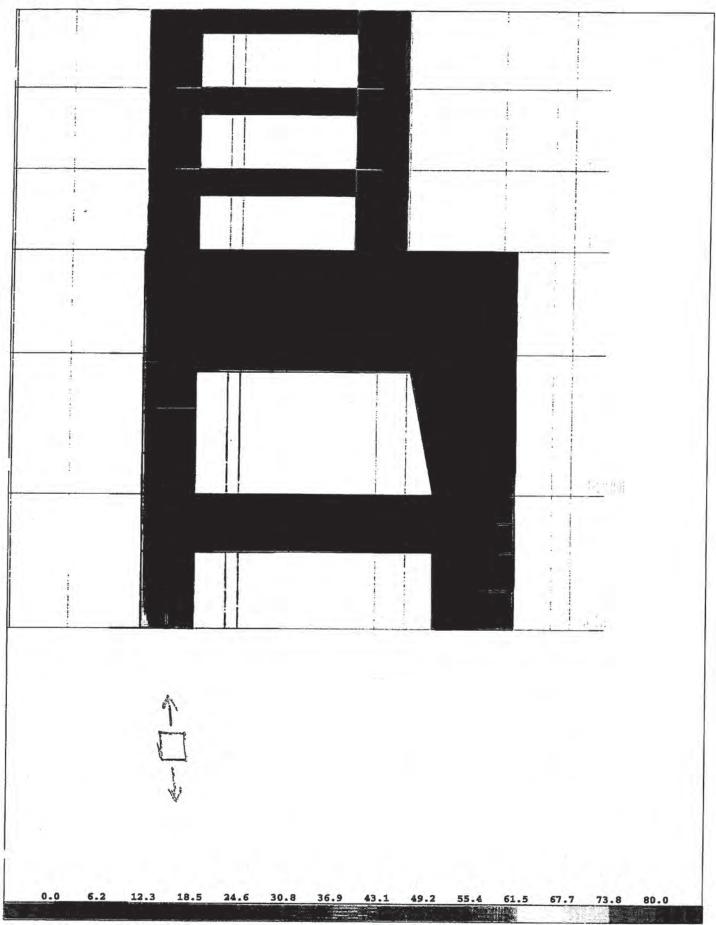




ETABS v8.4.9 - File: 4069-20050210-New Rigs 1.28 - February 10,2005 15:42 Elevation View - H Resultant F11 Diagram (E100X5100Y) - Kip-in Units

R17-19

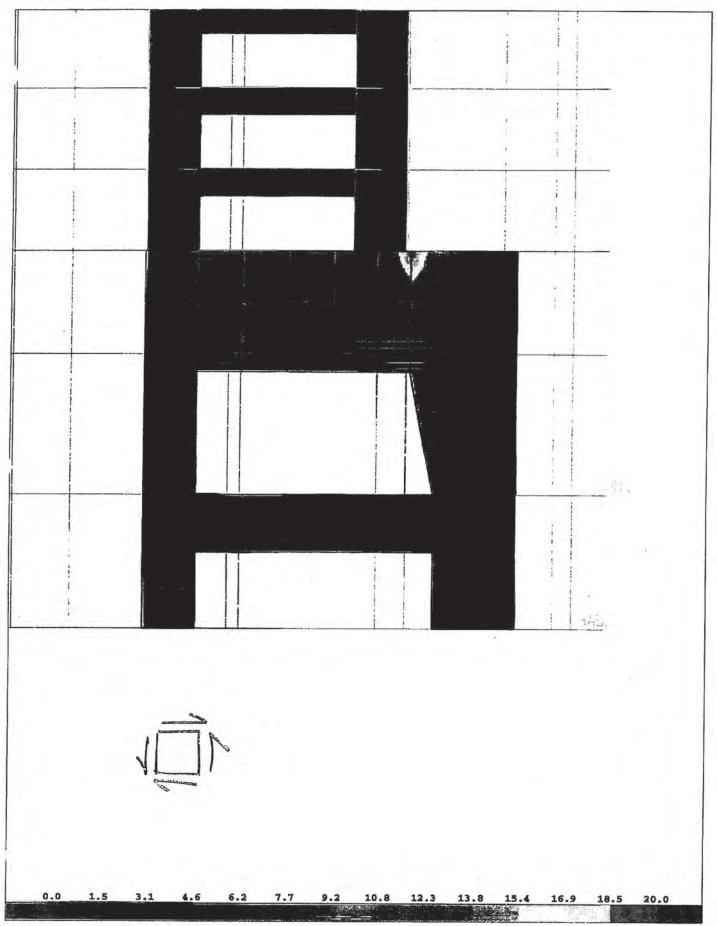
## ETABS



ETABS v8.4.9 - File: 4069-20050210-New Rigs 1.28 - February 10,2005 14:59 Elevation View - H Resultant F22 Diagram (E100X5100Y) - Kip-in Units

877-70

## ETABS



ETABS v8.4.9 - File: 4069-20050210-New Rigs 1.28 - February 10,2005 14:58 
Elevation View - H Resultant F12 Diagram (E100X5100Y) - Kip-in Units

301 Mission Street San Francisco, CA DESIMONE Project #4069

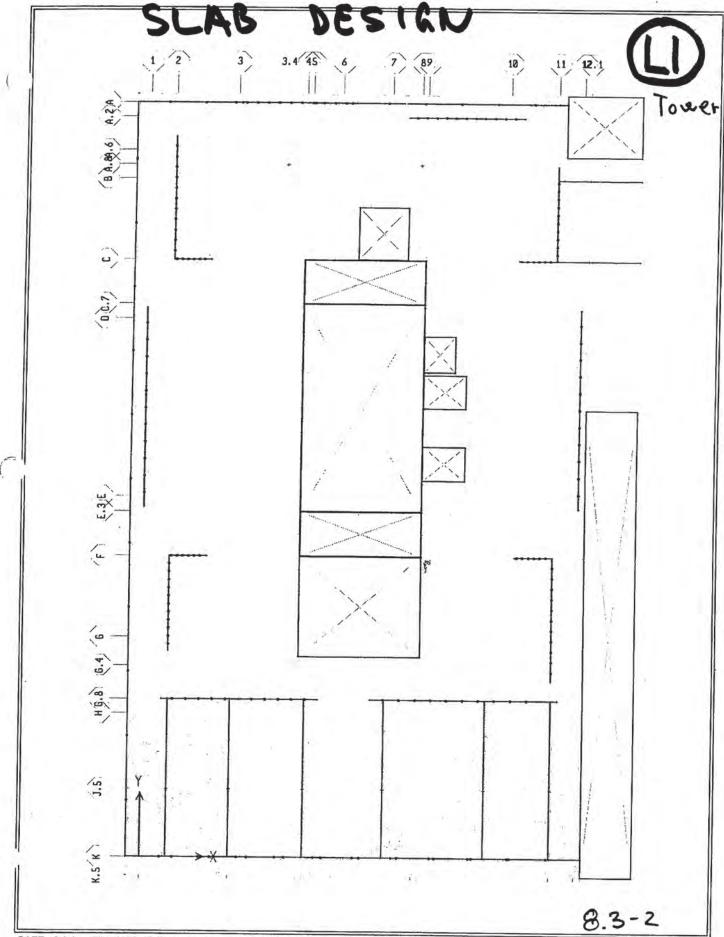
# 8.3 Gravity System

### **8.3 Gravity System**

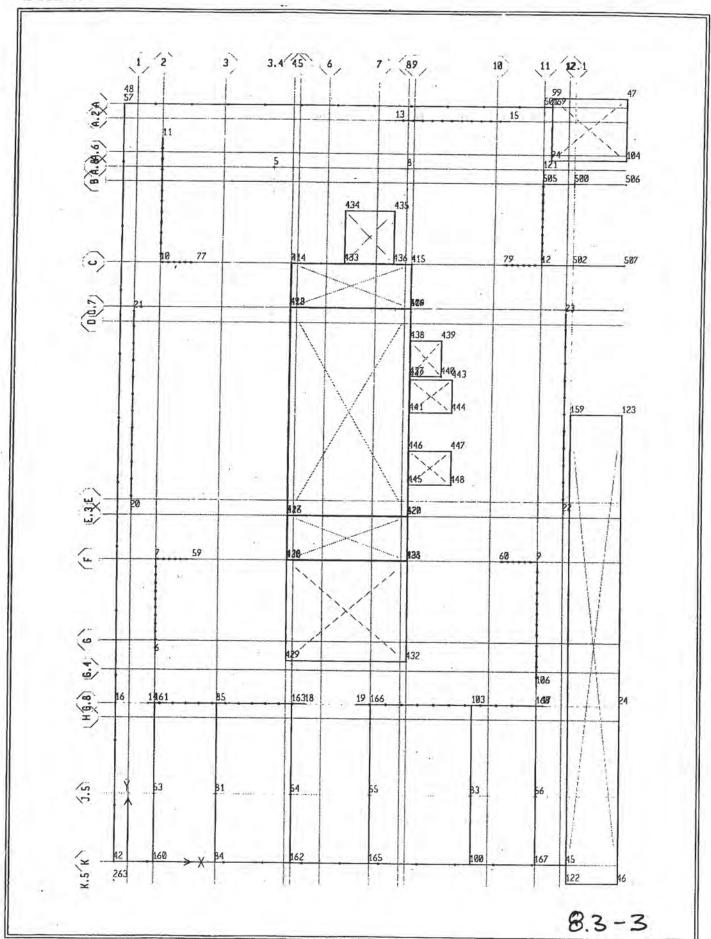
The 12 inch thick level 1 slab has been modeled in SAFE. The slab is seismically separated from the elevator core and is therefore simply supported.

The gravity beams at the southern side of the tower have been designed using SAFE.

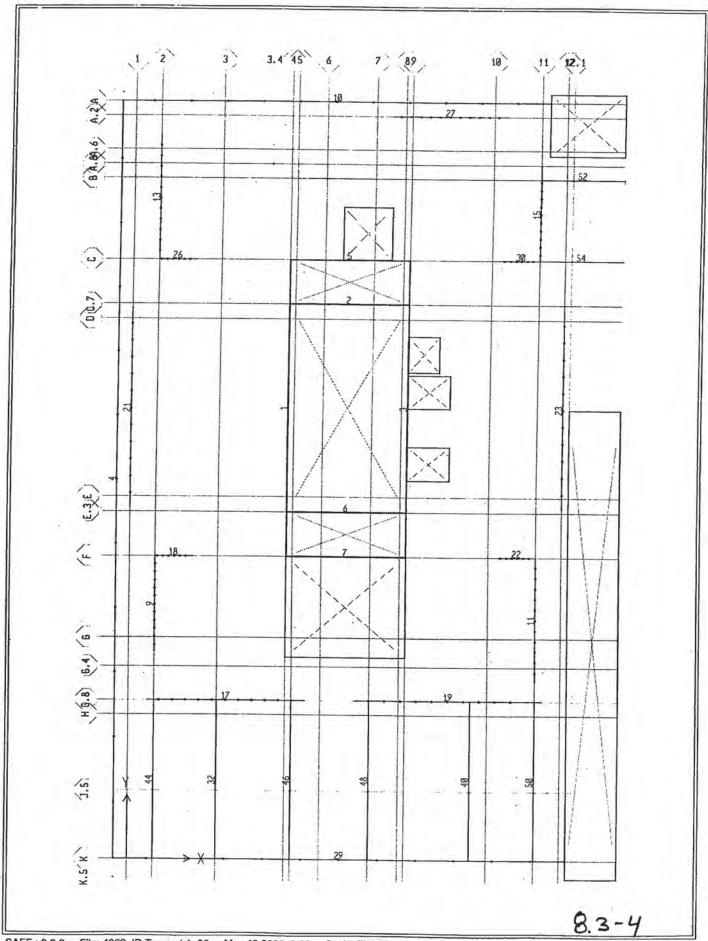
Loads for the gravity columns have been taken from SAFE slab analysis including beam transfers at levels 1 to level 3. The gravity columns have been design using PCA column. Slenderness effects have been accounted for in the design.



SAFE v8.0.6 - File: 4069-JP-Tower\_L1\_06 - May 18,2005 12:31 - Scale: Fit to Page Structural Layer Plan View - Kip-in Units



SAFE v8.0.6 - File: 4069-JP-Tower\_L1\_06 - May 19,2005 9:23 - Scale: Fit to Page



SAFE v8.0.6 - File: 4069-JP-Tower\_L1\_06 - May 19,2005 9:23 - Scale: Fit to Page Structural Laver Plan View - Kin-in Linite

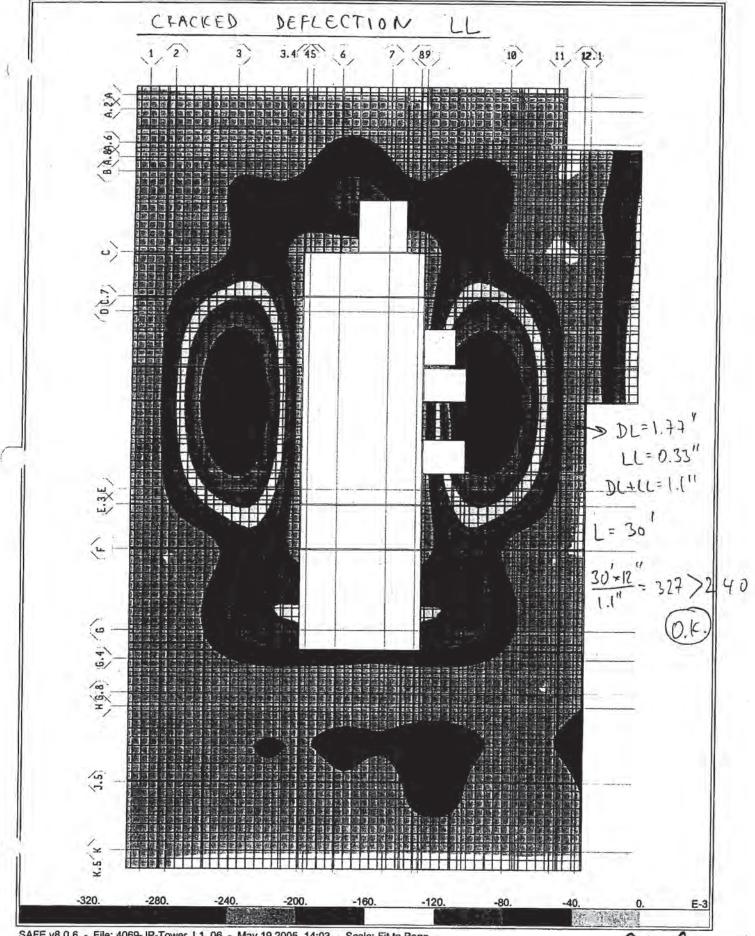
	1 2	3 3.4 45 6	7 89	10 11 12.1	
	1	Distance of the second	1 11		
A.S.	75.200 75.000	- 70 man 700 di 75 man 715	0005.00529075.0005.065.000	21 000 35 100	
24	73.600	75.000 MB. (0 75.000 75.	0005.005.8075.000.005.000	75.000 75 000	
8.6	75.000 - 75.000	75.000 75.05 75.000 -75.	0005.00520075.00%.065.009	75.000 75.05.000 75.000	ro
-	75.000 75.000	78.000 785.0 75.00 75.	2005.98529075.985.985.296	75.000 75.05.000 75.000	_
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SAFE v8.0.6 - File: 4069-JP-Tower\_L1\_06 - May 19,2005 9:16 - Scale: Fit to Page Surface Loading (SDL) - Ib-ft Units

<b>4</b> )-			3 3,4	Transport of the Park					10		
A.2.A	100.000	100.000	198.000198	189.29	0 23.00	20.200	000.00	.500.96	100.000 1	2 .000	
B. A. 86.6	136.900	100,000	100,200188	. 100.00	8 80.00	2.2100	00.00	. E00.00	100.000 1	300.00	100,000
, m	120,200	100.000	190.000 100	100.00	8 28.88	2.52	80.225	.20.00	100.000 1	.1000.00	100.220
2 K.	100.000	100.000	100,000183	. 100.00	9	10000	00.00	.20.02	128.200 1	.800.20	100.000
رْن	169.000	100.200	100.003106	1	199.20	100	.00.400	.508.82	100.900 15	.1990 . 992	100.000
- 10.7	100.000	100,000	100.000180	200 - 000 200	100.000	1	.00.90	108,08	100.000 10	1996 . 312	100.203
	199-096	100,000	198, 200 136	200		100	000 100	.100.00	130,000 10	.100.00	100.000
	100.038	100.000	100.000100	4			. 000 . 000	100.00	100.000 10	.00.00	100.000
	190.000	100,000	100.000100	000 000		1	08.990 08.990		100.000 to		
	190.000	100,000	199.099100	999		100.	Ш	130.00	100.000 12		
-3 <sub>x</sub>	100.006	190,000	130.000100				90. BM		190.000 12		
 				100 . 00m	100.000		00. and	200.00	199.009 12	.306.840	
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ا و	00.000	100.000	100.200			100.	00.200).	200. GE	102.200 12	.200.202	
4/	99.000	120.000	100,000162	100,000	00.00	0.12 000	60.0m).	10.20	199,006 12	190.900	
	52.020	250.000	250.900 <b>25</b> 6 .	258.002	50,000	. <b>2</b> 22501	50. 250.2	20.00	250.000 25	356 959	
K.5′K	50.000	250.7000	252,209225	250.000	59. 99 0	.20 950 S	SC. 1250 A	8.20	250, 000 25		

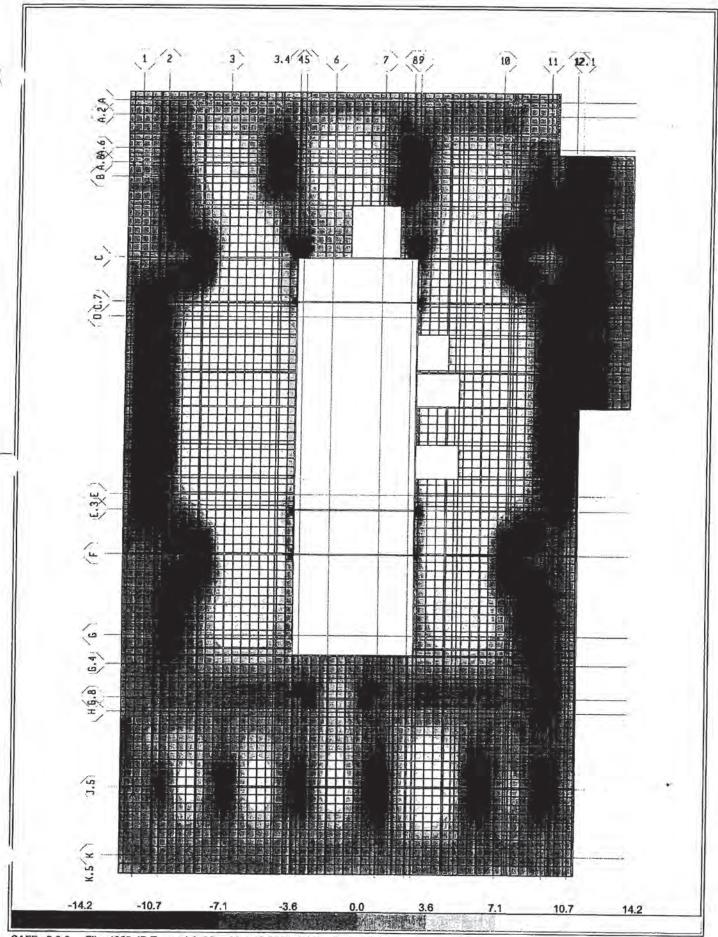
SAFE v8.0.6 - File: 4069-JP-Tower\_L1\_06 - May 19,2005 8:42 - Scale: Fit to Page Surface Loading (L) - Ib-ft Units

	1/2		3) 3.4	(5) 6	, 7	89	,		10	11/1	2)
A. S.	75.030	75.200	75.000 75	2 75.909	25.802	. 00.290	5.002	25,00	75.000	7. 000	
8.8.6	75.008	75.803	75.000 75	6 75.000	75.882	80 Z 90 7 S	. BVR.	Ø5.00	75.000	7. 85.00	75,900
- m	75.000	75.800	75.000 72	25.000		. 36 Z 80 PS			-		75,800
-160	75.006	75.000	75.900 79	25,000		7.280	. 90%	5.00	75.000 7	05.20	75,000
2)	75.000	75.200	75.00% 75 2		75.0005	75. 75	. 8078 . E	95.00	75.000 7	.25.20	75.000
16.2	75.990	25,000	75.000 75 8	00-	/5.0005.	75.75	. 2002 . 2	5.09	75.000 7	25.06	75.000
	75.900	75,000	75.000 75 0				- 1	5.00	75,000 7	. 05,00	75,000
	75.200	75.000	75,000 75 0	90		75.000	1. 7	5.00	75.000 7	.25.00	75.000
	75.000	75.000	75.000 75 0	90	ľ	75. 75.			75.900 7.	Ø5.20	
	75.008	75.000	75.000 75 0	00		75.000	7	5.00	75.900 7.	Ø5.000	
.3.E.	75.000	75.000	75.000 75 00	10	75 0005 (	75. 75.	200 . D	5.00	75,000 7	. 25. 200	
(F.	75.000	75.000	75.000 75 00			75. 75.	008.0	5,00	75,000 7	W5.000	
	75.000	75.000	75.000	and the second s		75. 75.	2003 . 83	5.00	75.800 7.	<b>2</b> 5.000	
H.G.8) (6.4)	75.900	75.000	75,988 754 €	75.000	75.000.0	85200 rs.1	2012 . 97	5.00	75.400 7.	Q5.20	
	7 <b>4</b> , 995	74.995	74.995 <b>78</b> .5	74.995	P4.9981.90	<b>3749</b> 74.9	P. P.	1.99	74.995 7	94.995	
K.5.K	74,995	74.995 <sup>A</sup>	74.995 72 5	74.995	4.99E .W	<b>Z4</b> 5 74.9	93,914	. 99	74.995 7	94.99	

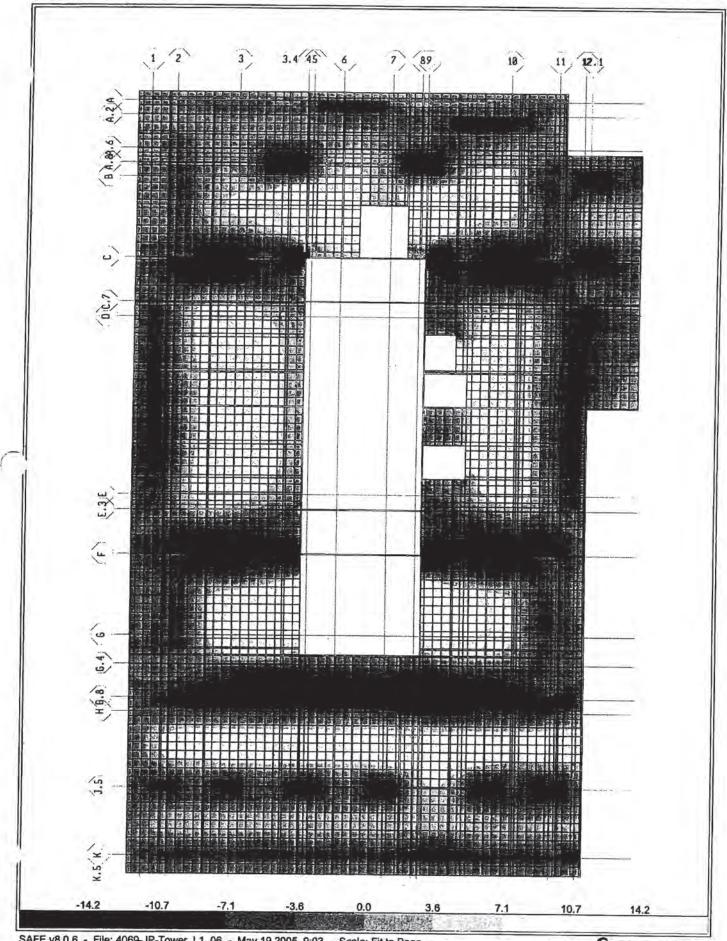


SAFE v8.0.6 - File: 4069-JP-Tower\_L1\_06 - May 19,2005 14:03 - Scale: Fit to Page Cracked Deformed Shape (L) - Kip-in Units

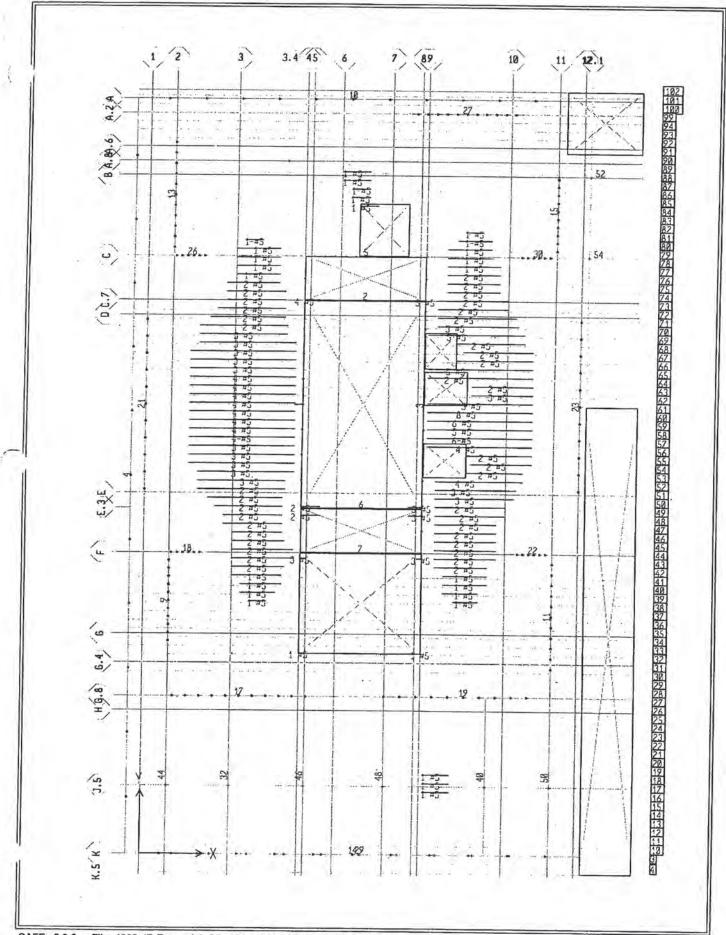
8.3-8



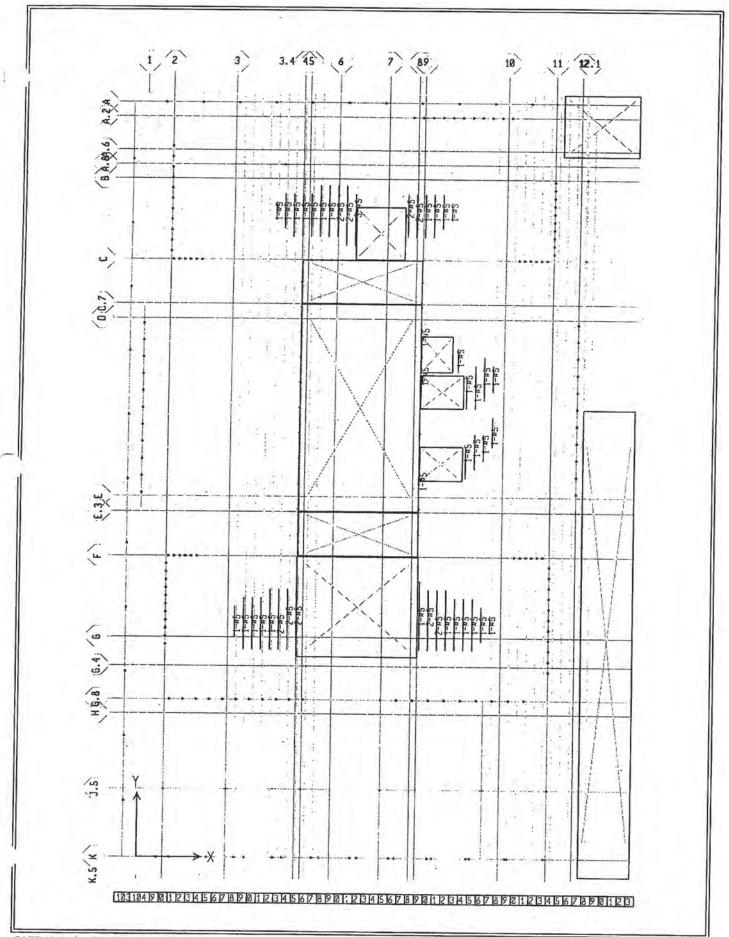
SAFE v8.0.6 - File: 4069-JP-Tower\_L1\_06 - May 19,2005 9:02 - Scale: Fit to Page Slab Resultant Mxx Diagram - (14D17L) - Kip-in Units



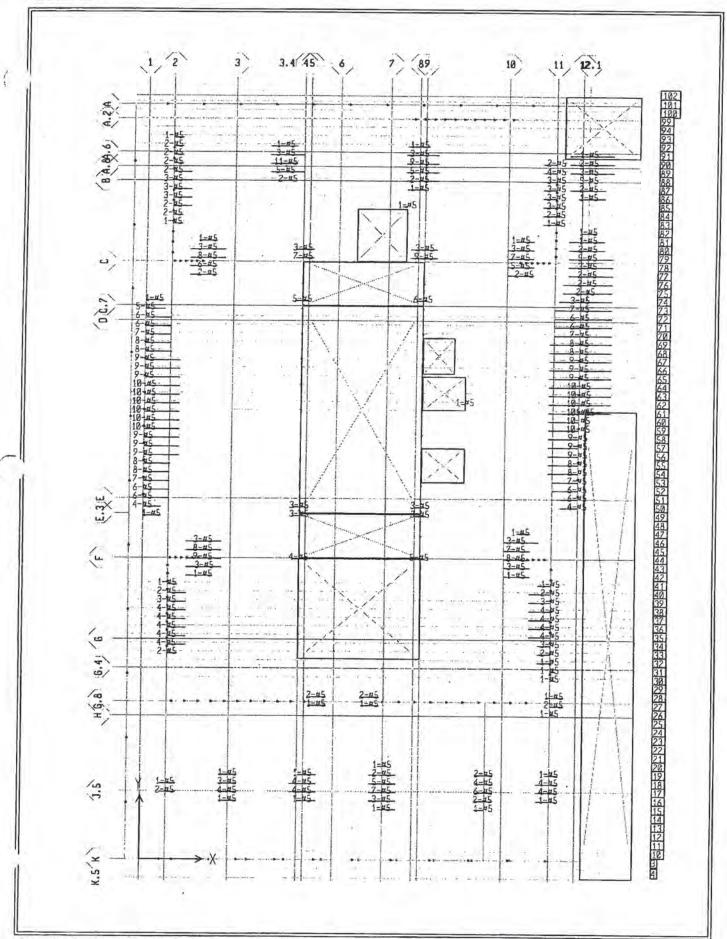
SAFE v8.0.6 - File: 4069-JP-Tower\_L1\_06 - May 19,2005 9:03 - Scale: Fit to Page Slab Resultant Myy Diagram - (14D17L) - Kip-in Units



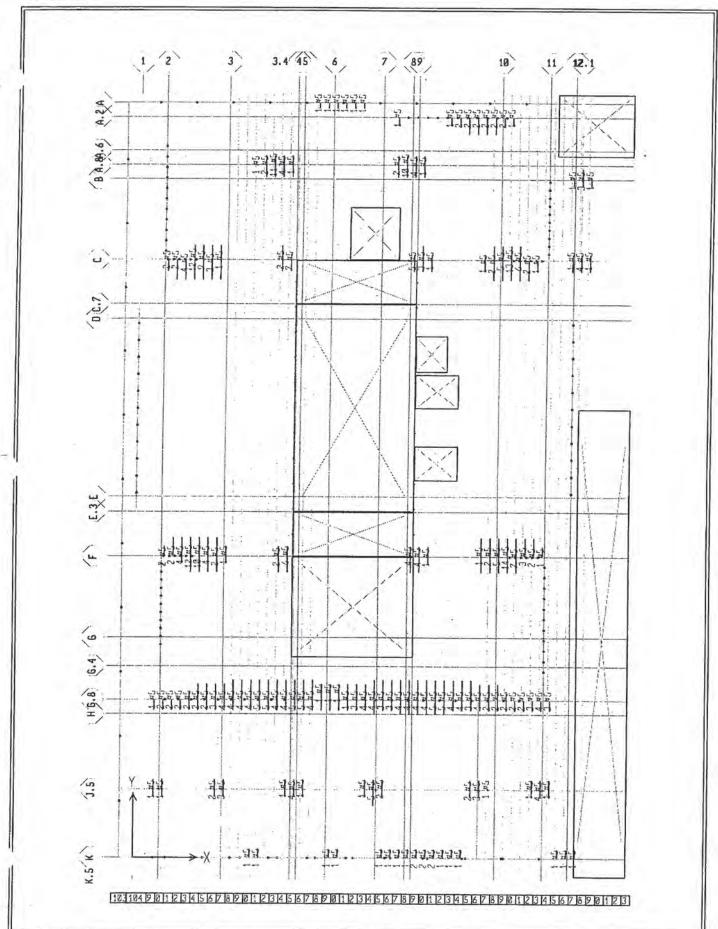
SAFE v8.0.6 - File: 4069-JP-Tower\_L1\_06 - May 19,2005 9:11 - Scale: Fit to Page Bottom X-Strip Reinforcement (Sq-in) in addition to #5 @ 12.000 (Bot) - Kip-in Units



SAFE v8.0.6 - File: 4069-JP-Tower\_L1\_06 - May 19,2005 9:11 - Scale: Fit to Page Bottom Y-Strip Reinforcement (Sq-In) in addition to #5 @ 12.000 (Bot) - Kip-in Units



SAFE v8.0.6 - File: 4069-JP-Tower\_L1\_06 - May 19,2005 9:11 - Scale: Fit to Page Top X-Strip Reinforcement (Sq-in) in addition to #5 @ 12.000 (Top) - Kip-in Units



SAFE v8.0.6 - File: 4069-JP-Tower\_L1\_06 - May 19,2005 9:11 - Scale: Fit to Page Top Y-Strip Reinforcement (Sq-in) in addition to #5 @ 12.000 (Top) - Kip-in Units

### Date: 5/19/2005 Time: 9:48 AM File: 4069-JP-Podium gravity columns and walls.xls

Jiri Pertold

**DESIMONE CONSULTING ENGINEERS** 10 United Nations Plaza, Suite 410

Job no. : Client

4069

San Francisco

Project :

Handel Architects 301 Mission street

CA 94102 T. 415.398.5740 F. 415.398.9834

Engineer:

Page No.: Revision:

### HIGH RISE SIDE

LEVEL L1

Reactions DON'T include wall self weight and cladding

### COLUMN SPRING REACTIONS

Z FZ L LL ips) (kips	1.4DL+1.7LL
ips) (kips	
	A /leina)
	(kips)
3 50	243
2 46	221
. 48	151
77	234
89	271
67	207
71	219
80	245
33	169
1	4
35	177
֡	3 50 2 46 48 77 89 67 71 80 33

	ONE	LEVEL	
WALL	FZ	FZ	
	DL	LL	1.4DL+1.7LL
	(kips)	(kips)	(kips)
1	287	127	618
2	-57	-25	-123
2 3 4	233	103	502
4	93	65	240
5	74	33	160
6	-60	-27	-130
7	20	9	43
9	85	38	184
10	142	63	306
11	106	46	227
13	101	45	218
15	40	18	87
17	242	177	640
18	123	55	265
19	282	215	760
21	272	121	587
22	120	53	259
23	314	140	677
26	114	51	245
27	81	36	175
29	285	301	912
30	100	45	216

SAFE v8.0.6 File: 4069-JP-TOWER\_L1\_06 Kip-in Units PAGE 1 May 19,2005 9:34

### LINE OBJECT DATA

### LINE JNT-1 JNT-2 SECTION SUPPORT RELEASES LENGTH

1	1	2	WALL30CORE	828,000	
2	49	50	WALL24CORE	330.000	
3	3	4	WALL30CORE	828.000	
4	42	57	WALL14	0 2103,000	
5	2	4	WALL36CORE	330.000	
6	51	52	WALL24CORE	330.000	
7	1	3	WALL36CORE	330.000	
9	6	7	WALL36	264.000	
10	57	169	WALL14	0 1175.000	
11	106	9	WALL36	344.000	
13	10	11	WALL36	344.000	
15	12	121	WALL36	264.000	
17	14	18	WALL36	428.000	
18	7	59	WALL42	102.000	
19	19	17	WALL36	511.000	
21	20	21	WALL36	556.000	
22	60	9	WALL42	102.000	
23	22	23	WALL36	556.000	
26	10	77	WALL42	102.000	
27	13	15	WALL36	0 316.000	
29	42	45	WALL14	1241.000	
30	79	12	WALL42	102.000	
32	84	85	18X18	437.000	
40	100	103	18X18	437.000	
44	160	161	18X18	437.000	
46	162	163	18X18	437.000	
48	165	166	18X18	437.000	
50	167	168	18X18	437.000	
52	505	506	18X18	228.000	
54	12	507	18X18	228.000	

SAFE v8.0.6 File:  $4069\text{-JP-TOWER\_L1\_06}$  Kip-in Units PAGE 2 May  $19,2005\ 9:34$ 

### POINT OBJECT DATA

POINT	GLOBA	L-X GLOB	AL-Y SUPPORTSPRIN	IG RESTRAINT	RES DIM X	RES DIM Y
1	428.000	836.000				
2	428.000	1664.000				
3	758.000	836.000				
4	758.000	1664.000				
5	377.000	1930.000	18X18			
6	70.000	572.000	A45.3.2			
7	70.000	836.000				
8	741.000	1930.000	18X18			
9	1116.000	836.000				
10	70.000	1664.000				
11	70.000	2008.000				
12	1116.000	1664.000				
13	705.000	2063.000				
14	53.000	437.000				
15	1021.000	2063.000				
16	-37.000	437.000				
17	1134.000	437.000				100
18	481.000	437.000				
19	623.000	437.000				
20	0.000	972.000				
21	0.000	1528.000				
22	1186.000	972,000				
23	1186.000	1528.000				
24	1344.000	437,000				
	COOCIETE	353356				

00	27.000	24.000	
25	-37.000	24.000	
26	1344.000 1344.000	0.000	
	1344.000	48.000	
28	-37.000	48.000	
29		72.000	
30	1344.000		
31	-37.000	72.000	
32	1344.000	96.000	
33	-37.000	96.000	
34	1344.000	120.000	
35	-37.000	120.000	
36	1344.000	144.000	
37	-37.000	144.000	
38	1344.000	168.000	
39	-37.000	168.000	
40	1344.000	192.000	
42	-37.000	0.000	
45	1204.000	0.000	
46	1344.000	-51.000	
47	1344.000	2127.000	
48	-37.000	2127.000	
49	428.000	1540.000	
50	758.000	1540.000	
51	428,000	960.000	
52	758.000	960.000	
53	70.000	188.000	18X18
54	445.000	188.000	18X18
55	660.900	188.000	18X18
56	1116.000	188.000	18X18
57	-37.000	2103.000	16000
59	172.000	836.000	
60	1014.000	836.000	
61	-37.000	192.000	
62	1344.000	216.000	
63	-37.000	216.000	
64	1344,000	240.000	
	-37.000	240.000	
65	1344.000	264.000	
66		264.000	
67	-37.000		
68	1344.000	288.000	
69	-37.000	288.000	
70	1344.000	312.000	
71	-37.000	312.000	
72	1344.000	336.000	
73	-37.000	336.000	
74	1344.000	360.000	
75	-37.000	360.000	
76	1344.000	384.000	
77	172.000	1664.000	
78	1344.000	408.000	
79	1014.000	1664.000	
80	1344.000	432.000	1204400
81	241.000	188.000	18X18
82	1344.000	456.000	100000
83	938.000	188.000	18X18
84	241.000	0.000	
85	241.000	437.000	
86	1344.000	504.000	
88	1344.000	528.000	
89	-37.000	528.000	
90	1344.000	552.000	
91	-37.000	552.000	
92	1344.000	576.000	
93	-37.000	576,000	
94	1138.000	1955.000	
95	1344.000	600.000	
96	-37.000	600.000	
97	1344.000	624.000	
98	-37.000	624.000	
99	1138.000	2127.000	
30	1100.000	2121.000	

220	1 22222	2000
100	938.000	0.000
101	1344.000	648,000
102	-37.000	648.000
103	938.000	437.000
104	1344.000	
105	1344.000	-24.000
106	1116,000	492.000
107	1344.000	672.000
108	-37.000	672.000
109	1344.000	696.000
110	-37.000	696,000
111	1344.000	720.000
112	-37.000	720.000
113	1344,000	744.000
114	-37.000	744.000
115	1344.000	768.000
116	-37.000	768.000
117		
	1344.000	792.000
118	-37.000	792.000
119	1344.000	816.000
120	-37.000	
2.25		816.000
121	1116.000	1928.000
122	1204.000	-51.000
123	1344.000	1246,000
125	1344.000	840.000
126	-37.000	840.000
127	1344.000	864.000
127		
128	-37.000	864.000
129	1344.000	888.000
130	-37.000	888.000
131	1344.000	912,000
132	-37.000	912.000
133	1344.000	936,000
134	-37.000	936.000
135	1344.000	960,000
136	-37.000	960,000
137	1344.000	984.000
138	-37.000	984.000
139	1344.000	1008.000
140	-37.000	1008.000
141	1344.000	1032,000
142	-37.000	1032.000
143	1344.000	
	1344.000	1056.000
144	-37.000	1056.000
145	1344,000	1080,000
146	-37.000	1080.000
6 - 6 - 6 - 6 - 6 - 6 - 6 - 6 - 6 - 6 -		
147	1344.000	1104.000
148	-37.000	1104.000
149	1344.000	1128,000
150		
	-37.000	1128.000
151	1344.000	1152.000
152	-37.000	1152.000
153	1344.000	
		1176.000
154	-37.000	1176.000
155	1344.000	1200.000
156	-37.000	
		1200.000
157	1344.000	1224.000
158	-37.000	1224.000
159	1204.000	1246.000
160	70.000	0.000
161	70.000	437.000
162	445.000	0.000
163	445.000	437.000
164	1344.000	1248,000
165	660.900	100.15.55.6
		0.000
166	660.900	437.000
167	1116.000	0.000
168		
	1116.000	437.000
169	1138.000	2103.000
177	-37.000	1248.000
7.	77777	

470	4044.000	
178	1344.000	
179	-37.000	1272.000
180	1344.000	1296,000
181	-37.000	1296,000
182	1344.000	
183	-37.000	1320.000
184	1344.000	1344.000
185	-37.000	1344,000
0.00		
186	1344.000	1368.000
187	-37.000	1368.000
188	1344,000	1392,000
189		1392.000
	-37.000	
190	1344.000	1416.000
191	-37.000	1416,000
192	1344.000	1440.000
193	-37.000	1440.000
194	1344.000	1464,000
195	-37.000	1464.000
196	1344.000	1488.000
197	-37.000	1488.000
198	1344.000	1512.000
199	-37.000	1512.000
200	1344.000	1536.000
201	-37.000	1536,000
202		1100
	1344.000	1560.000
203	-37.000	1560.000
204	1344.000	1584.000
205	-37.000	
		1584.000
206	1344.000	1608.000
207	-37.000	1608.000
208	1344,000	1632.000
209	-37.000	1632.000
210	1344,000	1656.000
211	-37.000	
		1656.000
212	1344.000	1680.000
213	-37.000	1680.000
214	1344.000	
		1704.000
215	-37.000	1704.000
216	1344.000	1728.000
217	-37.000	
		1728.000
218	1344.000	1752.000
219	-37.000	1752.000
220	1344.000	
	1 - 11 - 1 - 1	1776.000
221	-37.000	1776.000
222	1344,000	1800,000
223	-37.000	1800.000
224	1344.000	1824.000
225	-37.000	1824.000
226	1344.000	1848,000
227	-37.000	1848.000
228	1344.000	1872.000
229	-37.000	1872.000
230	1344.000	1.50.000
		1896.000
231	-37.000	1896.000
232	1344.000	1920.000
233	-37.000	
		1920.000
234	1344.000	1944.000
235	-37.000	1944.000
236	1344.000	
		1968.000
237	-37.000	1968.000
238	1344.000	1992.000
239	-37.000	
		1992.000
240	1344.000	2016.000
241	-37,000	2016.000
242	1344.000	2040.000
243	-37.000	2040.000
245	1344.000	2139.996
246	24.000	-51.000
247	-37.000	2139.996
48	-37.000	480.000
. 10	07.000	400.000

414		50025
249	-37.000	-60.000
251	1344.000	-60.000
252	1344.000	2064.000
253	-37.000	2064.000
254	1344.000	2088.000
255	-37.000	2088.000
256	1344.000	2112.000
257	-37.000	2112.000
258	1344.000	-51.000
259	1344.000	2127.000
260	240.000	-51.000
261	0.000	-51.000
262	0.000	2127.000
263	-37.000	-51.000
264	-24.000	-51.000
265	-24.000	
		2127.000
269	24.000	2127.000
270	240,000	2127.000
273	-37,000	-51,000
281	48.000	-51.000
282	48.000	2127.000
283	72.000	-51.000
284	72.000	2127.000
285	96.000	-51.000
286	96.000	2127.000
287	120:000	-51,000
288	120.000	2127.000
289	144.000	-51.000
290	144,000	2127.000
291	168,000	-51.000
292	168.000	2127.000
293	192.000	-51.000
294	192,000	2127.000
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296	216.000	2127.000
298	-37.000	2127.000
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300	264.000	2127.000
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303	312.000	
		-51.000
304	312.000	2127.000
305	336.000	-51.000
306	336,000	2127.000
		and the same of th
307	360.000	-51.000
308	360.000	2127.000
309	384.000	-51.000
310	384.000	2127.000
311	408.000	-51.000
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330	624.000	2127.000
331	648.000	-51.000
332	648.000	2127.000

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		2127.000
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353	912.000	-51.000
354	912.000	2127.000
355	936.000	-51.000
356	936.000	2127,000
357	960.000	-51.000
358	960.000	2127.000
359	984.000	-51.000
360	984.000	2127.000
361	1008.000	-51.000
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364	1032.000	2127.000
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366	1056.000	2127.000
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370		
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372	1128.000	2127.000
373	1152.000	-51.000
374	1152.000	2127.000
375	1176,000	-51.000
376	1176.000	2127.000
377	1200.000	-51.000
378	1200.000	2127.000
379	1224.000	-51.000
380	1224.000	2127.000
381	1248.000	-51.000
382	1248.000	2127.000
383	1272.000	-51.000
384	1272.000	2127.000
304		
385	1296.000	-51.000
386	1296.000	2127.000
7		
387	1320.000	-51.000
388	1320.000	2127.000
413	429.000	1541.000
414	429.000	1663.000
415	757.000	1663.000
416		
	757.000	1541.000
417	429.000	961.000
418	429.000	1539.000
C C X		
419	757.000	1539.000
420	757.000	961.000
425	429.000	837.000
		2.
426	429.000	959.000
427	757.000	959.000
128		
	757.000	837.000
129	428.000	556.000
130	428.000	835.000
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431
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                    1665.000
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                    1450.000
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                    1142.000
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                    1048.000
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                    -24.000
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490
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492
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                                   18X18
                                   18X18
18X18
501
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       1204.000
502
                    1664.000
505
        1116.000
                    1890.000
506
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                    1890.000
507
       1344.000
                    1664.000
511
       1344.000
                    480.000
518
                   504.000
        -37.000
```

SAFE v8.0.6 File: 4069-JP-TOWER\_L1\_06 Kip-in Units PAGE 1 May 19,2005 9:35

### COLUMN SPRING REACTIONS

COLU	IMN (	GRID I	GRID J	LOAD	FZ	MX	MY
53 53	7	12 12	L	48.41 48.92	49.948 48.747	-91.018 -87.914	
54 54	29 29	12 12	L DL	76.58 74.09	98.084 86.149	-7.761 -3.534	
55 55	43 43	12 12	L	89.09 85.31	107.914 94.885	-87.308 -81.532	
56 56	75 75	12 12	L	66.93 66.64	103.459 100.865	-92.343 -86.811	
81 81	17 17	12 12	L	70.92 70.06	86.022 82.064	-29.429 -25.587	
83 83	64 64	12 12	L	80.02 77.88	93.284 87.996	150.146 135.145	
500 500	83 83	114 114	L	32.67 81.24	-48.418 -111.661	-198.309 -488.023	
501 501	75 75	130 130	L	0.91	-14.787 -32.951	1.274	
502 502	83 83	102 102	L	34.58 84.53	34.030	-213.572 -513.973	
5	23 23	118 118	L DL		-134.016 -301.546	-49.079	
			-	10000	30 110		

8 49 118 L 45.53 -156.876 26.176 8 49 118 DL 102.44 -352.995 58.882

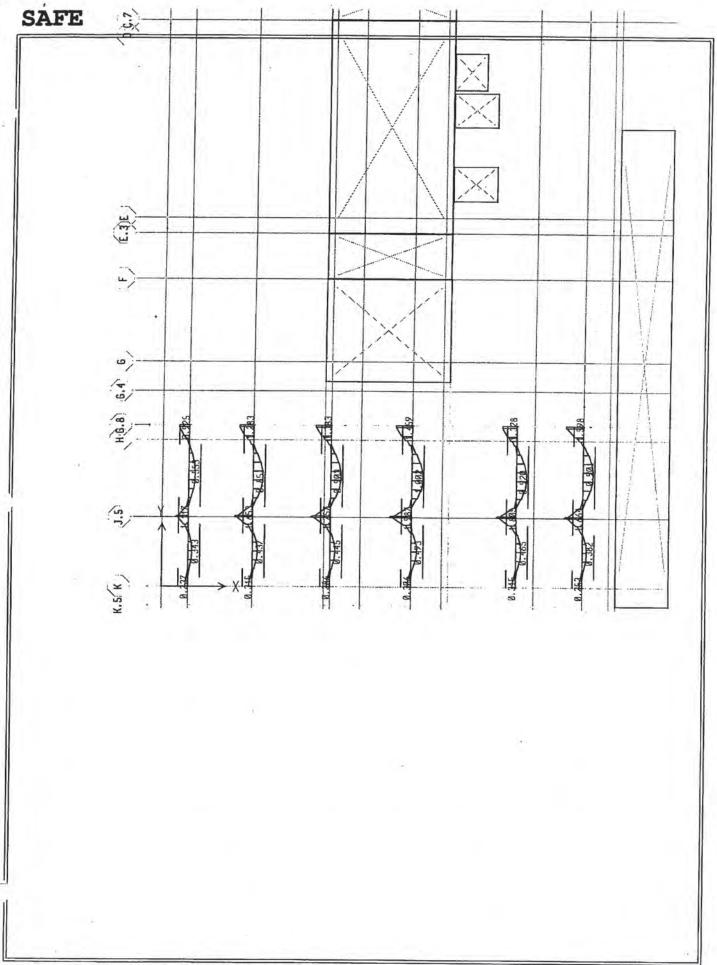
SAFE v8.0.6 File: 4069-JP-TOWER\_L1\_06 Kip-In Units PAGE 2 May 19,2005 9:35

### WALL REACTIONS

WA	LL L	DAD	FZ	MX	MY
1	L	127.46			0.000
- 1	DL	286.7	3 -576	1.649	0.000
3	L	103.43	-1120	.156	0.000
3	DL	232.6	8 -251	5.311	0.000
5	L	33.04			510
5	DL	74.35			8.804
7	L	8.73	0.00		
7	DL	20.12			2.738
9	L	37.74	-1964.		
9	DL				91.776
		85.42			170.747
11	L	45.89	-960.		1.231
11	DL	106.3			508.058
13	L	44.93	1930.		74.936
13	DL	101.0	9 434	4.398 -2	418.692
15	L	18.35	1504.		8.341
15	DL	39.70	3300	.195 2	231,210
17	L	176,61	-2047		98.576
17	DL	242.46			339.580
19	1	214.54			22.471
19	DL	282.40			709.072
21	L	120.99	80.5		
					7.495
21	DL	272.22			383.453
23	E	139.57	5311.		45.688
23	DL	314.32			927.567
27	L	36.06	-1181.		39.807
27	DL	81.12	-2657	.445 -24	150.260
29	L	301.09	2802.	374 -42	07.260
29	DL	285.48	2843		802.895
2	L	-25.33	0.000	-358.	374
2	DL	-56.99	0.00		
6	1	-26.87	0.000		
6	DL	-60.44	0.00		
18	L	54.54	667.1		9.141
18	DL	122.74			
22	L	53.47		.1/1 -0	123.846
			650.4		8.867
22	DL	120.35			373.930
26	L	50.58	-785.8		3.531
26	DL	113.82			340.615
30	L	44.54	-590.8	20 264	8.230
30	DL	99.91	-1317.		66.472
4	L	65.00 -	27670.2	66 -117	2.941
4	DL		-18655.		29.634
10	L	63,15	-1240.3		5.941
10	DL	142.09			157.921
	-	142.00	2,00		101.021

SAFE v8.0.6 - File: 4069-JP-Tower\_L1\_06 - May 19,2005 9:18 - Scale: Fit to Page Beam Flexural Reinforcement (Sq-in) - Ib-ft Units

8.3-24



6.4

1.

# COLUMN DESIGN (I)

0000	0000	000	000	00	000	00	000	00000		00	
00	00	00	00	00	00	00	00	00	00	00	
00	00	00	00	00	00	00	00	00	00	00	
00	00	00		00	00	00		00	00	00	
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00		00	00	00	00	00	00	00	00	00	
00		000	000	00	00	000	000	000	000	00000	(MT)

Computer program for the Strength Design of Reinforced Concrete Sections

Licensee stated above acknowledges that Portland Cement Association (PCA) is not and cannot be responsible for either the accuracy or adequacy of the material supplied as input for processing by the PCACOL(tm) computer program. Furthermore, PCA neither makes any warranty expressed nor implied with respect to the correctness of the output prepared by the PCACOL(tm) program. Although PCA has endeavored to produce PCACOL(tm) error free, the program is not and can't be certified infallible. The final and only responsibility for analysis, design and engineering documents is the licensees. Accordingly, PCA disclaims all responsibility in contract, negligence or other tort for any analysis, design or engineering documents prepared in connection with the use of the PCACOL(tm) program.

### General Information: \_\_\_\_\_

File Name: F:\PROJECTS\4069\PCA\TOWER\L1COL\B01-03.COL

Project: 4069 Column: B4-24x48" Code: ACI 318-95

Engineer: JP Units: English

Run Option: Investigation

Run Axis: X-axis

Slenderness: Considered Column Type: Structural

Rupture strain = Infinity

### Material Properties:

f'c = 7 ksi= 3043 ksi Ec fc = 5.95 ksi

fy = 60 ksiEs = 29000 ksi

Ultimate strain = 0.003 in/in

Betal = 0.7

### Section:

Rectangular: Width = 18 in

Depth = 18 in

Gross section area, Ag = 324 in^2

 $Ix = 8748 in^4$ Xo = 0 in

 $Iy = 8748 in^4$ Yo = 0 in

### Reinforcement: \_\_\_\_\_

Rebar Database: ASTM A615

S	ize	Diam (i	n) Are	ea (in	^2)	S	ize	Diam (	in)	Area	(in^2)	S.	ize	Diam	(in)	Area	(in^2)
-						-						-					
#	3	0.	38	0	.11	#	4	0	.50		0.20	#	5		0.63		0.31
#	6	0.	75	0	.44	#	7	0	.88		0.60	#	8		1.00		0.79
#	9	1.	13	1	.00	#	10	1	.27		1.27	#	11		1.41		1.56
#	14	1.	59	2	. 25	#	18	2	.26		4.00						-100

Confinement: Tied; #3 ties with #10 bars, #4 with larger bars. phi(a) = 0.8, phi(b) = 0.9, phi(c) = 0.7

Layout: Rectangular

Pattern: All Sides Equal (Cover to transverse reinforcement)

Total steel area, As = 8.00 in^2 at 2.47%

8 #9 Cover = 1.5 in

### Slenderness: \_\_\_\_\_

### Sway Criteria:

------

X-axis: Braced column.

Column	Axis	Height ft	Width in	Depth in	I in^4	f'c ksi	Ec ksi
Design Above Below	x x x		18 specified) specified)	18	8748	7	3043
X-Beams Locatio	0.00	Length ft	Width in	Depth in	in^4	f'c ksi	Ec ksi
Above I Above F Below I Below R	Right Left	30 30 30 30	30 30 36 36	12 12 12 12	4320 4320 5184 5184	7 7 7 7	1000 1000 1000 1000

### Effective Length Factors:

Axis	Psi(top)	Psi(bot)	k(Braced)	k(Sway)	klu/r
X	11.737	9.781	1.000	(N/A)	34.06

### Moment Magnification Factors:

Stiffness reduction factor, phi(K) = 0.75

Cracked-section coefficients: cI(beams) = 0.35; cI(columns) = 0.7

 $0.2 \times \text{Ec} \times \text{Ig} + \text{Es} \times \text{Ise} (X-\text{axis}) = 1.28 \text{e} + 007 \text{ kip-in}^2$ 

X-axis		Braced			Sway	
Ld/Comb	Pc(kip)	Betad	Cm	Delta	Pc(kip)	
1 U1	2776	0.454	1.000	1.154	N/A	

### Load Combinations:

U1 = 1.400\*Dead + 1.700\*Live + 0.000\*Lateral

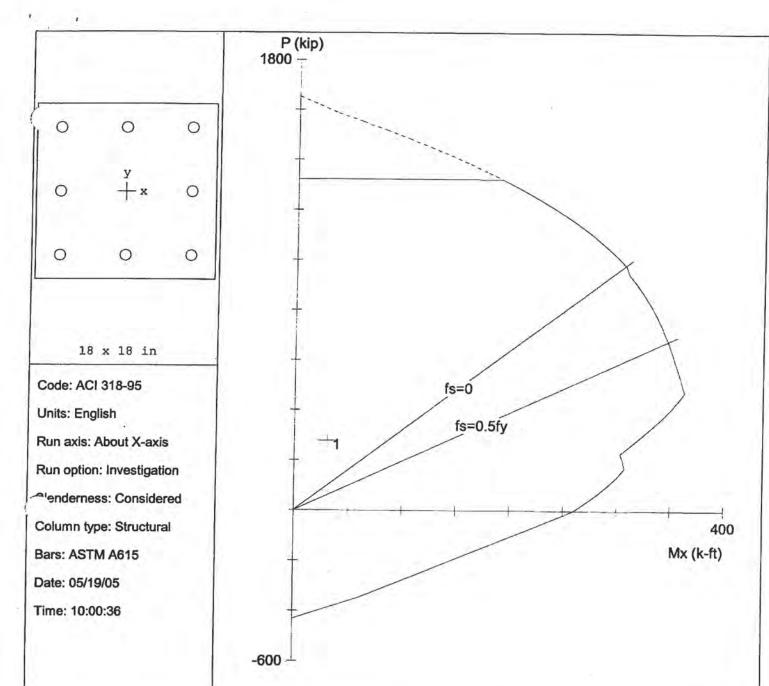
### Service Loads:

No.	Load Case				My @ Top k-ft	My @ Bot k-ft
1	Dead	90.0	0.0	0.0	0.0	0.0
	Live	89.0	0.0	0.0	0.0	0.0
	Latl	0.0	0.0	0.0	0.0	0.0

Factored Loads and Moments with Corresponding Capacities: (see user's manual for notation)

	Lo	ad	Pu	Mux	fMnx	
No.	Cor	nbo	kip	k-ft	k-ft	fMn/Mu
1	1	U1	277.3	30.4	317.7	10.454

\*\*\* Program completed as requested! \*\*\*



### PCACOL V3.00 (PCA 1999) - Licensed to: Licensee name not yet specified.

File: F:\PROJECTS\4069\PCA\TOWER\L1COL\B01-03.COL

Project: 4069

Column: B4-24x48"

fc = 7 ksi

fy = 60 ksi

Engineer: JP Ag = 324 in^2

8 #9 bars

Ec = 3043 ksi

Es = 29000 ksi

As = 8.00 in^2

Rho = 2.47%

c = 5.95 ksi

e\_rup = Infinity

Xo := 0.00 in

Ix = 8748 in^4

e\_u = 0.003 in/in

Yo = 0.00 in

ly = 8748 in^4

Beta1 = 0.7

1.4

Confinement: Tied

Clear spacing = 5.43 in

Clear cover = 1.88 in 8.3-29